



AGENDA
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
DECEMBER 13, 2018
9:00 AM
CITY HALL COMMISSION CHAMBERS
204 ASH STREET
FERNANDINA BEACH, FL 32034

1. CALL TO ORDER

2. ROLL CALL

3. APPROVAL OF MINUTES

3.1 September 13, 2018 Regular Meeting

3.2 September 27, 2018 Regular Meeting

3.3 November 8, 2018 Regular Meeting

4. DISCUSSION ITEMS

4.1 Change of Use at 329 S. 8th Street (329 Place) converting from an adult day care to a religious facility "Love Walk Church" with ancillary functions such as daycare, office space, and future private school. Pastor Dan Sawyer will be present with contractor to discuss building and site changes. This property is zoned MU-8.

4.2 Change of Use at 1016 S. 14th Street: Guilherme Camargo is considering opening juice bar with smoothies, protein shakes, solids and sandwiches. H&R Block was the most recent tenant, this property is zoned C-2.

4.3 Pre-Application Discussion: John Cotner, Cotner & Associates - A proposed multi-tenant commercial building at the northwest corner of South 9th and Date Street.

4.4 Change of Use at 1214 Beech Street: Bruce Palmer - Possibly converting retail space back to a restaurant. The property is zoned C-1.

4.5 Pre-Application discussion: Phillip Feora, United Brothers Development Corporation - to discuss operating a concrete recycling plant at 3169 Bailey Road. This property is zoned I-1

4.6 Pre-Application Discussion: Gillette & Associates for Bent Wing Flight Services - Development of 5 hangers on City property at the Airport. The project is located to the north of the new FBO on I-A zoned property

4.7 Change of Use at 22 S. 8th Street: Mike McCluskey representing CMR Island Properties -A new grocery store/restaurant plans to open in the former BuyGo space, adding 40 seats for the restaurant.

- 4.8 Pre-Application Discussion: Phil Griffin will be present to discuss the possibility of operating a craft distillery at 400 S. 8th Street. This property is zoned MU-8

5. DEVELOPMENT INQUIRIES FROM APPLICANTS NOT ON THE AGENDA - DISCUSSION ONLY, NO ACTION TO BE TAKEN.

6. COMMITTEE BUSINESS

- 6.1 Discuss Municipal Code revisions with regard to underground electric utilities.
- 6.2 Sign-off: SPR 2018-03 1015 Atlantic Avenue Condo
- 6.3 Sign-off: SPR 2018-05 Skyway Amelia
- 6.4 2nd Distribution of SPR 2018-10 Courtyard/Springhill
- 6.5 1st Distribution of SPR 2018-12 Holiday Inn Express

7. ADJOURNMENT

THE NEXT REGULARLY SCHEDULED TRC MEETING ON DECEMBER 27, 2018 IS CANCELLED. THE NEXT MEETING WILL BE HELD ON JANUARY 10, 2019

Persons with disabilities requiring accommodations in order to participate in this program or activity should contact 310-3115, TTY 711, or through the Florida Relay Services at 1-800-955-8771 (TTY number for all City offices) at least 24 hours in advance to request such accommodations.



**MINUTES
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
SEPTEMBER 13, 2018**

1. CALL TO ORDER – 9:03 AM

2. ROLL CALL

MEMBERS PRESENT:

Donald Kukla, Building	Sal Cumella, Historic Preservation
Jacob Platt, Planning	Rex Lester, Streets
Michelle Forstrom, Code Enforcement	Andre Desilet, Stormwater
John Mandrick, Utilities	

3. APPROVAL OF MINUTES: *June 28, 2018 Regular Meeting, July 26, 2018 Regular Meeting*

ACTION TAKEN: A motion was made by Member Desilet, seconded by Member Forstrom, to approve all minutes as presented.

Motion passed by unanimous voice vote.

4. DISCUSSION ITEMS

4.1 Pre-Application Discussion: *Gillette & Associates – Redevelopment of 2700 Atlantic Avenue into a 239 room hotel with an in house restaurant, including ground floor retail. This property is zoned C-1.*

Nick Gillette and Ron Flick were present for the discussion and provided information and details specific to the proposed project. They were advised to make a formal submittal for review by the group.

5. OLD BUSINESS

5.1 SPR 2018-07 Mocama Brewery

Applicant was not present. The committee discussed the outstanding issues and will continue to work with the applicant for final approval.

6. DEVELOPMENT INQUIRIES FROM APPLICANTS NOT ON THE AGENDA – DISCUSSION ONLY, NO ACTION TO BE TAKEN.

7. COMMITTEE BUSINESS

7.1 Distribute: SPR 2018-08 Townies Pizza, TJ Courson Road

7.2 Distribute: SPR 2018-09 WestRock Drop Yard, 600 N. 8th Street

7.3 Distribute: SPR 2018-05 Skyway Amelia, Amelia Road & Amelia Island Parkway – 2nd Review

8. ADJOURNMENT – 9:52 AM



**MINUTES
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
SEPTEMBER 27, 2018**

1. CALL TO ORDER – 9:01 AM

2. ROLL CALL

MEMBERS PRESENT:

Judd Allen, Building	Rex Lester, Streets
Jacob Platt, Planning	John Mandrick, Utilities
Michelle Forstrom, Code Enforcement	Andre Desilet, Stormwater
Jason Higginbotham, Fire	

Sal Cumella, Historic Preservation & David Holley, Trees arrived at 9:10 AM.

3. DISCUSSION ITEMS

3.1 Suwanee & Altoona Avenue Right of Way Improvements - Access to lots 9-12 and 22-26 of the Seaview Subdivision.

Mr. Jeff Hanes, representative for lots 9-12, was present to provide information and details specific to the proposed project.

4. DEVELOPMENT INQUIRIES FROM APPLICANTS NOT ON THE AGENDA – DISCUSSION ONLY, NO ACTION TO BE TAKEN.

Paula and Laneya Warren, 15 N. 4th Street, joined the group to discuss a proposed project of converting a gift shop to a 1 bedroom Bed & Breakfast with an abbreviated breakfast.

The group indicated that the project would trigger a change of use. The applicant must obtain an LBTR and the 3 separate meters must be consolidated to 1 service box.

5. COMMITTEE BUSINESS

Mike Antonopoulos joined the committee to briefly discuss crosswalks and sidewalks in Amelia Park.

The committee discussed right of way standards.

6. ADJOURNMENT – 10:02 AM



**MINUTES
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
NOVEMBER 8, 2018**

1. CALL TO ORDER – 9:07 AM

2. ROLL CALL

MEMBERS PRESENT:

Donald Kukla, Building	Sal Cumella, Historic Preservation
Kelly Gibson, Planning	John Mandrick, Streets
Michelle Forstrom, Code Enforcement	Jason Higginbotham, Fire
Andre Desilet, Stormwater	

3. APPROVAL OF MINUTES

- 3.1 October 11, 2018 Regular Meeting
- 3.2 October 25, 2018 Regular Meeting

ACTION TAKEN: A motion was made by Member Forstrom, seconded by Member Desilet, to approve all minutes as presented.

Motion passed by unanimous voice vote.

4. DISCUSSION ITEMS

- 4.1 Pre-Application Discussion: PIN# 00-00-30-0800-0023-0040 South 8th Street. A proposed two tenant restaurant space with drive-thru lane. This property is zoned C-2 General Commercial.**

Lance Cooper, representative to the applicant, joined the committee and provided information and details specific to the proposed project.

The committee provided feedback and advised the applicant to return for site plan review.

5. DEVELOPMENT INQUIRIES FROM APPLICANTS NOT ON THE AGENDA – DISCUSSION ONLY, NO ACTION TO BE TAKEN.

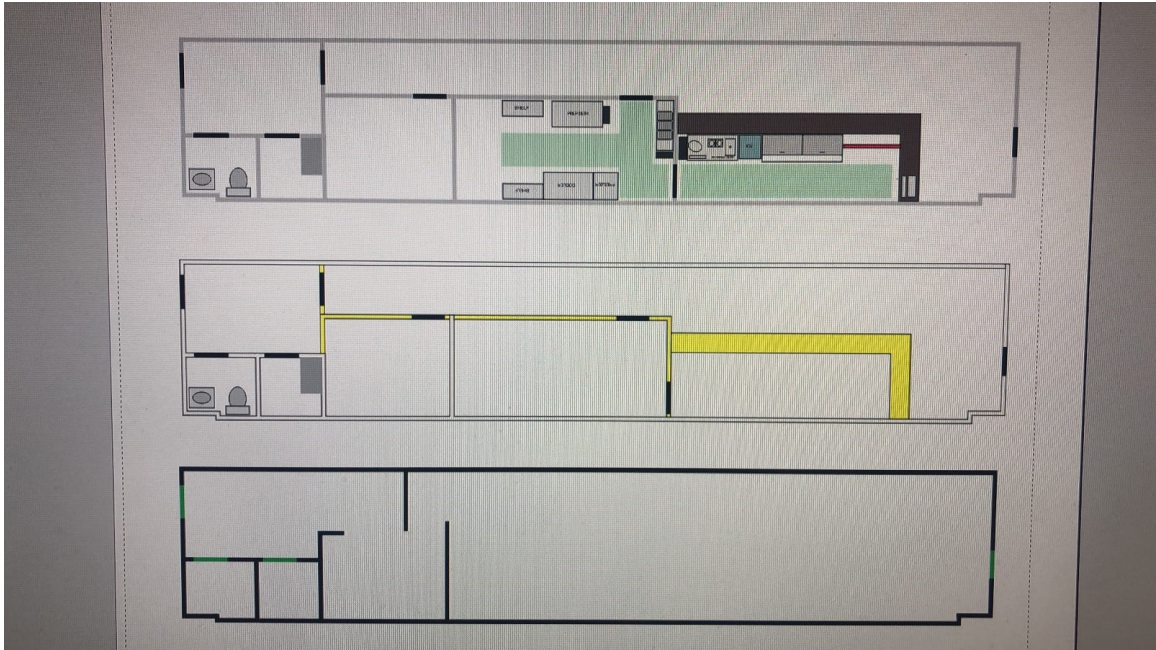
Sean Watkins joined the group to discuss a change of use from Beauty Salon to Mixed Martial Arts studio in Amelia Plaza and was advised to move forward to permitting.

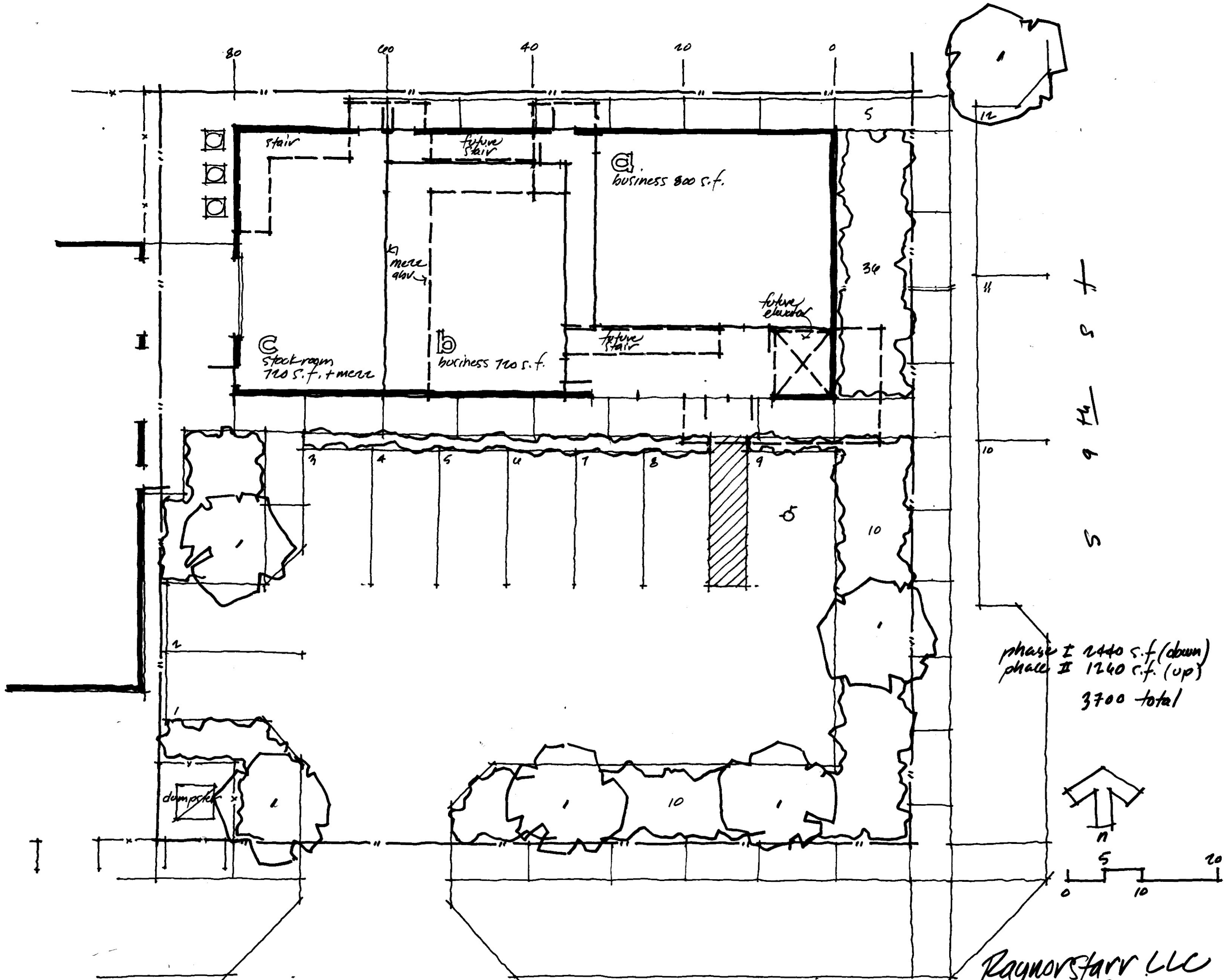
6. COMMITTEE BUSINESS

- 6.1 Roadway Standards**

The committee briefly discussed Roadway Standards and moving Old Town utilities underground. The November 22 Regular Meeting of the TRC was discussed and cancelled.

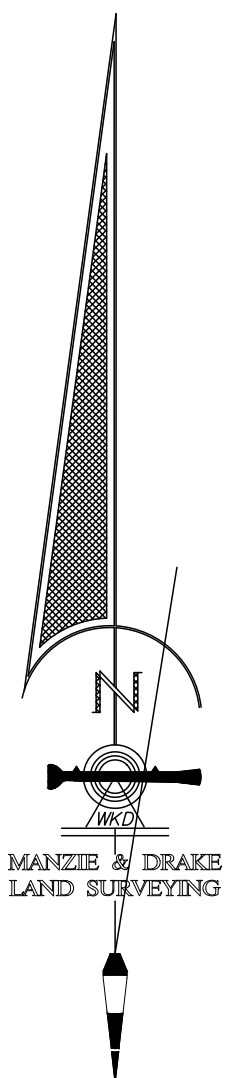
7. ADJOURNMENT – 10:49 AM





d a + e s +

Raynorstarr LLC
 404 + 944 St. South
 Fernandina Beach, FL
 contact as per inv. 11/14-18



K				Scale: 1" = 20'	Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034	G & A[®] GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819	BENT WING FLIGHT SERVICES	AIRPORT HANGARS	GEOMETRY PLAN	Registered Professional	Sheet No. GE-2
I			Project Mgr: AG	<u>2</u> of <u>2</u>							
H			Designed by: AG	Issue Date NOVEMBER 29, 2018							
F			Drawn by: MP/SS	Project No. 18-10.19							
E			QA/QC: NG								
D											
C											
B											
A	SS	11-29-18	DEVELOPMENT - CLIENT REVIEW								
No	By	Date	Revision								

ENGINEERING PLANS

FOR

FERNANDINA BEACH CONDOMINIUMS
1015 ATLANTIC AVENUE

FOR

A.I. PARTNERS, LLC

GILLETTE & ASSOCIATES, INC.

CIVIL & ENVIRONMENTAL ENGINEERING
MECHANICAL & STRUCTURAL ENGINEERING
PERMITTING & CONSTRUCTION MANAGEMENT

20 South 4th Street Amelia Island, Florida 32034
Phone: 904/261-8819

G & A

GILLETTE & ASSOCIATES, INC

20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034

PHONE: (904) 261-8819

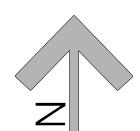
Issue Date:
APRIL 20, 2018
Revision:
DECEMBER 3, 2018

FERNANDINA BEACH CONDOMINIUMS

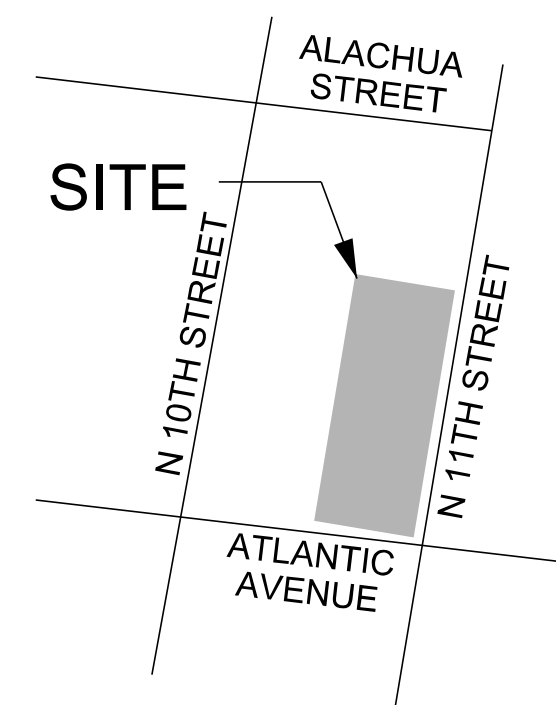
FOR

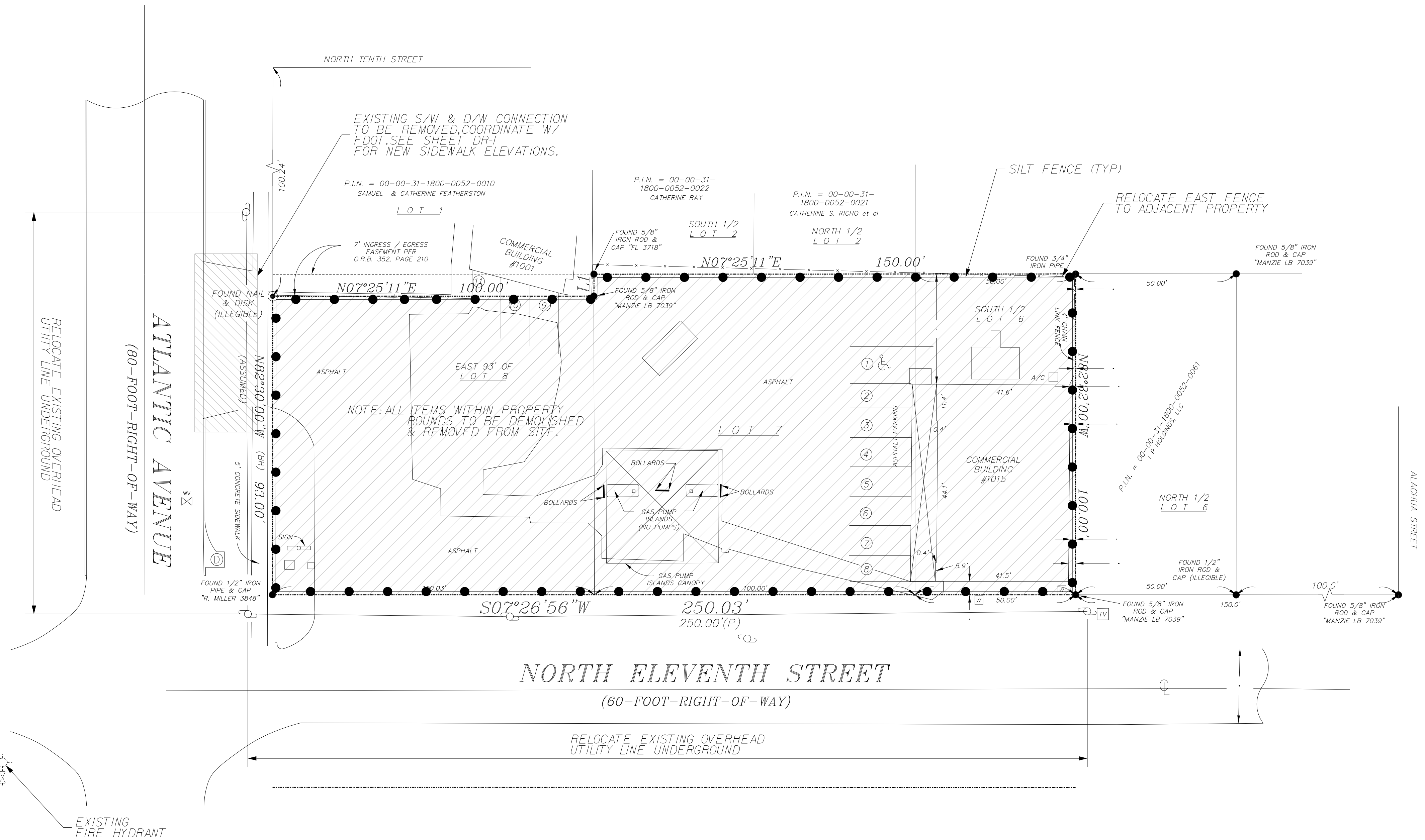
A.I. PARTNERS, LLC

RELEASED FOR CONSTRUCTION BY



SITE LOCATION MAP

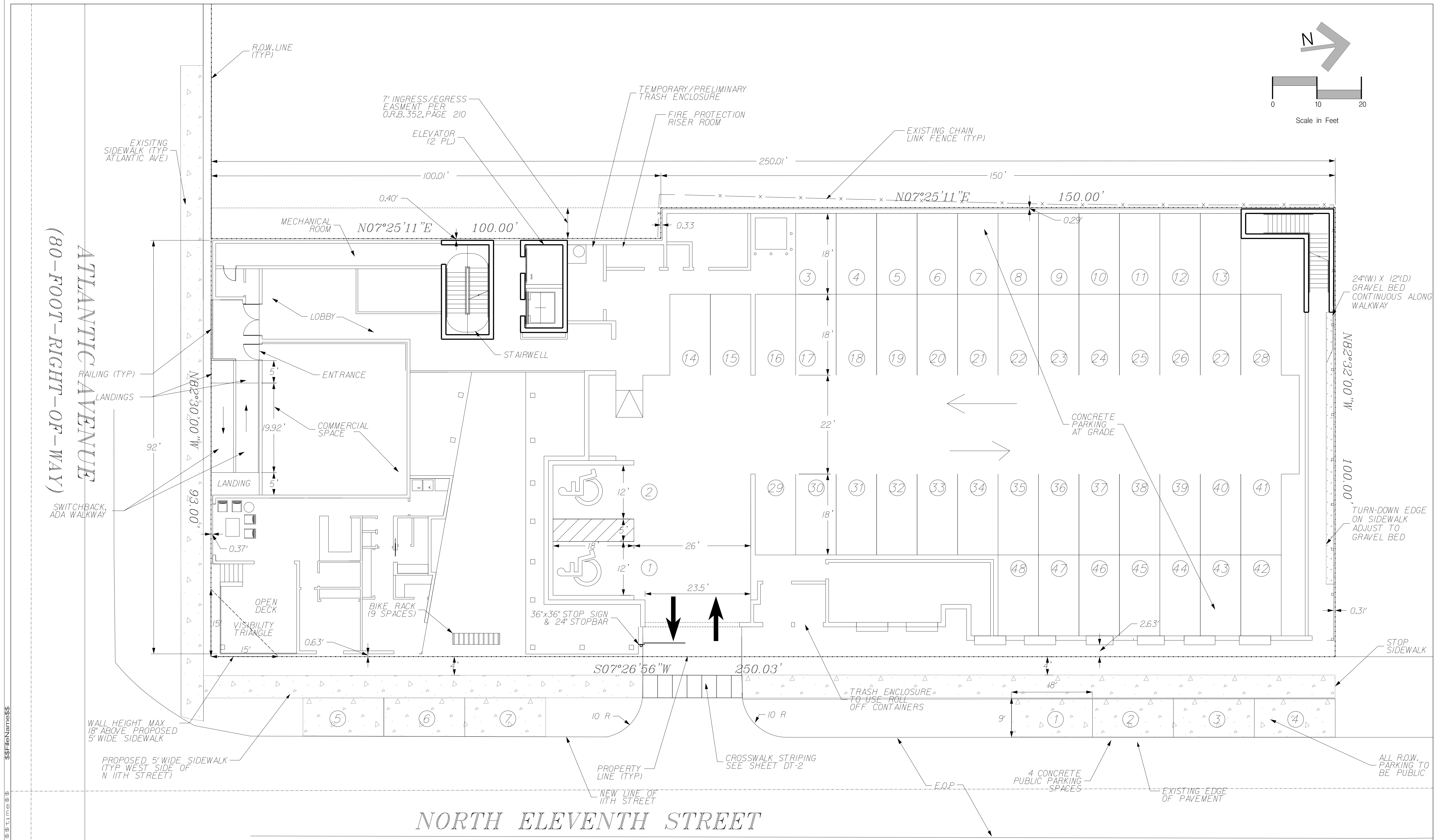




11						
10						
9						
8						
7						
6						
5	MP	12/3/18		Revised	Per	TRC comments
4	LA	11/2/18		Revised	Per	TRC comments
3	MP	9/26/18		Revised	Per	TRC comments
2	MP	6/28/18		Revised	Per	TRC comments
1	MP	4/20/18				TRC Submittal
No.	By	Date		Revision		

Scale:		
Project Mgr: AG		Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034
Designed by: AG		
Drawn by: MP		Certificate of Authorization No. 9332
QA/QC: BM		DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

A.J. PARTNERS, LLC



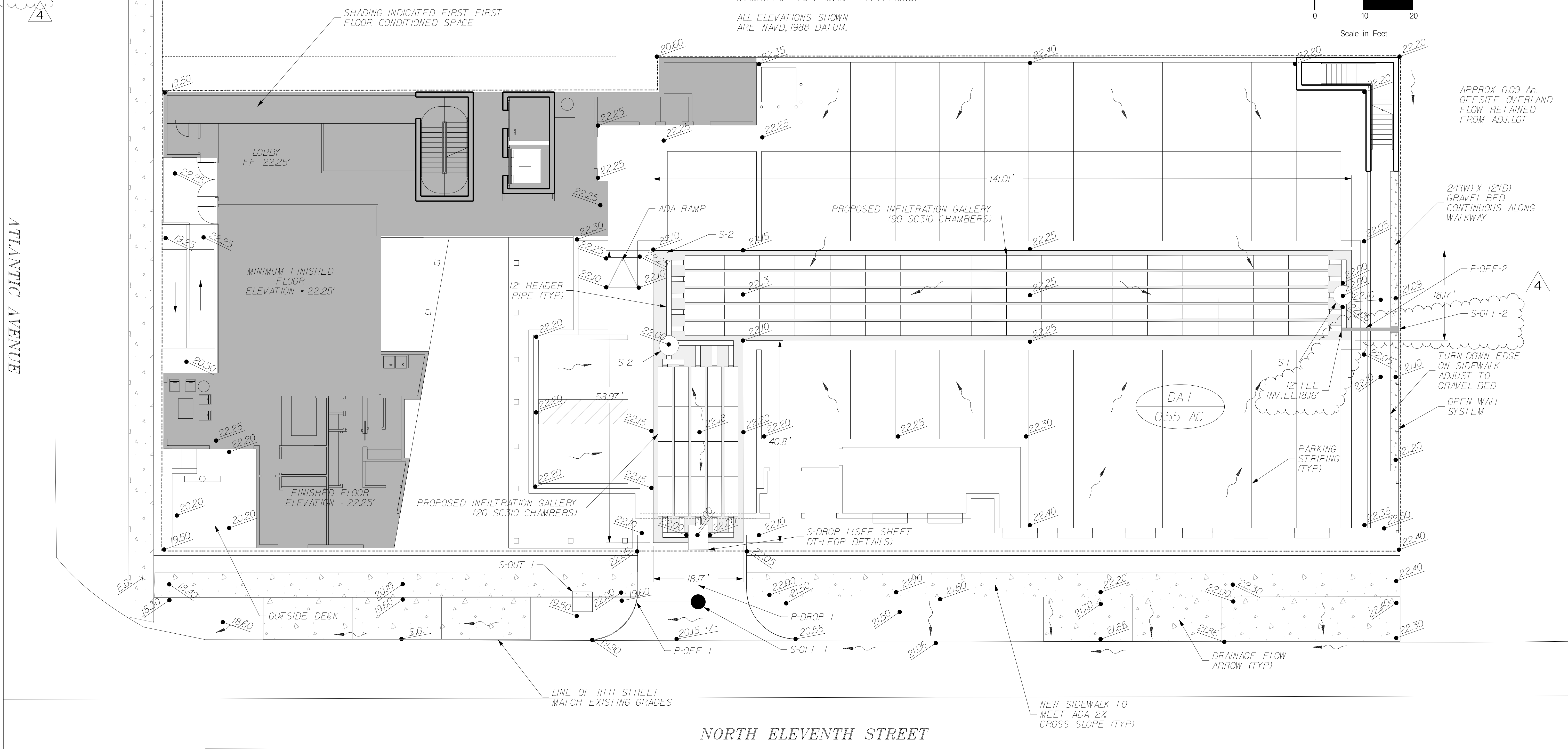
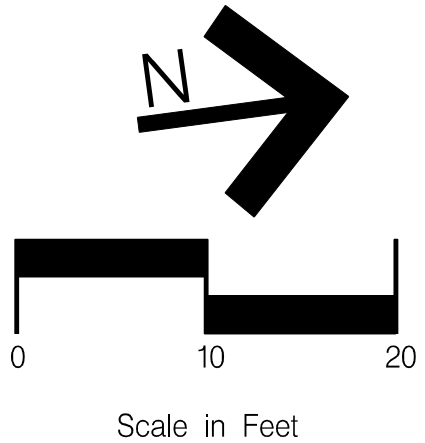
11					Scale: 1" = 10'		G & A⁺ GILLETTE & ASSOCIATES, INC 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819	FERNANDINA BEACH CONDOMINIUMS	A.J.PARTNERS,LLC	GEOMETRY PLAN	Sheet No. GE-2
10						6 of 12					
9						Issue Date October 31,2018					
8					Project Mgr: AG						Project No. 18-10,XX
7					Designed by: AG						
6											
5	MP	12/3/2018	Revised	Per TRC comments							
4	LA	10/31/2018	Revised	Per TRC comments	Drawn by: MP	Certificate of Authorization No. 9332					
3	MP	9/26/18	Revised	Per TRC comments							
2	MP	6/28/18	Revised	Per TRC comments	QA/QC: BM	DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"					
1	MP	4/20/18		TRC Submittal							
No.	By	Date		Revision							Registered Professional

STRUCTURE TABLE							PIPE TABLE						
NAME	TYPE	TOP/ INLET ELEV.	INVERT ELEVATION				NAME	FROM	TO	SIZE	LENGTH	MATERIAL	
			N	S	E	W							
S-1	STORMTECH INLET	22.00	18.16	—	18.16	18.16	P-DROP 1	S-DROP 1	S-OFF 1	18"	9'	RCP	
S-2	STORMTECH INLET	22.00	18.16	—	18.16	18.16	P-OFF 1	S-OFF 1	S-OUT 1	18"	20'	RCP	
S-DROP 1	DROP STRUCTURE	22.00	18.16	18.16	16.00	18.16	P-OFF 2	S-OFF 2	DR. GAL.	12"	10'	RCP	
S-OFF 1	MANHOLE	20.50	—	15.50	—	15.50							
S-OUT 1	TYPE C INLET	19.50	15.50	—	—	—	4						
S-OFF 2	TYPE C INLET	21.10	—	18.20	—	—							

NOTES:

ALL ROOF DRAINS AND ANCILLARY DRAINS TO CONNECT TO S-1 OR S-2 (ARCHITECT TO PROVIDE ELEVATIONS)

ALL ELEVATIONS SHOWN ARE NAVD, 1988 DATUM.



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2

1

No.

By

Date

MP

LA

MP

MP

MP

MP

Revised Per TRC comments

Revised Per TRC comments

Revised Per TRC comments

Revised Per TRC comments

Revised Per TRC comments

TRC Submittal

Revision

Scale: 1" = 10'

Project Mgr: AG

Designed by: AG

Drawn by: MP

QA/QC: BM

Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL 32034

Certificate of Authorization No. 9332

DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

G & A⁺
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

FERNANDINA BEACH
CONDOMINIUMS

AJ. PARTNERS, LLC

POST DEVELOPMENT
DRAINAGE PLAN

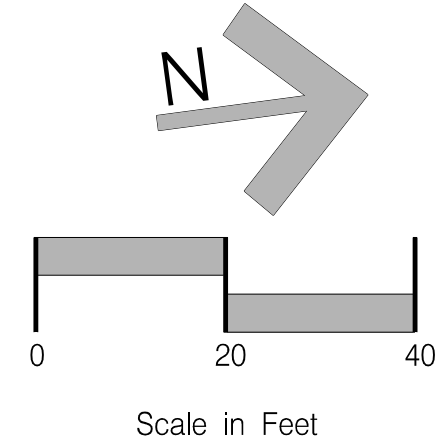
Registered Professional

Sheet No.
DR-1

7 of 12

Issue Date
November 2, 2018

Project No.
18-10-XX



Scale: 1" = 20'	
Project Mgr: AG	Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034
Designed by: AG	
Drawn by: MP	Certificate of Authorization No. 9332
QAQC: BM	DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PRECEDENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

UTILITY PLAN

Sheet No. <i>UT-1</i>
<i>8</i> of <i>12</i>
Issue Date <i>October 31, 2018</i>
Project No. <i>18-10.XX</i>

LANDSCAPING REQUIREMENTS:

10% OF LOT AREA TO BE LANDSCAPED
TOTAL LOT AREA: 24,192 SF
REQUIRED LANDSCAPE AREA: 2,419 SF
PROVIDED LANDSCAPE AREA
GROUND FLOOR: 1,865 SF
SECOND FLOOR: 1,137 SF
TOTAL PROVIDED AREA: 3,002 SF

1 TREE REQUIRED/2500 SF OF LOT AREA

24,192 SF/2,500 SF = 10 SHADE TREES REQUIRED
(USE OF UNDERSTORY TREES ARE ALLOWED IN
AREAS WHERE SHADE TREES ARE IMPRACTICAL)

2 SHADE TREES ON SITE

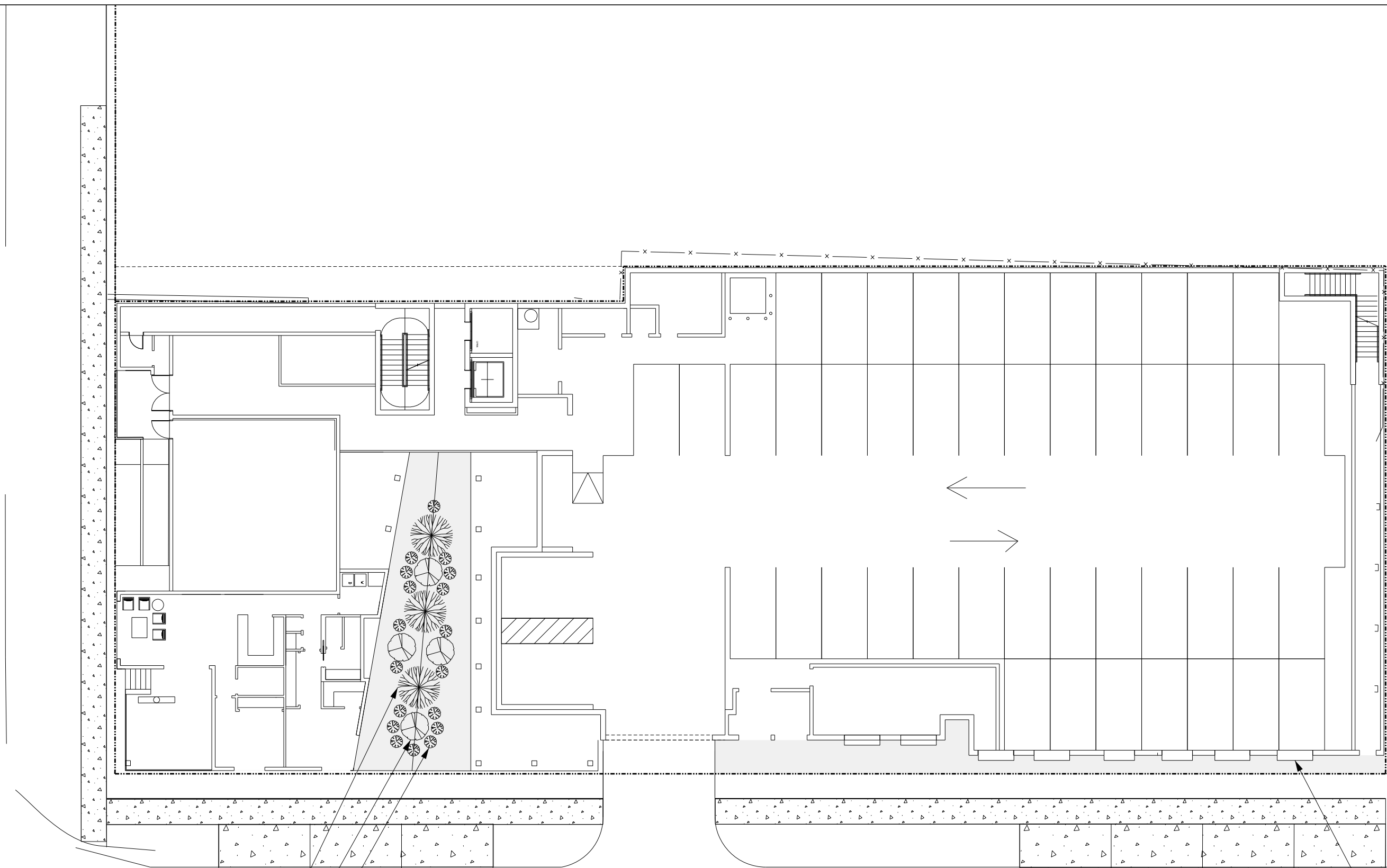
8 SHADE TREES OFFSITE ON C.O.F.B
PROPERTY/R.O.W.

DEVELOPER TO COORDINATE FINAL LOCATION OF
CANOPY TREES WITH THE CITY OF FERNANDINA
BEACH PER HARDSHIP RELIEF FOR TREE
PLANTINGS AGREEMENT WITH AI PARTNER,LL DATED
NOVEMBER 12,2018.

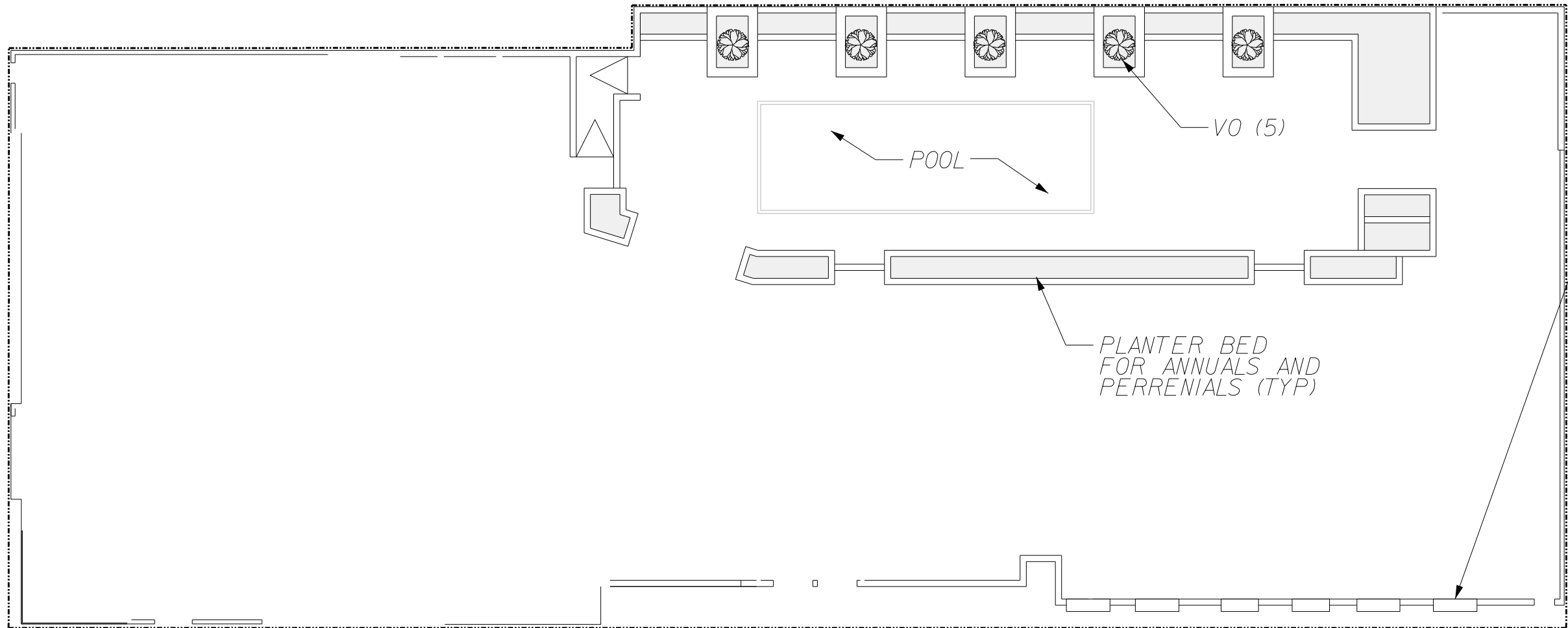
- NOTES:
- Plant materials shall meet the standards for Florida No.1 or better, as set out in Grades and Standards for Nursery Plants, Department of Agriculture, State of Florida.
 - Shrubs shall be at least eighteen (18) inches in height at the time of installation.
 - Shrubs shall be installed at least thirty-six (36) inches apart, measured on center.
 - Hedges shall be planted in double-staggered rows.
 - All landscaped areas shall be sodded or covered with ground cover.
 - Groundcover for all street parking islands shall be a mulched bed with muhly grasses.
 - All ROW trees shall be planted with a root barrier (see details this sheet).
 - Grassed areas shall be planted with species normally grown on permanent lawns in Nassau County. Grassed areas may be sodded, plugged, sprigged, or seeded, provided that full coverage shall be achieved within one (1) year after planting. Solid sod shall be used in swales or areas subject to erosion. Grass sod shall be clean and reasonably free of weeds and noxious pests or diseases. Grass seed shall be delivered to the job site in bags with Florida Department of Agriculture tags attached indicating the quality control program.
 - Installation and maintenance of all plantings shall be in accordance with articles 4.05.03(D) and 4.05.03(E) of the City of Fernandina Beach Land Development Code.
 - All signage for the project will be located on the building and will be in accordance with article 5.03.00.
 - All disturbed areas not depicting landscaping to be replanted with sand cordgrass and purple muhly grass.

INTERNAL PLANTING PLANT/MATERIAL SCHEDULE					
	QUANTITY	KEY	SPECIES	TYPE	SIZE/REMARKS
	4	PH	PHILODENDRON	SHRUB	18" MIN HT, 7 GAL.
			PHILODENDRON SELLOUM		
	5	VO	VIBURNUM OBARTUM	SHRUB	18" MIN HT, 3 GAL.
			WALTER'S VIBURNUM		
	19	DI	DIETES IRIDIOIDES	PERENNIAL	3 GAL.
			FORTNIGHT LILY		
	3	GL	GORDANIA LASIANTHUS	MEDIUM SHADE TREE	8' MIN HT 2.5" MIN CAL.
			LOBLOLLY BAY		
	8	LGW	LIVING GREEN WALL	VINE	2 - 3 GAL.
			CONFEREATE JASMINE		
	AREA	SOD	STENOTAPHRUM SECUNDATUM	GRASS	LAYER EVENLY WITHOUT GAPS
			ST. AUGUSTINE GRASS		

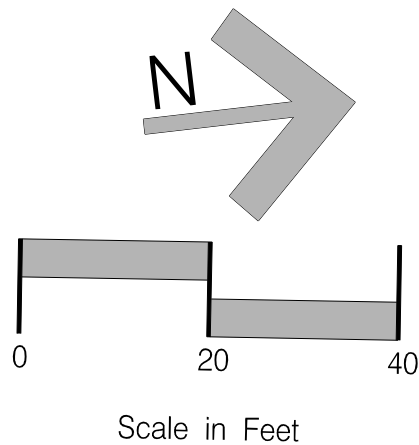
NOTE: DEVELOPER TO SECURE CONSTRUCTION AND
MAINTENANCE AGREEMENT W C.O.F.B. FOR ALL
OFFSITE PLANTINGS



NORTH ELEVENTH STREET
FIRST FLOOR LANDSCAPE PLAN



SECOND FLOOR LANDSCAPE PLAN
AROUND POOL DECK



Scale in Feet

LIVING GREEN WALL (TYP) (8)

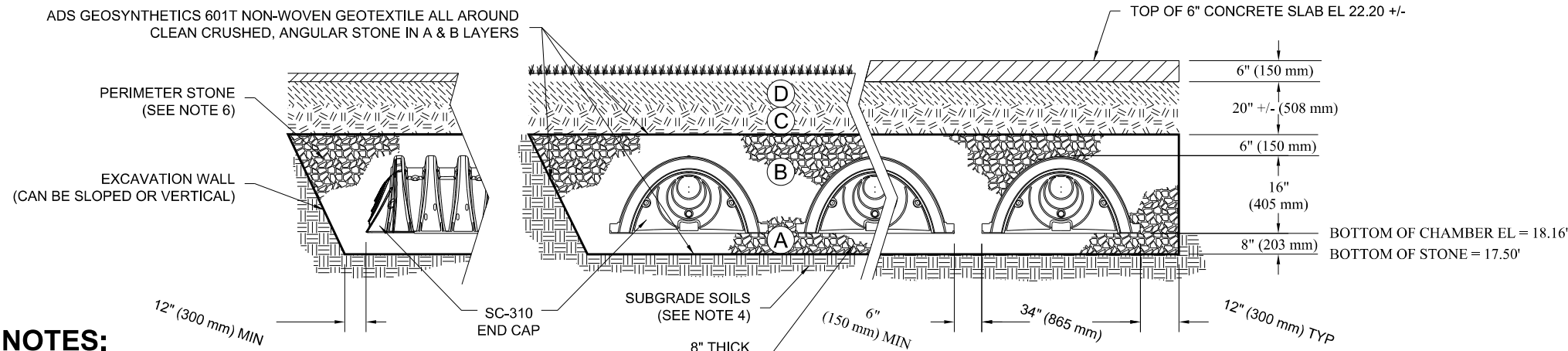
\$\$\$ times \$\$\$\$\$\$ File Name \$\$\$

11				Scale: 1" = 20"	G & A GILLETTE & ASSOCIATES, INC 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819 FERNANDINA BEACH CONDOMINIUMS AJ.PARTNERS,LLC LANDSCAPE PLAN	Registered Professional	Sheet No. LS-1 <u>9</u> of <u>12</u> Issue Date October 31, 2018 Project No. 18-10,XX
10				Project Mgr: AG			
9				Designed by: AG			
8				Drawn by: MP			
7				QAQC: BM			
6	MP	12/3/18	Revised Per TRC comments	DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"			
5	LA	10/31/18	Revised Per TRC comments				
4	MP	9/26/18	Revised Per TRC comments				
3	MP	6/28/18	Revised Per TRC comments				
2	MP	4/20/18	TRC Submittal				
1	No.	By	Date	Revision			

	MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER	ANY SOIL/ROCK MATERIALS; NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBBASE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 18" (450 mm) ABOVE THE TOP OF THE CHAMBERS. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145* A-1, A-2.4, A-3 OR AASHTO M433* 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 15" (380 mm) LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN), DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M433* 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M433* 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. **

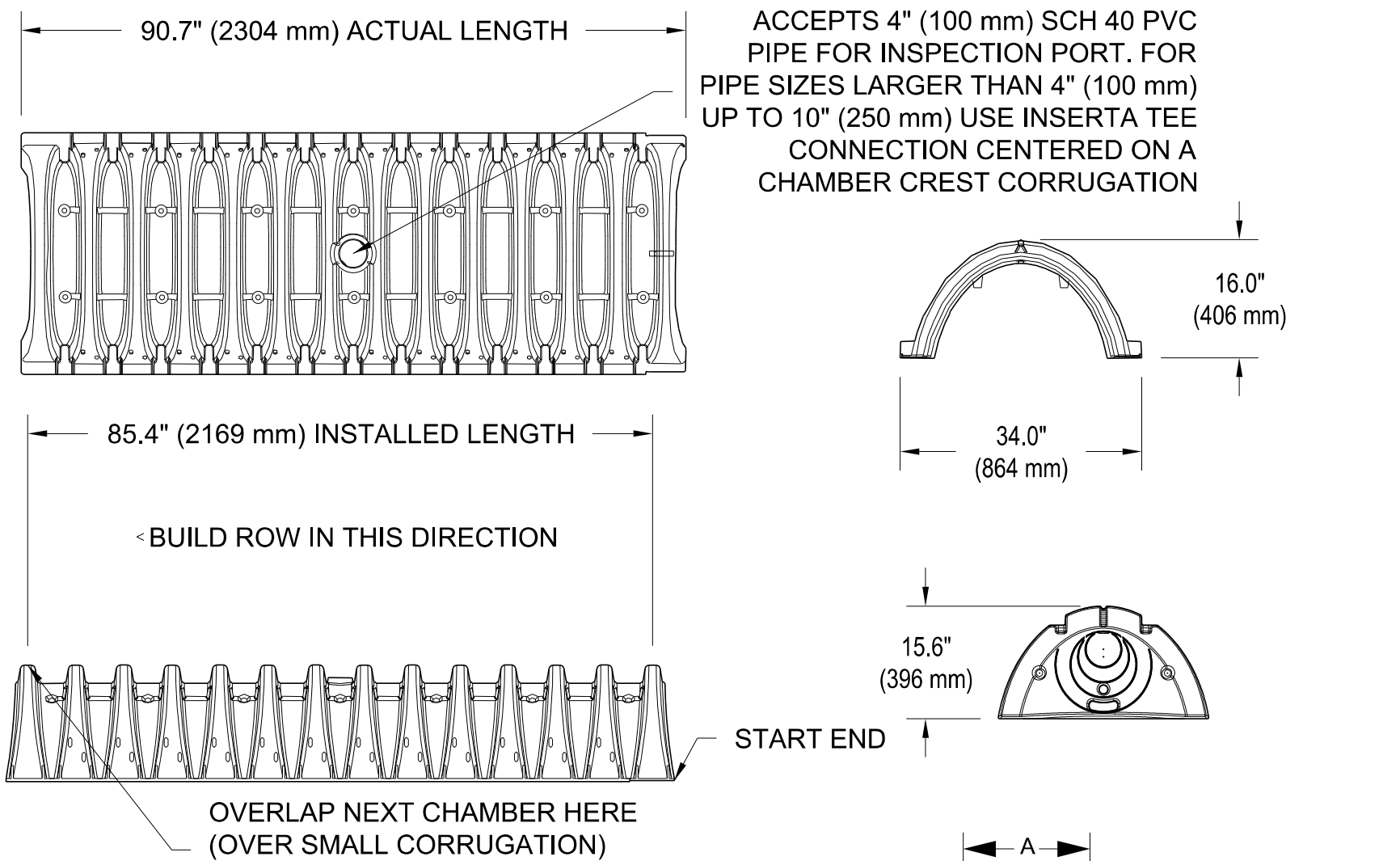
PLEASE NOTE:

1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR R LICATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERSAGES WITH A VIBRATORY COMPACTOR.
3. THE STORMTECH COMPACTION REQUIREMENTS ARE BASED ON THE ASSUMPTIONS OF STANDARD DESIGN AND CONDITIONS. A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTOR EQUIPMENT. FOR SPECIAL ROAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.



NOTES:

1. SC-310 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS", OR ASTM F2922 "STANDARD SPECIFICATION FOR POLYETHYLENE (PE) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-310 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. "ACCEPTABLE FILL MATERIALS" TABLE ABOVE PROVIDES MATERIAL LOCATIONS, DESCRIPTIONS, GRADATIONS, AND COMPACTION REQUIREMENTS FOR FOUNDATION, EMBEDMENT, AND FILL MATERIALS. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE UPGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEERS DISCRETION.



SIZE (W x H x INSTALLED LENGTH) :	34.0" x 16.0" x 85.4"
CHAMBER STORAGE:	14.7 CUBIC FEET
MINIMUM INSTALLED STORAGE:	31.0 CUBIC FEET
WEIGHT:	35 LBS.

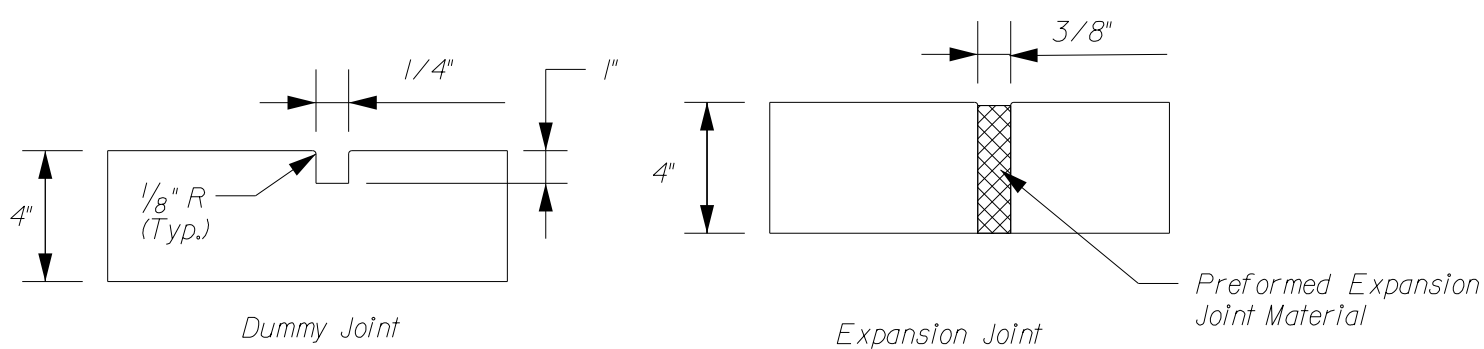
STUBS AT TOP OF END CAP
FOR PARTS NUMBERS
ENDING WITH "T"

STUBS AT BOTTOM OF END CAP
FOR PARTS NUMBERS
ENDING WITH "B"

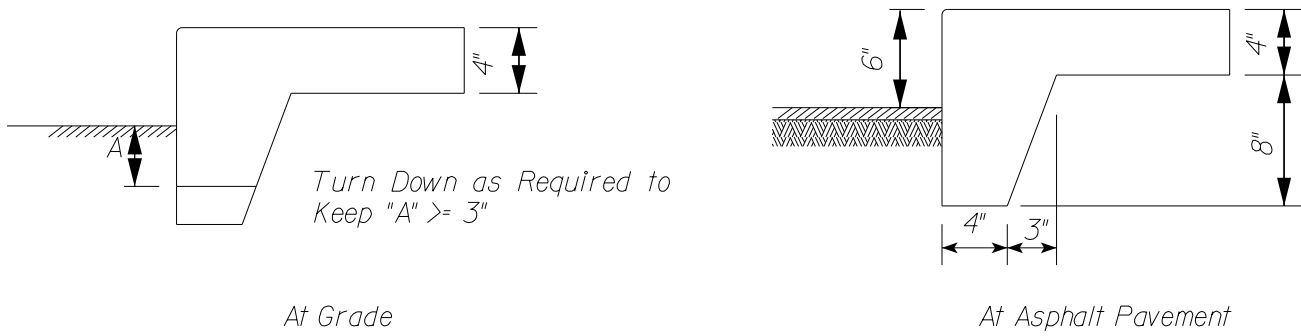
PART #	CHAMBER	PIPE SIZE	A	B
SC310EPE12B	SC-310	12 in (300 mm)	13.50in (343 mm)	- -

NOTE: ALL DIMENSIONS ARE NOMINAL

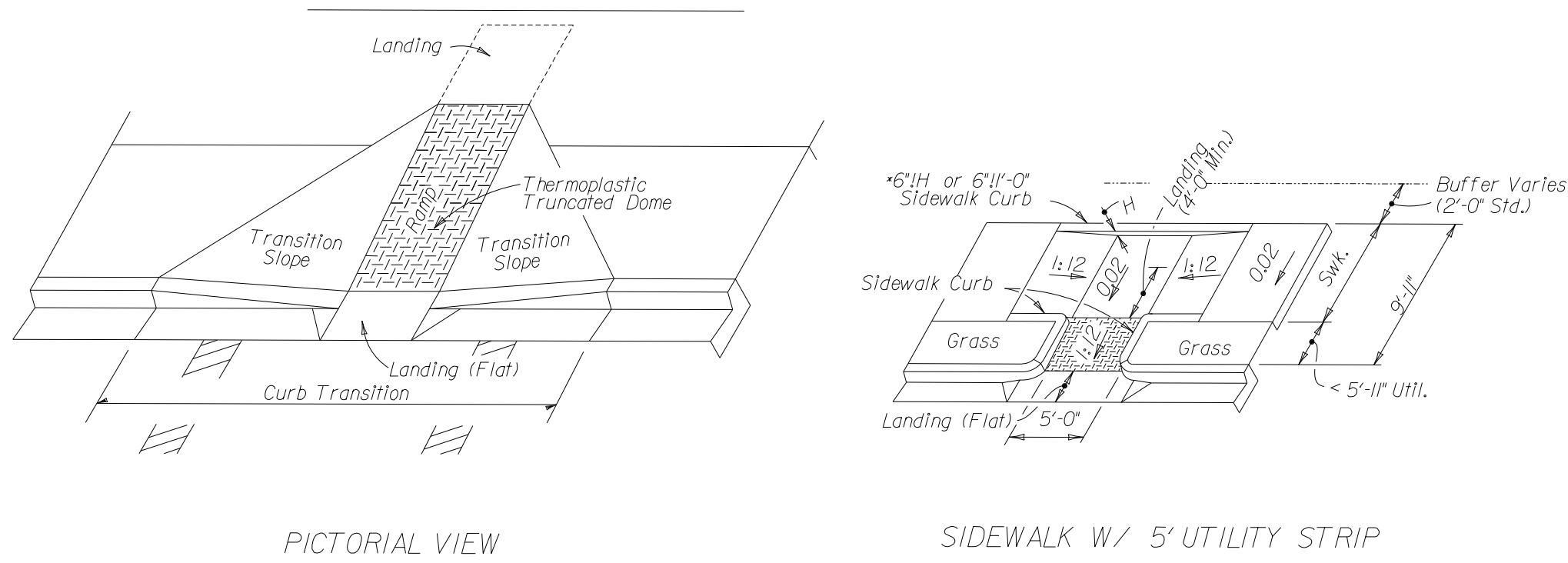
INFILTRATION GALLERY DETAILS



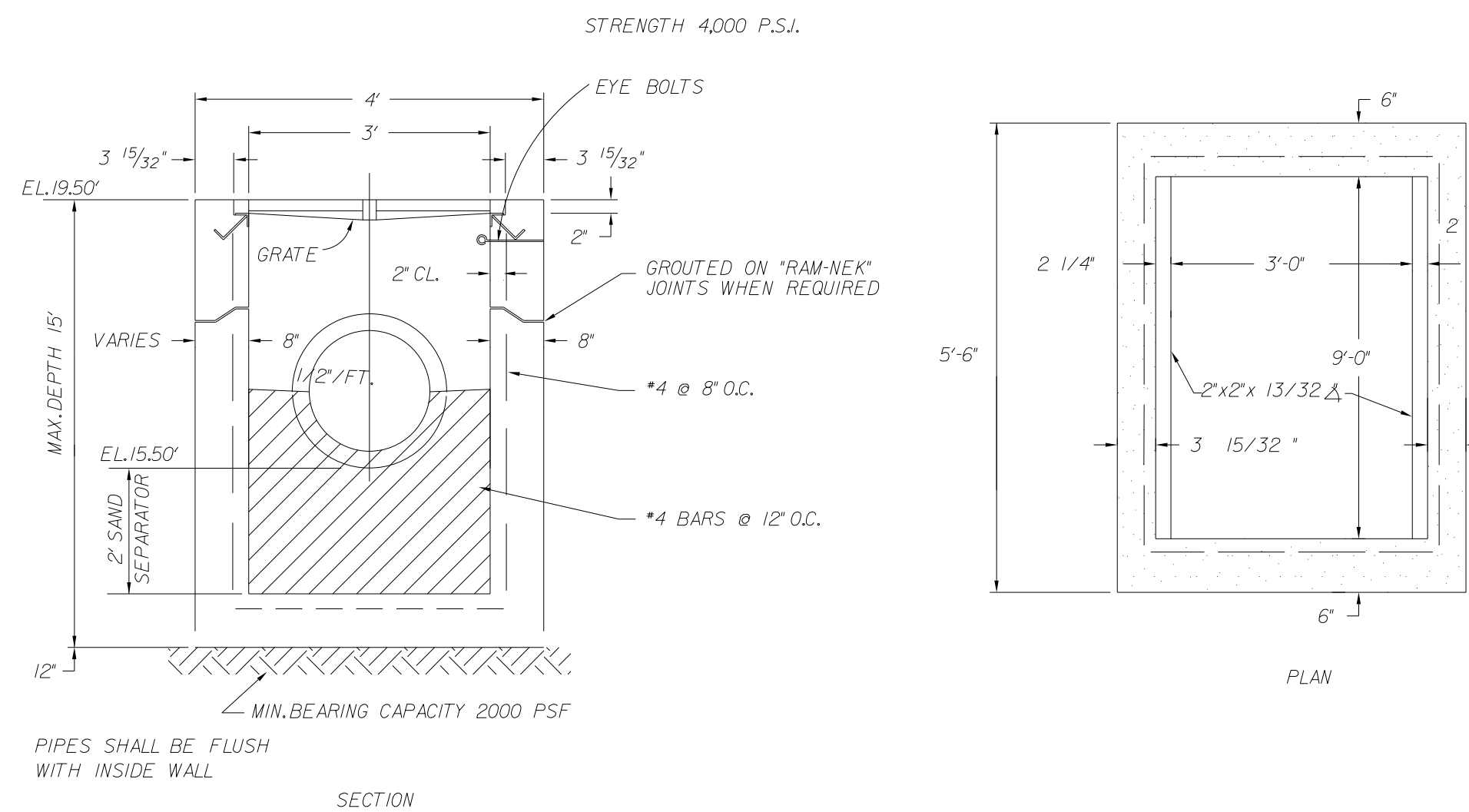
SIDEWALK JOINT DETAILS



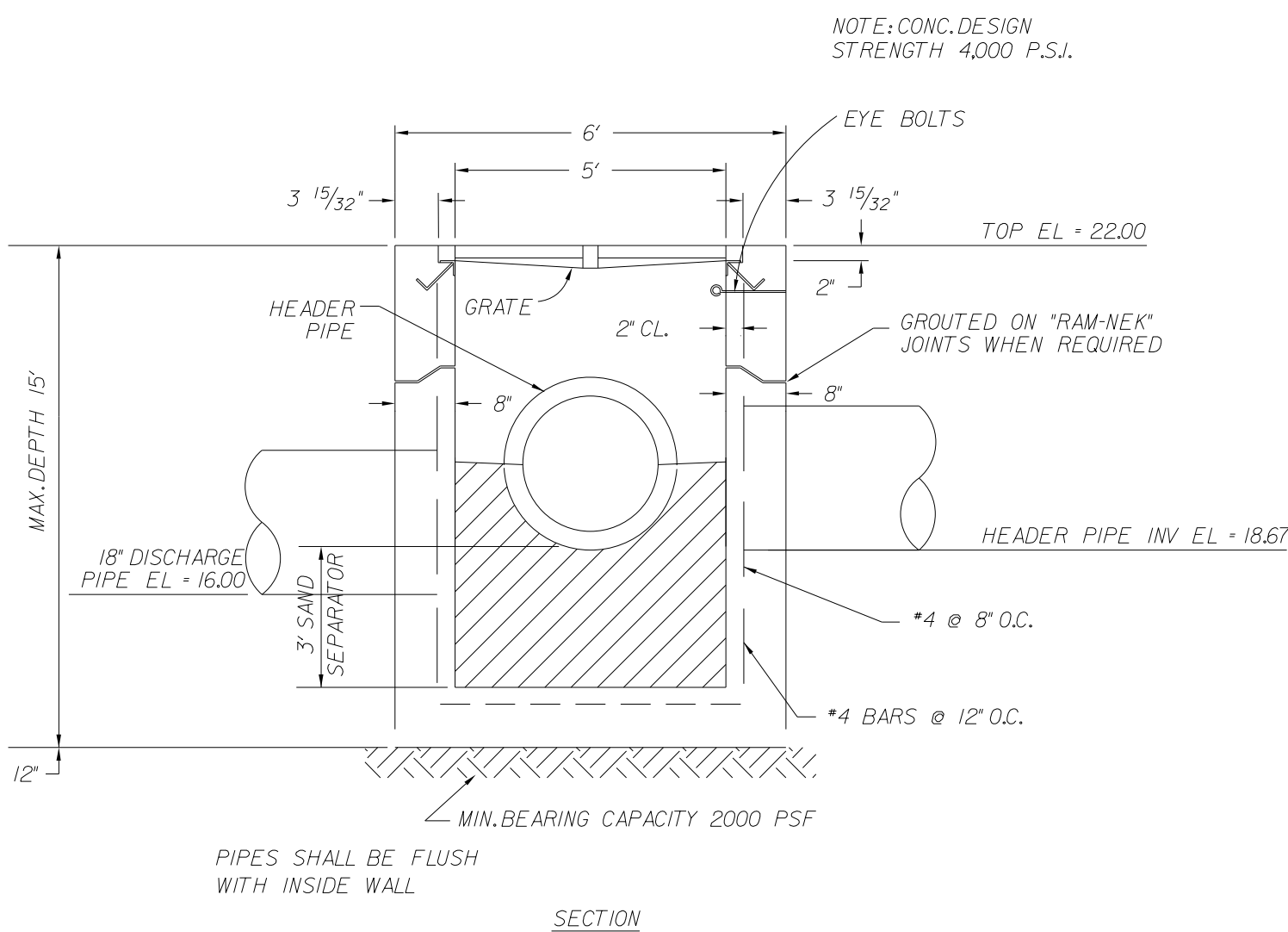
SIDEWALK TURNDOWN DETAILS



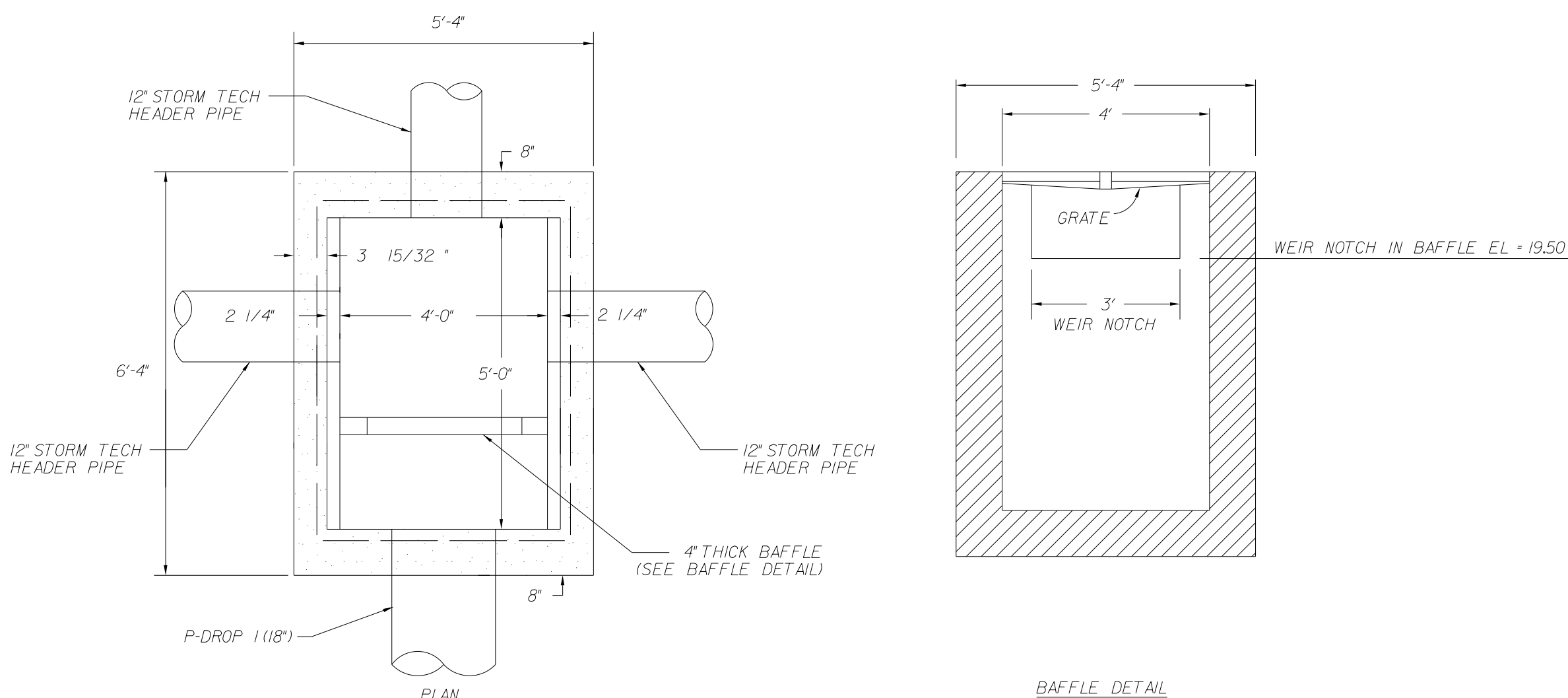
CURB CUT DESIGN



TYPE C INLET DETAIL (S-OUT-1)



OUTFALL STRUCTURE S-DROP 1 DETAIL



11				Scale: NONE	Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034 Certificate of Authorization No. 9332 DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE, DRAWING IS REDUCED IF LESS THAN 22' x 34'
10				Project Mgr: AG	
9				Designed by: AG	
8				Drawn by: MP	
7				QA/QC: BM	
6					
5	MP	12/3/18	Revised Per TRC comments		
4	LA	10/31/18	Revised Per TRC comments		
3	MP	9/26/18	Revised Per TRC comments		
2	MP	6/28/18	Revised Per TRC comments		
1	MP	4/20/18	TRC Submittal		
	No.	By	Date	Revision	

G & A[†]
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

FERNANDINA BEACH
CONDOMINIUMS

A.I. PARTNERS, LLC

MISCELLANEOUS DETAILS

Registered Professional

Sheet No.

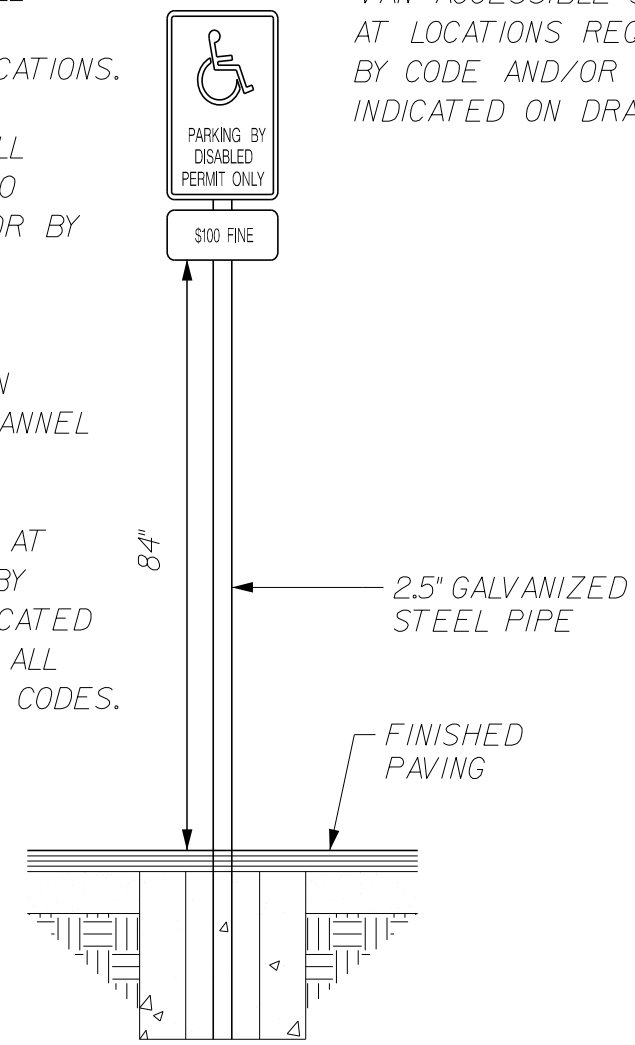
DT-1
10 of 12
 Issue Date
 October 31, 2018
 Project No.
 18-10,XX

ALL SIGNS SHOWN SHALL
CONFORM TO FEDERAL
ACCESSIBILITY SPECIFICATIONS.

ALL SIGNS SHOWN SHALL
BE REFLECTORIZED TO
SHOW THE SAME COLOR BY
NIGHT AS BY DAY.

ALL SIGNS SHALL BE
SECURELY MOUNTED ON
GALVANIZED STEEL CHANNEL
POSTS.

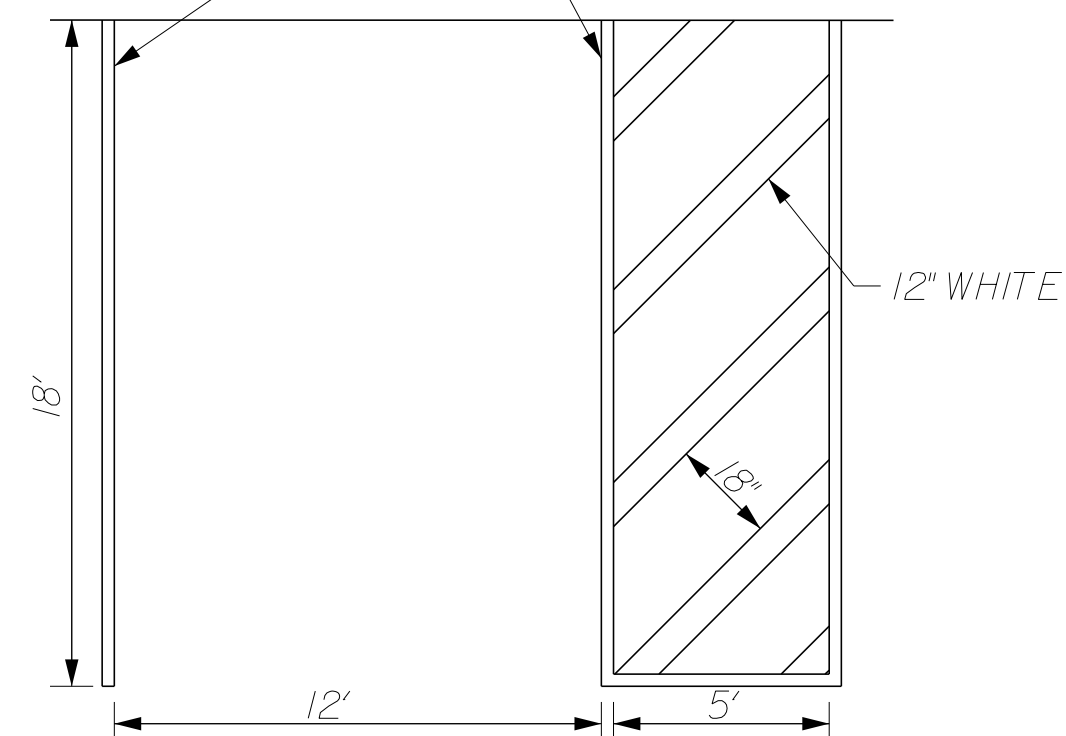
INSTALL QUANTITY AND AT
LOCATIONS REQUIRED BY
CODE AND/OR AS INDICATED
ON DRAWINGS. CONFIRM ALL
HANDICAPPED SIGNAGE CODES.



NOTE: PROVIDE ADDITIONAL
"VAN ACCESSIBLE" SIGNAGE
AT LOCATIONS REQUIRED
BY CODE AND/OR AS
INDICATED ON DRAWINGS.

ADA PARKING SIGN -
(SEE DETAIL
THIS SHEET)

4" STRIPES - PAINT
BLUE (15 MIL)
MINIMUM THICKNESS



*Stall dimension measured from
inside to inside striping*

Handicap Space and Access aisle
to be concrete pads

4" STRIPES - PAINT
YELLOW (15 MIL)
MINIMUM THICKNESS

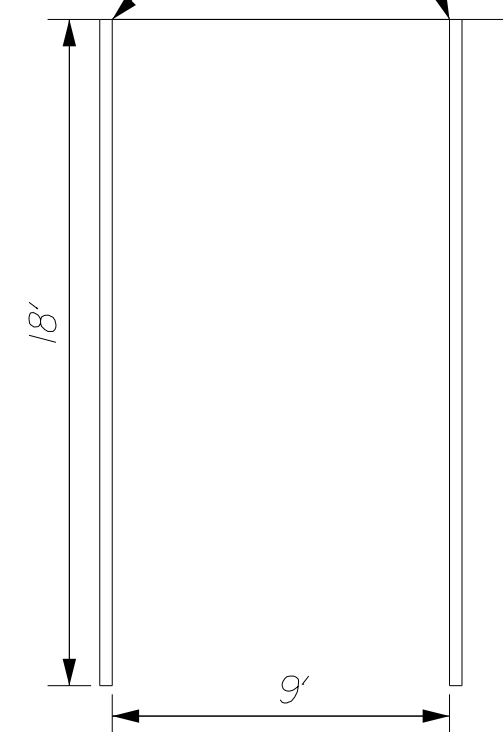
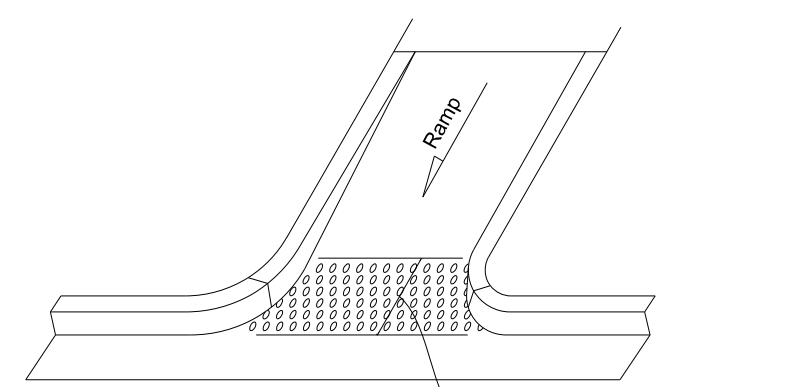
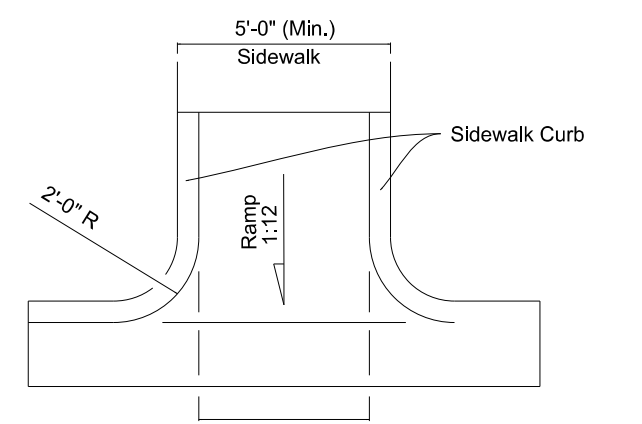


Diagram illustrating the dimensions and components of a vertical sign structure:

- NEW 5' SIDEWALK
- RAMP 1:12, 5'-0" WIDE MIN.
- 36" X 36" STOP SIGN
- 24" WIDE STOP BAR
- 60" MAX.
- 12" WHITE
- 24" WHITE
- RAMP 1:12, 5'-0" WIDE MIN.
- NEW 5' SIDEWALK
- PROPERTY LINE
- 2'-0" DETECTABLE WARNING
- SEE ADA CROSSWALK DETAIL
- 10'

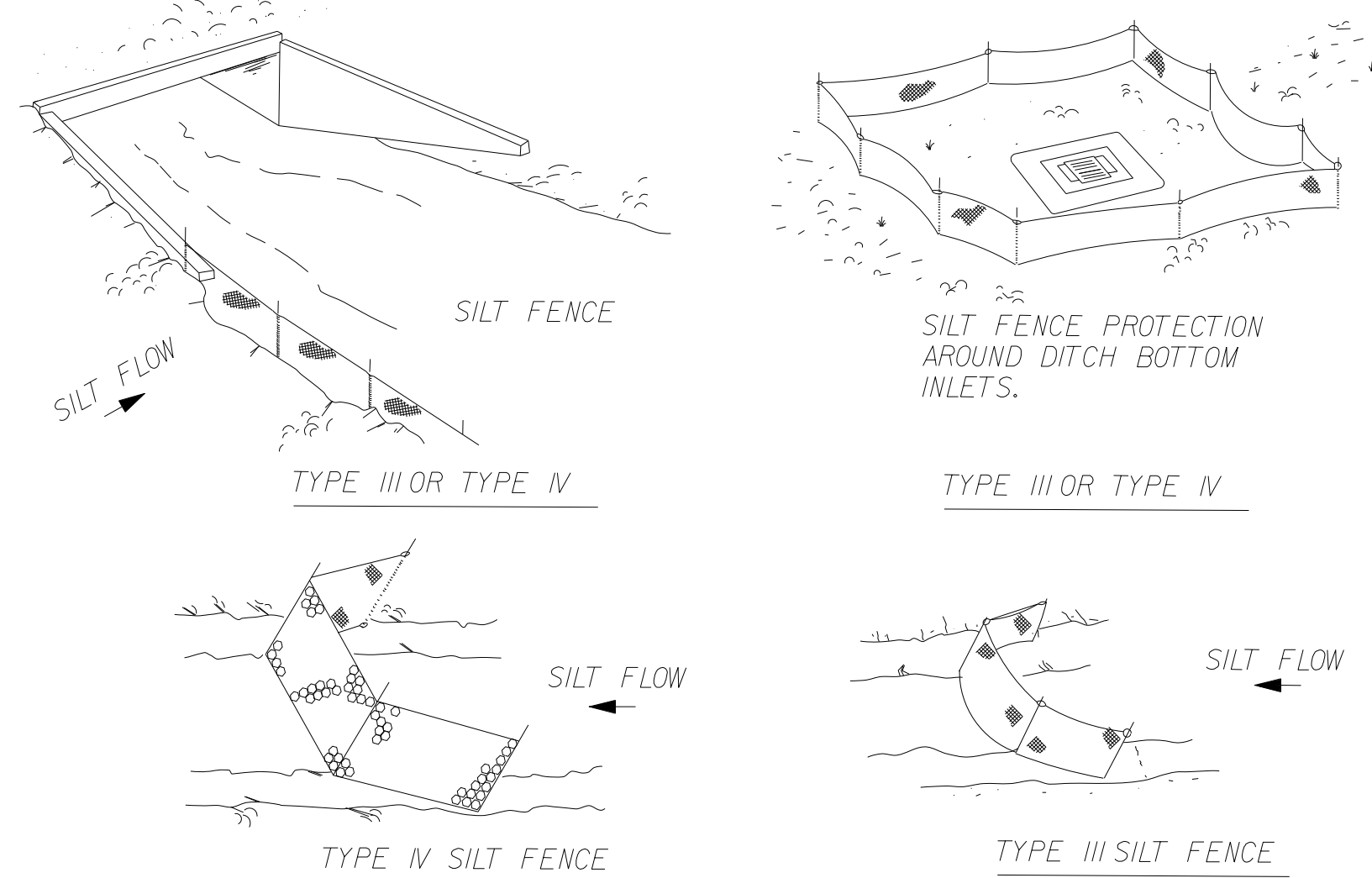


ISOMETRIC VIEW

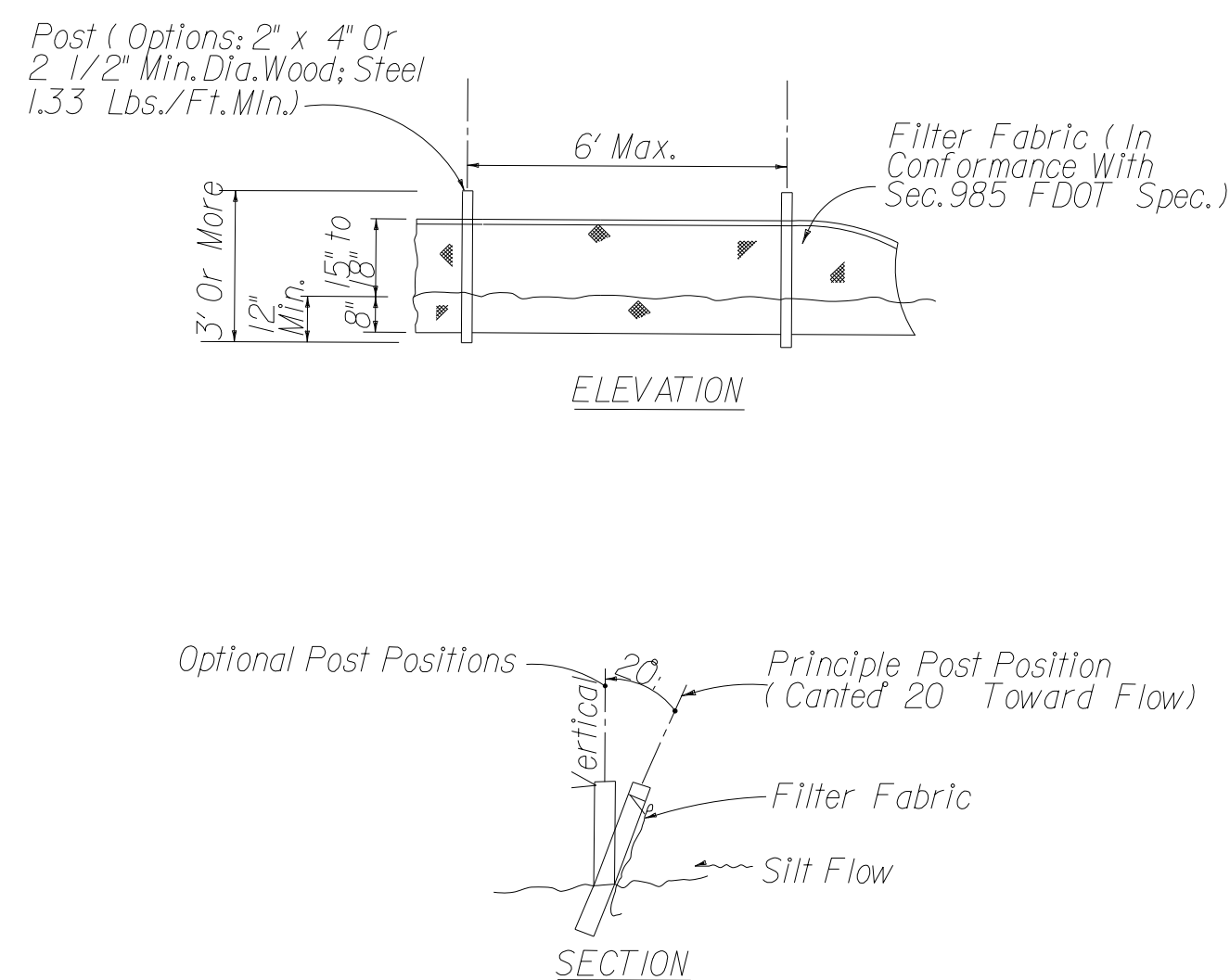


PLAN VIEW

ADA CROSSWALK
DETAIL CR-E



DO NOT DEPLOY IN A MANNER THAT SILT FENCES WILL ACT AS A DAM ACROSS PERMANENT FLOWING WATERCOURSES. SILT FENCES ARE TO BE USED AT UPLAND LOCATIONS AND TURBIDITY BARRIERS USED AT PERMANENT BODIES OF WATER.



6" HEADER AND DROP CURB

[illegible]

G & A[†]
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

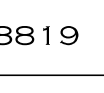
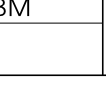
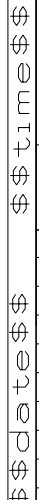
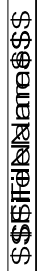
FERNANDINA BEACH
CONDOMINIUMS

A.I. PARTNERS, LLC

MISCELLANEOUS DETAILS

Registered Professional

Sheet No.	DT-2
Issue Date	11 of 12
Project No.	October 3, 2018 October 31, 2018
	18-10.XX



PERMIT SET
NOT FOR CONSTRUCTION

ENGINEERING PLANS

FOR

Amelia Road and Parkway

FOR

Skyway Amelia, LLC

GILLETTE & ASSOCIATES, INC.

CIVIL & ENVIRONMENTAL ENGINEERING
MECHANICAL & STRUCTURAL ENGINEERING
PERMITTING & CONSTRUCTION MANAGEMENT

20 SOUTH 4TH STREET AMELIA ISLAND, FLORIDA 32034
PHONE: 904/261-8819

G & A

ISSUE DATE: June 5, 2018

REVISION DATE: November 26, 2018

PERMITTING AGENCIES

CITY OF FERNANDINA BEACH: 904-310-3421
SJRWMD: 904-730-6270

UTILITY PROVIDERS

WATER/SEWER: CITY OF FERNANDINA BEACH
ELECTRIC: FLORIDA PUBLIC UTILITIES
CABLE: COMCAST
TELEPHONE: AT&T

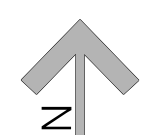
G&A Project
No.:17-10-23

Amelia Road and Parkway

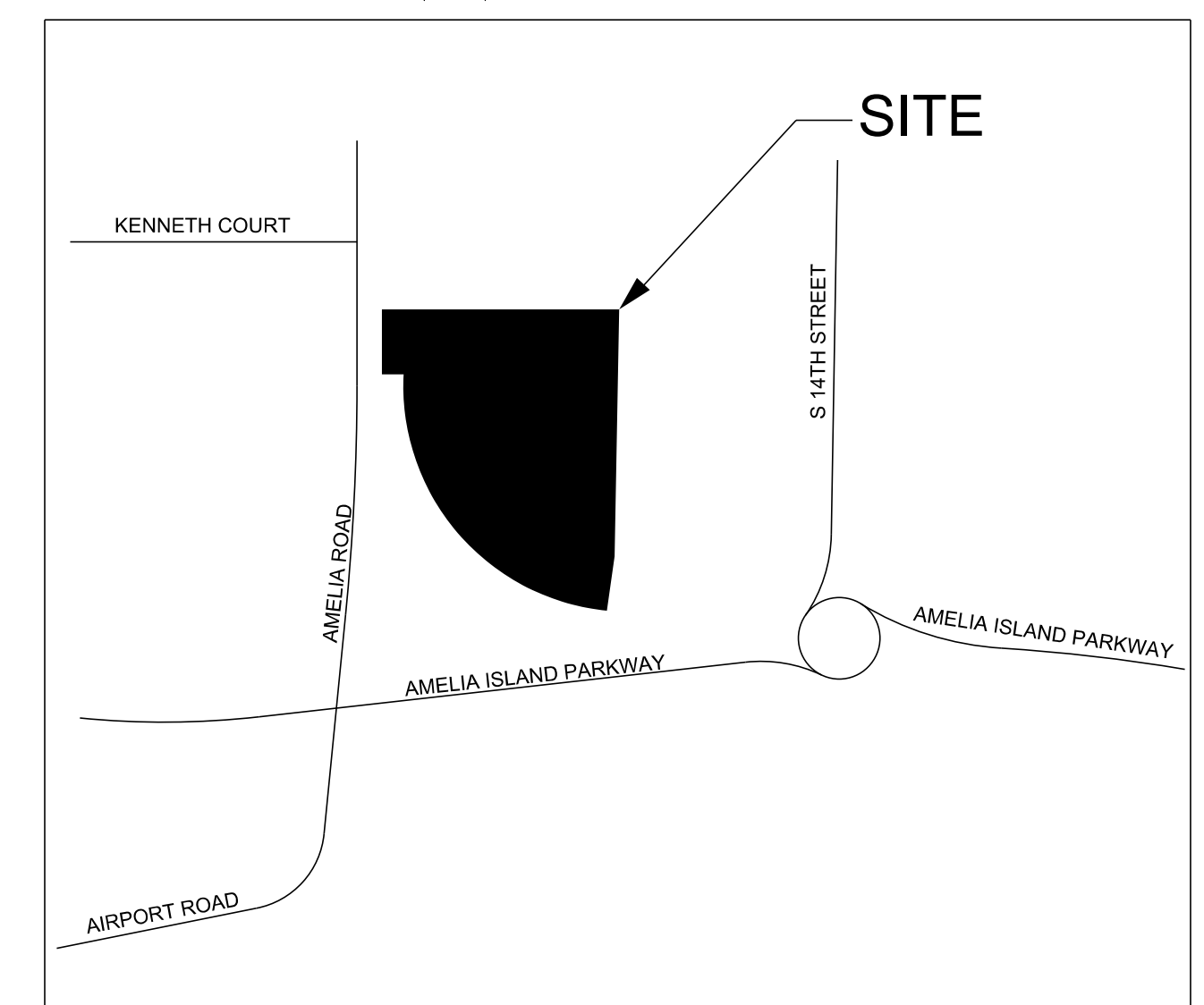
FOR

Dale Cole

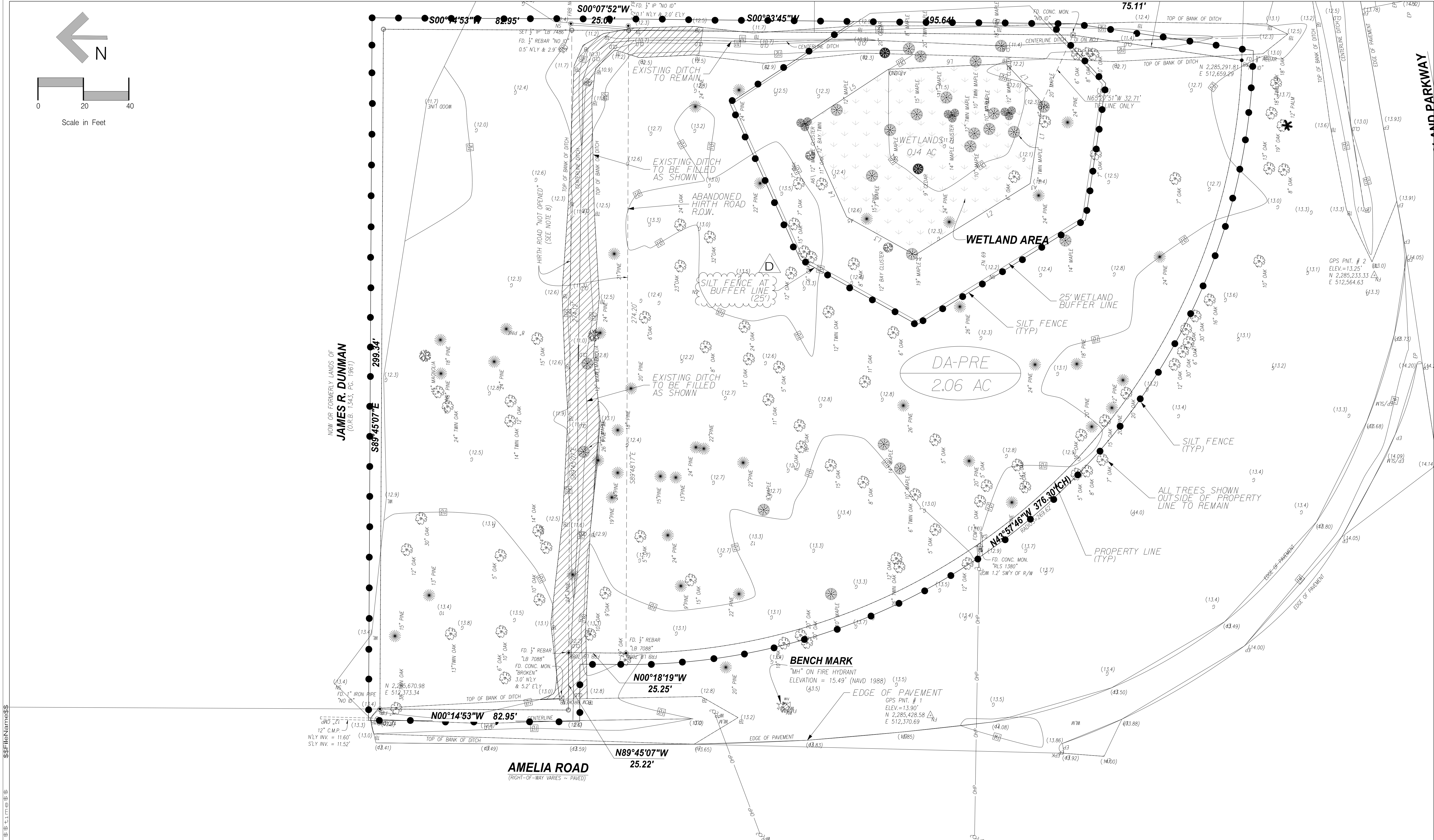
RELEASED FOR CONSTRUCTION BY



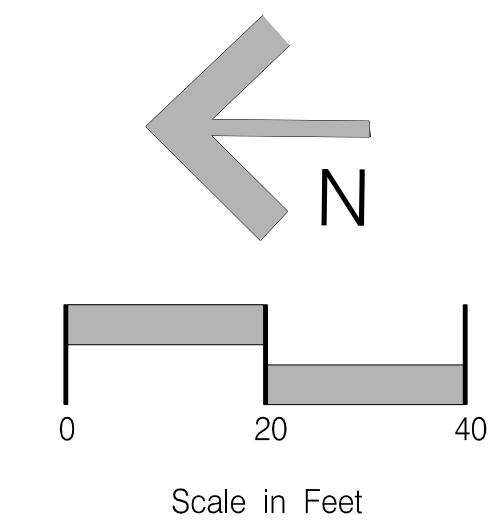
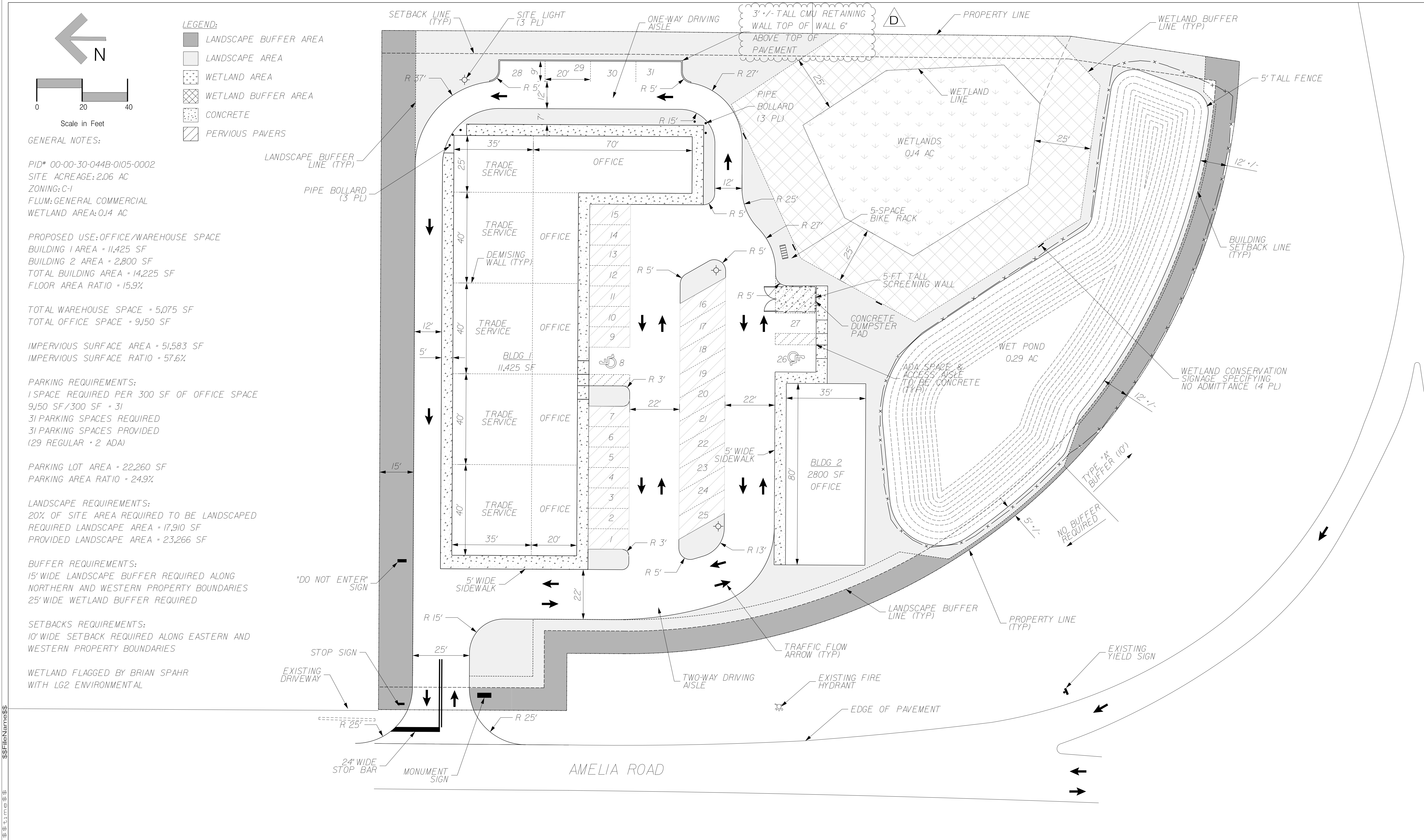
SITE LOCATION MAP



</



			Scale: 1" = 20'	G & A GILLETTE & ASSOCIATES, INC 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819	SKYWAY AMELIA, LLC	AMELIA ROAD AND PARKWAY	PREDEVELOPMENT PLAN	Registered Professional	Sheet No. PRE-1 3 of 11
			Project Mgr: AG						Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034
			Designed by: AG						
			Drawn by: MP	Certificate of Authorization No. 9332				Issue Date 6-5-18	
			QAQC: BM	DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"				Project No. 17-10-23	



- LEGEND:
- LANDSCAPE BUFFER AREA
 - LANDSCAPE AREA
 - WETLAND AREA
 - WETLAND BUFFER AREA
 - CONCRETE
 - PERVIOUS PAVERS

Scale in Feet

GENERAL NOTES:

PID# 00-00-30-044B-0105-0002
SITE ACREAGE: 2.06 AC
ZONING: C-1
FLUM: GENERAL COMMERCIAL
WETLAND AREA: 0.14 AC

PROPOSED USE: OFFICE/WAREHOUSE SPACE
BUILDING 1 AREA = 11,425 SF
BUILDING 2 AREA = 2,800 SF
TOTAL BUILDING AREA = 14,225 SF
FLOOR AREA RATIO = 15.9%

TOTAL WAREHOUSE SPACE = 5,075 SF
TOTAL OFFICE SPACE = 9,150 SF

IMPERVIOUS SURFACE AREA = 51,583 SF
IMPERVIOUS SURFACE RATIO = 57.6%

PARKING REQUIREMENTS:
1 SPACE REQUIRED PER 300 SF OF OFFICE SPACE
9,150 SF / 300 SF = 31
31 PARKING SPACES REQUIRED
31 PARKING SPACES PROVIDED
(29 REGULAR + 2 ADA)

PARKING LOT AREA = 22,260 SF
PARKING AREA RATIO = 24.9%

LANDSCAPE REQUIREMENTS:
20% OF SITE AREA REQUIRED TO BE LANDSCAPED
REQUIRED LANDSCAPE AREA = 17,910 SF
PROVIDED LANDSCAPE AREA = 23,266 SF

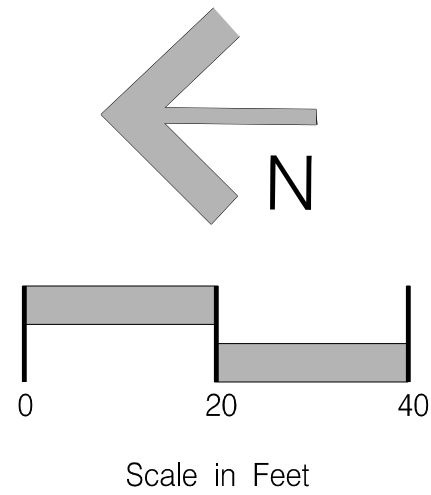
BUFFER REQUIREMENTS:
15' WIDE LANDSCAPE BUFFER REQUIRED ALONG
NORTHERN AND WESTERN PROPERTY BOUNDARIES
25' WIDE WETLAND BUFFER REQUIRED

SETBACKS REQUIREMENTS:
10' WIDE SETBACK REQUIRED ALONG EASTERN AND
WESTERN PROPERTY BOUNDARIES

WETLAND FLAGGED BY BRIAN SPAHR
WITH LG2 ENVIRONMENTAL

\$\$\$FileNames\$\$\$
\$\$\$Time\$\$\$
\$\$\$Date\$\$\$

				Scale: 1" = 20'												Sheet No. GE-1 4 of 11	
				Project Mgr: AG		Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034		G & A GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819		SKYWAY AMELIA, LLC		AMELIA ROAD AND PARKWAY		GEOMETRY PLAN		Issue Date 6-5-18	
				Designed by: AG												Project No. 17-10-23	
				Drawn by: MP		Certificate of Authorization No. 9332											
				QA/QC: BM		DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"											



STRUCTURE TABLE

NAME	TYPE	TOP/ INLET ELEV.	INVERT ELEVATION			
			N	S	E	W
S-1	TYPE C INLET	14.50	-	11.00	-	-
S-2	TYPE C INLET	14.50	10.66	-	-	10.41
S-3	TYPE C INLET	14.50	10.86	8.68	10.18	9.88
S-4	TYPE C INLET	14.50	-	-	-	11.00
S-5	TYPE C INLET	14.50	-	-	10.75	10.50
S-6	TYPE C INLET	14.50	-	9.83	10.35	-
S-7	TYPE C INLET	14.50	9.64	-	9.64	-
S-8	TYPE C INLET	14.50	10.86	-	10.00	9.54
S-9	TYPE C INLET	14.50	-	11.00	-	-
S-10	TYPE C INLET	14.50	-	11.00	-	-
S-11	MANHOLE	14.50	8.64	8.64	-	-
S-12	M.E.S.	-	8.59	-	-	-
S-OFF-1	M.E.S.	-	-	11.15	-	-
S-OFF-2	M.E.S.	-	11.05	-	-	-
S-OUT-1	DROP STRUC.	14.65	-	-	10.90	-
S-OUT-2	M.E.S.	-	-	-	-	10.70

PIPE TABLE

NAME	FROM	TO	SIZE	LENGTH	MATERIAL
P-1	S-1	S-2	15"	106'	HDPE
P-2	S-2	S-3	18"	87'	HDPE
P-3	S-3	S-11	36"	45'	HDPE
P-4	S-4	S-5	15"	76'	HDPE
P-5	S-5	S-6	18"	58'	HDPE
P-6	S-6	S-7	24"	123'	HDPE
P-7	S-7	S-8	24"	54'	HDPE
P-8	S-8	S-3	24"	68'	HDPE
P-9	S-9	S-3	15"	41'	HDPE
P-10	S-10	S-8	15"	41'	HDPE
P-11	S-11	S-12	36"	41'	HDPE
P-OFF-1	S-OFF-1	S-OFF-2	18"	43'	HDPE
P-OUT-1	S-OUT-1	S-OUT-2	18"	20'	HDPE

12' C.M.P.
NLY INV. = 11.60'
SLY INV. = 11.52'

AMELIA ROAD

EDGE OF PAVEMENT

K			
J			
I			
H			
G			
F			
E			
D	MP	11-26-18	Revised Per CDFB TRC Comments
C	MP	11-2-18	Revised Pond & Site Per SJRWMD Comments
B	MP	10-8-18	Revised Per SJRWMD & CDFB Comments
A	MP	6-5-18	Issue For TRC Review
No.	By	Date	Revision

Scale: 1" = 20'

Project Mgr: AG

Designed by: AG

Drawn by: MP

QAQC: BM

1" = 20'

Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL 32034

Certificate of Authorization No. 9332

DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

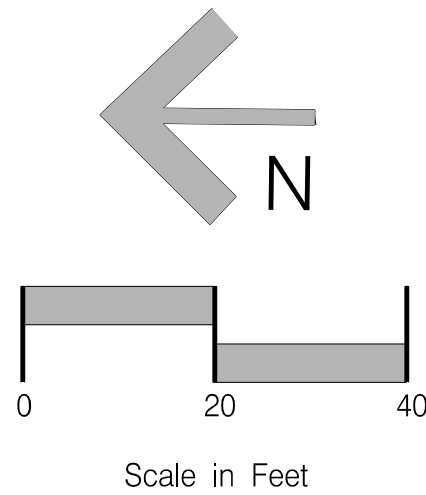
G & A
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

SKYWAY AMELIA, LLC

AMELIA ROAD
AND PARKWAYPOST-DEVELOPMENT
DRAINAGE PLAN

Registered Professional

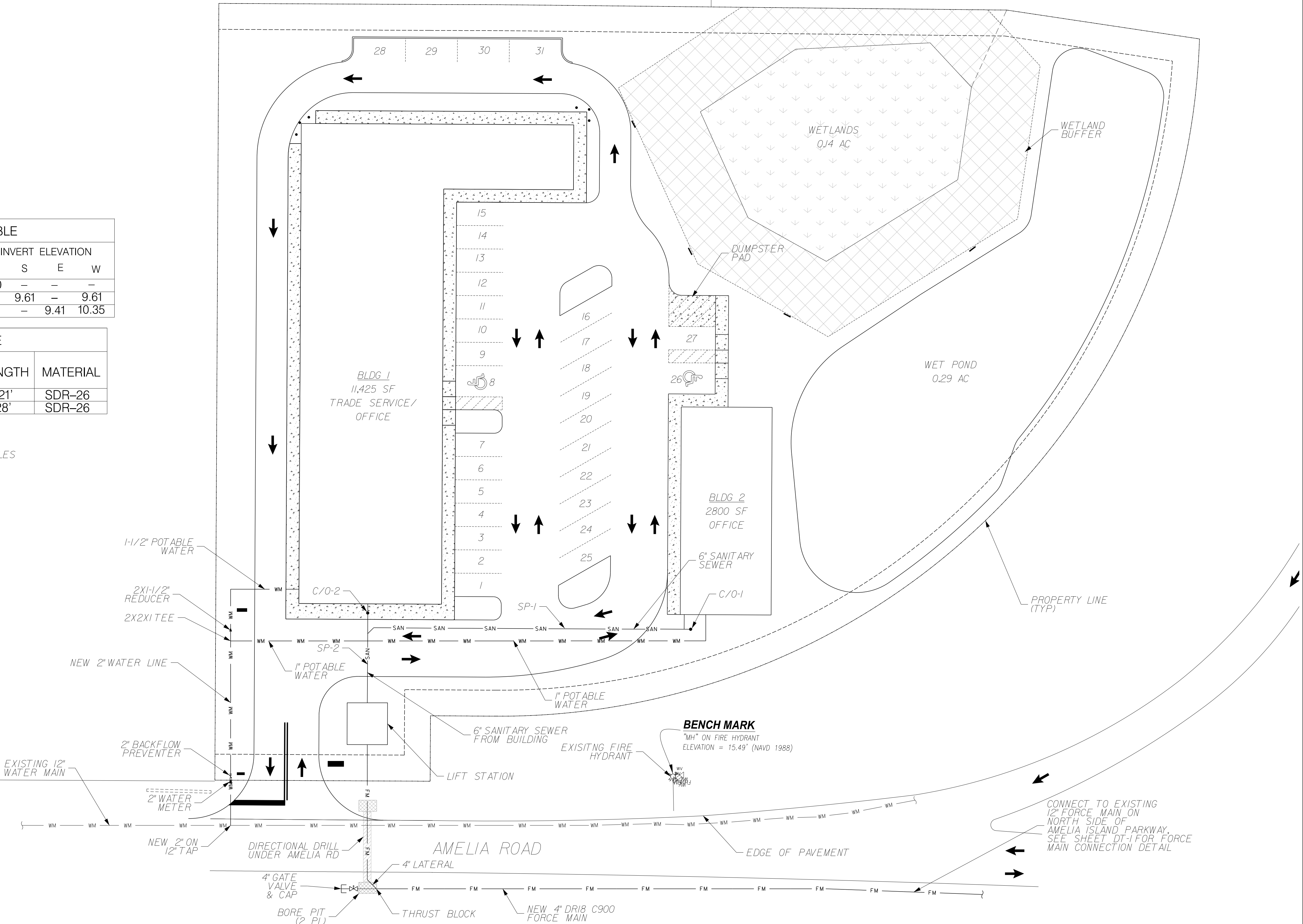
Sheet No.
DR-1
5 of 11
Issue Date
6-5-18
Project No.
17-10-23



SANITARY STRUCTURE TABLE						
NAME	TYPE	TOP/ INLET ELEV.	INVERT ELEVATION			
			N	S	E	W
CO-1	CLEANOUT	14.40	10.10	-	-	-
CO-2	CLEANOUT	13.90	-	9.61	-	9.61
L.S.	LIFT STATION	13.0 +/-	-	-	9.41	10.35

SANITARY PIPE TABLE					
NAME	FROM	TO	SIZE	LENGTH	MATERIAL
SP-1	CO-1	CO-2	6"	121'	SDR-26
SP-2	CO-2	L.S.	6"	28'	SDR-26

NOTE:
1. ALL CLEAN-OUTS (C/O) LOCATED IN DRIVING AISLES
OR SIDEWALKS TO BE HOUSED IN A
CAST IRON CLEAN-OUT VAULT.
2. NEW FORCE MAIN TO BE 4" Ø,
DR18 C900, GREEN OR BROWN COLOR



No.	By	Date	Revision
K			
J			
I			
H			
G			
F			
E			
D	MP	11-26-18	Revised Per CDFB TRC Comments
C	MP	11-2-18	Revised Pond & Site Per SJRWMD Comments
B	MP	10-8-18	Revised Per SJRWMD & CDFB Comments
A	MP	6-5-18	Issue For TRC Review

Scale: 1" = 20'
Project Mgr: AG
Designed by: AG
Drawn by: MP
QAQC: BM

Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL 32034
Certificate of Authorization No. 9332
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20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

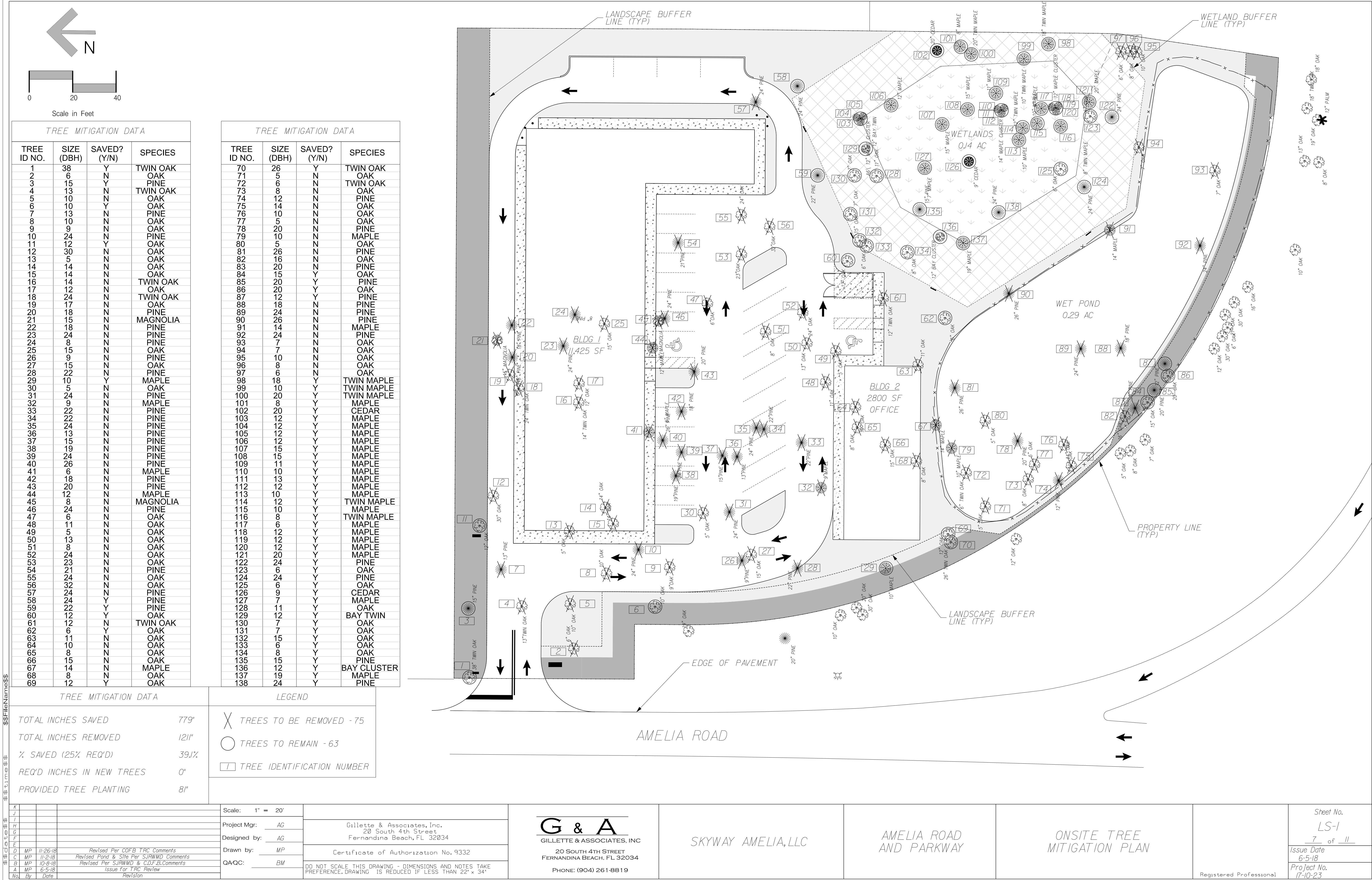
SKYWAY AMELIA, LLC

AMELIA ROAD
AND PARKWAY

UTILITY PLAN

Registered Professional

Sheet No.
UT-1
6 of 11
Issue Date
6-5-18
Project No.
17-10-23



4. INSTALL QUANTITY AND AT LOCATIONS REQUIRED BY CODE AND/OR AS INDICATED ON DRAWINGS.CONFIRM ALL HANDICAPPED SIGNAGE CODES.

NOTE: PROVIDE ADDITIONAL "VAN ACCESSIBLE" SIGNAGE.

PARKING BY DISABLED PERMIT ONLY

\$100 FINE

84"

2.5" GALVANIZED STEEL PIPE

FINISHED PAVING

HC PARKING SIGN

4" STRIPES - PAINT
BLUE (15 MIL)
MINIMUM THICKNESS

12" WHITE

18'

12'

5'

18"

Stall dimension measured from
inside to inside striping

Handicap Space and Access aisle
to be concrete pads

4" STRIPES - PAINT
YELLOW (15 MIL)
MINIMUM THICKNESS

A diagram of a rectangular frame. The vertical side on the left is labeled l and the horizontal side at the bottom is labeled b . The frame is composed of two vertical lines and two horizontal lines, with the labels l and b indicating the overall dimensions.

Note: For use adjacent to concrete or flexible pavement, concrete shown. Expansion joint, preformed joint filler and joint seal are required between curbs and concrete pavement only.

ORIFICE EL = 12.05

C B

NEW 4" FORCE MAIN,
CONNECT TO EXISTING
12" FORCE MAIN ON
NORTH SIDE OF AMELI
ISLAND PARKWAY

— EXISTING 12" FORCE MAIN,
CONTRACTOR TO FIELD
VERIFY LOCATION



AMELIA ISLAND PARKWAY

AIRPORT
ROAD

FORCE MAIN CONNECTION DETAIL

MATCH EXISTING
TOP OF BANK

$\therefore \text{SLOPE} = 3:1$

$SLOPE = 2:1$

POND BOTTOM EL = 4.50

18" OUTFALL PIPE
(P-OUT-1)

— DROP STRUCTURE (S-OUT-1)
MODIFIED TYPE "C" INLET
(SEE DETAIL THIS SHEET)

WET POND CROSS SECTION DETAIL

D B

		DIMENSIONS AND QUANTITIES										
		D	X	A	B	C	E	F	G	H	M	N
12 Slopes	15°	27"	1.92	2.18	2.00	2.06	5	122	29	4.63	1.19	
	18°	210"	1.87	274"	3.47	2.90	6	141	34	4.92	1.21	
	21°	30"	1.96	3.85	3.91	3.56	7	173	34	5.50	1.20	
	30°	43"	2.15	4.95	7.10	4.50	8	200	34	6.08	1.29	
	36°	51"	2.25	6.08	8.33	5.56	9	224	34	6.67	1.33	
	42°	60"	2.34	7.31	9.55	6.56	10	245	34	7.05	1.38	
	48°	69"	2.43	8.33	10.76	7.59	11	265	34	7.53	1.42	
54°	78"	2.52	9.44	11.90	8.50	12	283	34	8.42	1.46		
60°	8"	2.62	10.56	13.18	9.56	14	300	44"	9.00	1.50		

[illegible]

M.E.S. TABLE DIM.

TYPICAL DRIVEWAY DETAIL

PERVIOUS PAVEMENT/PAVER DETAIL

N		J	
I		J	
H		J	
G		J	
F		J	
E		J	
D	MP	11-26-18	Revised Per COFB TRC Comments
C	MP	11-2-18	Revised Pond & Site Per SJRWMD Comments
B	MP	10-8-18	Revised Per SJRWMD & COFB Comments
A	MP	6-5-18	Issue for TRC review
No.	By	Date	Revision

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PHONE: (904) 261-8819

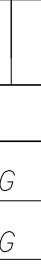
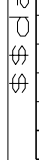
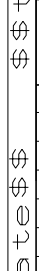
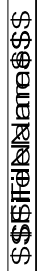
SKYWAY AMELIA, LLC

AMELIA ROAD
AND PARKWAY

MISCELLANEOUS DETAILS

17-10-23

Registered Professional



e one
SEWER SYSTEMS
MODEL DH152
NA0052P02

SPECIFICATIONS:

RATED FOR FLOWS OF 3000 GPD

150 GALLONS OF CAPACITY

INDOOR/OUTDOOR INSTALLATION

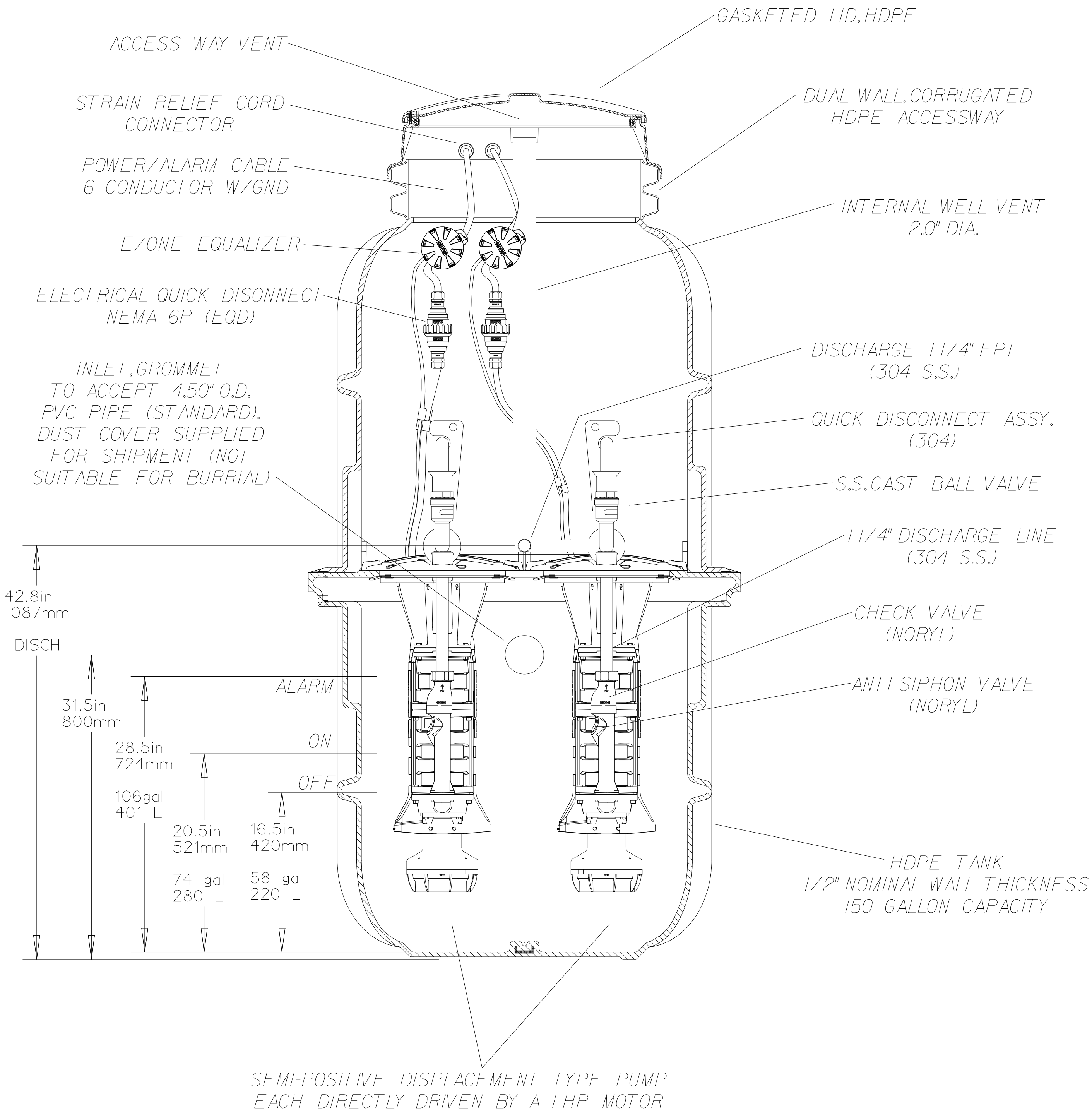
STANDARD OUTDOOR HEIGHTS RANGE
FROM 93 INCHES TO 160 INCHES

MOTOR:
1 HP, 1725 RPM, HIGH TORQUE, CAPACITOR
START, THERMALLY PROTECTED, 120/240V,
60 HZ, 1 PHASE

INLET CONNECTIONS:
4 INCH INLET GROMMET STANDARD DMV PIPE.
OTHER INLET CONFIGURATIONS AVAILABLE
FROM FACTORY

DISCHARGE CONNECTIONS:
PUMP DISCHARGE TERMINATES IN 1.25-INCH NPT
FEMALE THREAD. CAN BE ADAPTED TO 1.25-INCH
PVC PIPE OR ANY OTHER MATERIAL REQUIRED
BY LOCAL CODES.

DISCHARGE:
15 GPM AT 0 PSIG
11 GPM AT 40 PSIG
7.8 GPM AT 80 PSIG



CONCRETE BALLAST MAY BE REQUIRED
SEE INSTALLATION INSTRUCTIONS
FOR DETAILS

NOTE: DIMENSIONS ARE FOR REF ONLY

				Scale:		G & A GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819	SKYWAY AMELIA, LLC	AMELIA ROAD AND PARKWAY	LIFT STATION DETAILS	Registered Professional	Sheet No. DT-3 11 of 11	
				Project Mgr: AG	Certificate of Authorization No. 9332						Issue Date 6-5-18	
				Designed by: AG							Project No. 17-10-23	
				Drawn by: MP								
				QA/QC: BM	DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"							

PERMIT SET

ENGINEERING PLANS

FOR

COURTYARD/SPRINGHILL
DUAL BRAND HOTEL

FOR

THE COMPASS GROUP

961687 GATEWAY BLVD
SUITE 201 M
FERNANDINA BEACH, FL 32034

GILLETTE & ASSOCIATES, INC.

Civil & Environmental Engineering
Mechanical & Structural Engineering
Permitting & Construction Management
20 South 4th Street Amelia Island, Florida 32034
Phone: 904/261-8819 Fax: 904/261-9905

PERMITTING AGENCIES

CITY OF FERNANDINA BEACH: 904-310-3421
SJRWMD: 904-730-6270

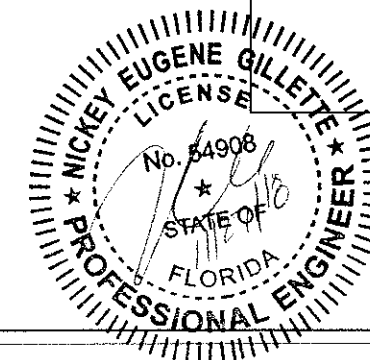
UTILITY PROVIDERS

WATER/SEWER: CITY OF FERNANDINA BEACH
ELECTRIC: FLORIDA PUBLIC UTILITIES
CABLE: COMCAST
TELEPHONE: AT&T

G&A Project
No.: 14-10.27

G & A

Issue Date: September 27, 2018
Revision Date: November 26, 2018

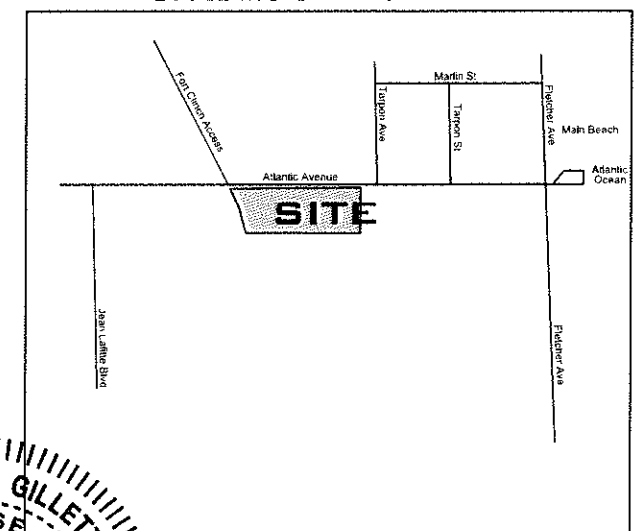


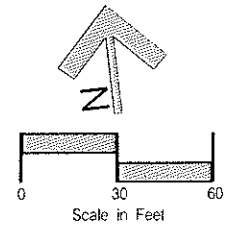
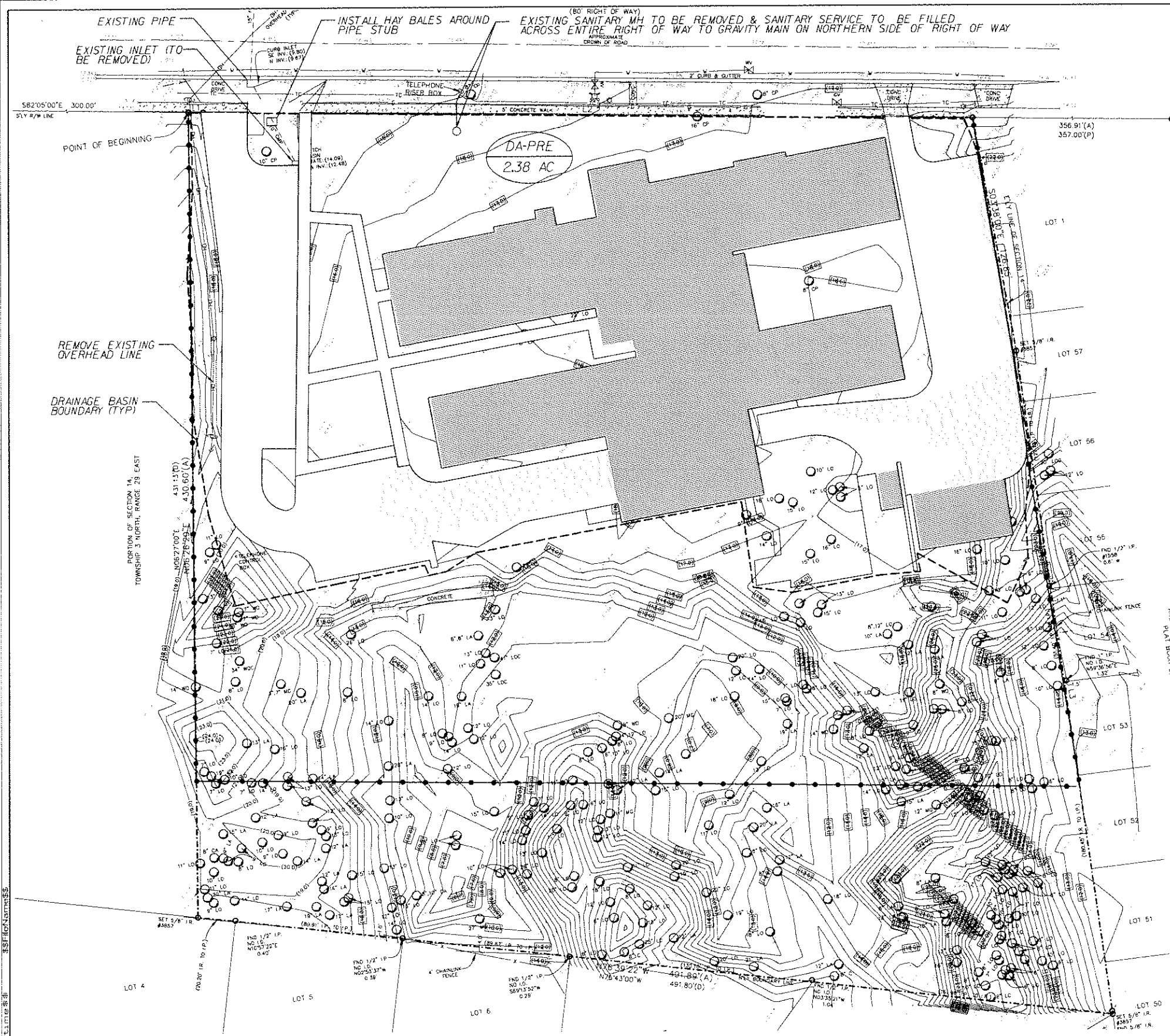
COURTYARD/SPRINGHILL
DUAL BRAND HOTEL

FOR
THE COMPASS
GROUP

RELEASED FOR CONSTRUCTION BY


SITE LOCATION MAP





—●— SILT FENCE (TYP)

LEGEND	NOTES
BUILDING AREA PAVEMENT AREA	32,003 SF BUILDING PREVIOUSLY CONSTRUCTED 30,084 SF PREVIOUSLY PAVED AREA TO BE REMOVED BIRMINGHAM, AL 35203 NO. 54908 STATE OF FLORIDA PROFESSIONAL ENGINEER SHEET NO. PRE-1 3 of 12 Issue Date SEPTEMBER 27, 2018 Project No. 18-10-27

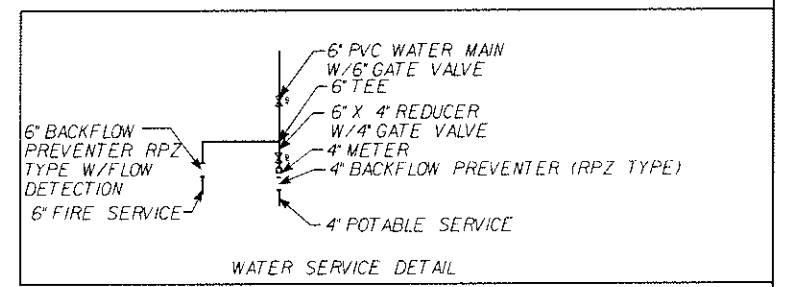
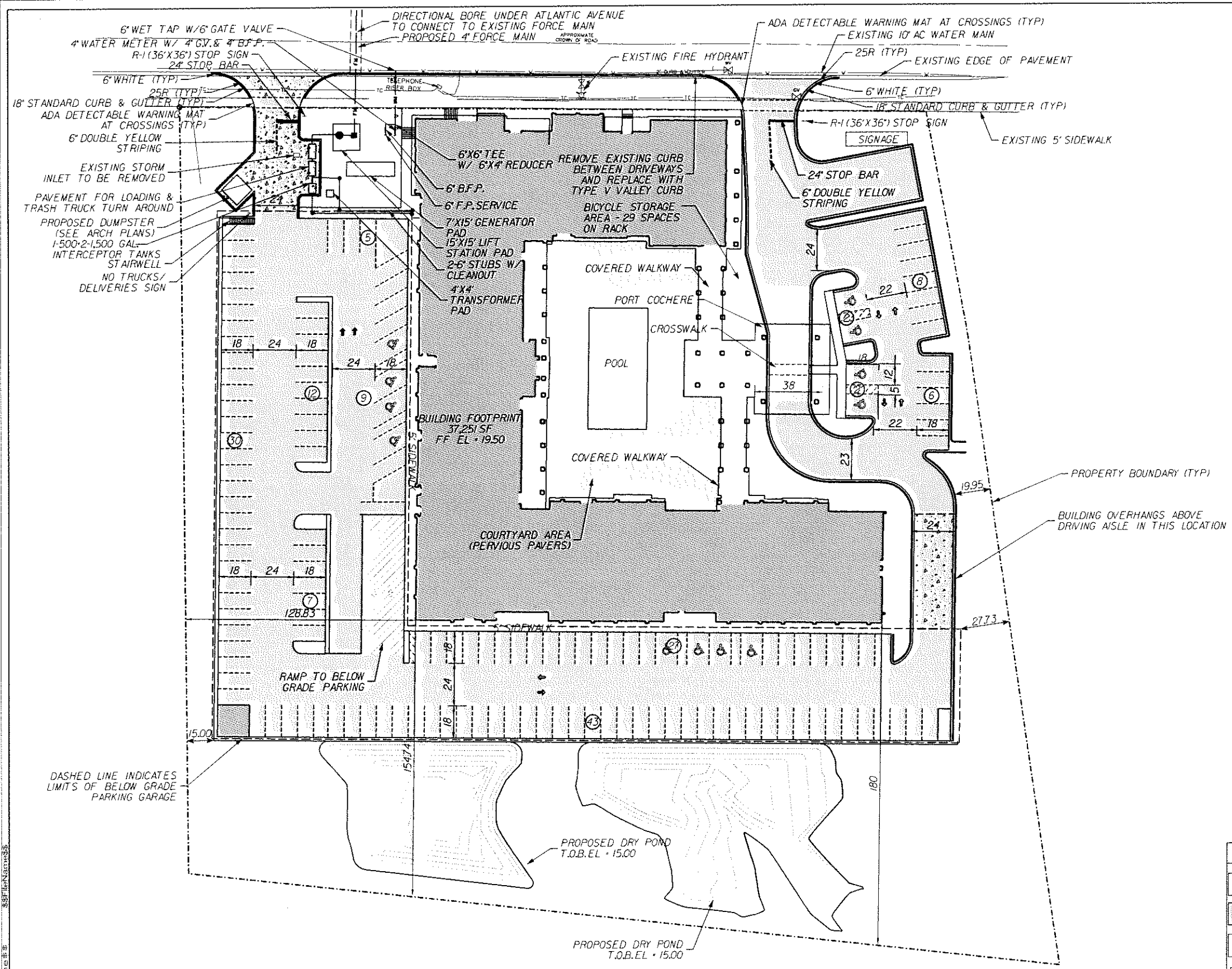
No.	By	Date	Revision
1	NG	11-26-18	Revised to address City of FBSJRWMD comments

Scale: 1" = 30'
Project Mgr: NG
Designed by: NG
Drawn by: BM
QA/QC: AG
DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PRECEDENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

G & A⁺
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819
FAX: (904) 261-9905

THE COMPASS GROUP
COURTYARD/SPRINGHILL DUAL BRAND

PREDEVELOPMENT DRAINAGE PLAN



NOTES

ZONING: C-1

FLUM DESIGNATION: GENERAL COMMERCIAL

PROPOSED 239 ROOM HOTEL

GROSS SITE ACREAGE = 4.70 AC.

143,968 SF OF TOTAL BUILDING AREA: (FAR = 0.70); MAX = 1.5

BUILDING HEIGHT = 45'

IMPERVIOUS AREA = 2.43 AC. 51.7% (60% MAX) BREAKDOWN:

37,091 SF ENCLOSED FOOTPRINT; 4,271 SF COVERED WALKWAY/PORT COCHERE; 2,241 SF POOL AREA; 62,275 SF PAVEMENT 11,921 PAVER AREA

MINIMUM VEHICLE CLEARANCE ON SITE IS 86 INCHES

SITE IS SITUATED IN FLOOD ZONE "X"

PARKING REQUIREMENTS
LODGING ACCOMMODATIONS WITH RESTAURANTS OR LOUNGES

1 SPACE PER SLEEPING ROOM PLUS 50% OF THE PARKING REQUIREMENT FOR THE RESTAURANT OR LOUNGE

RESTAURANT - 1 SPACE PER 50 SF OF THE CUSTOMER SERVICE AREA

239 ROOMS = 239 SPACES

2,101 SF RESTAURANT SERVICE AREA = 42 SPACES
50% = 21 SPACES

RETAIL - 1 SPACE / 300 SF
2,083 SF RETAIL SPACE = 7 SPACES

TOTAL SPACES REQUIRED = 267

TOTAL SPACES PROVIDED ABOVE GRADE = 151

TOTAL SPACES PROVIDED BELOW GRADE = 139

TOTAL SPACES PROVIDED = 290

12 TOTAL HANDICAPPED SPACES PROVIDED

SETBACKS FROM SIDE OF ROAD = 10'

REAR YARD SETBACK = 10'

LEGEND	
[Symbol]	PROPOSED PAVEMENT
[Symbol]	BUILDING FOOTPRINT
[Symbol]	PERVIOUS PAVERS
[Symbol]	6" THICK CONCRETE PAVEMENT; DUMPSTER PAD AREA TO BE 8" THICK

Scale: 1" = 30'
Project Mgr: NG
Designed by: NG
Drawn by: BM
QA/QC: AG
Revised to address City of Fernandina comments

Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL 32034
Certificate of Authorization No. 9332
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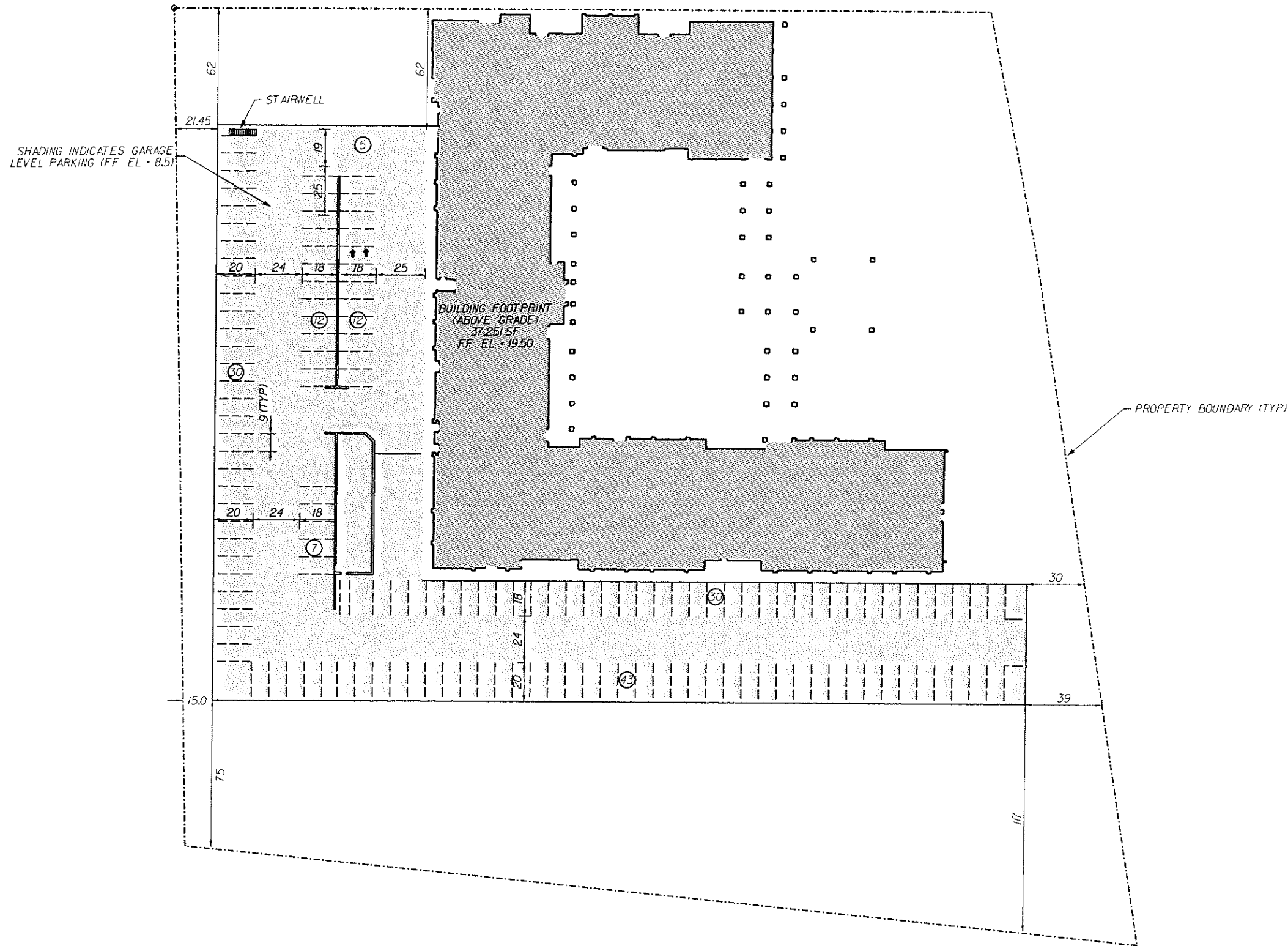
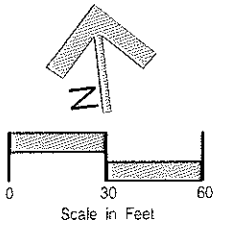
G & A
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819
FAX: (904) 261-9905

THE COMPASS GROUP

COURTYARD/SPRINGHILL
DUAL BRAND

ABOVE GRADE
GEOMETRY PLAN

Professional Engineer
No. 54908
EUGENE GILLETTE
STATE OF FLORIDA
REGISTERED PROFESSIONAL ENGINEER
EXPIRATION DATE: DECEMBER 27, 2013



\$\$\$\$\$Name\$\$\$\$\$
\$\$\$\$\$Time\$\$\$\$\$

Rev	By	Date	Description
A	NG	11-26-18	Revised to address City of FBS/JRWMO comments.

Scale:	
Project Mgr:	NG
Designed by:	NG
Drawn by:	BM
QA/QC:	AG

Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034
Certificate of Authorization No. 9332
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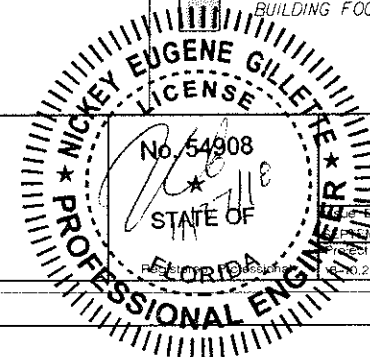
G & A
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819
FAX: (904) 261-9905

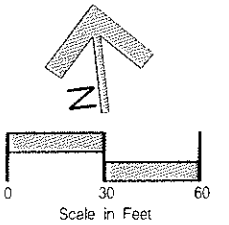
THE COMPASS
GROUP

COURTYARD/SPRINGHILL
DUAL BRAND

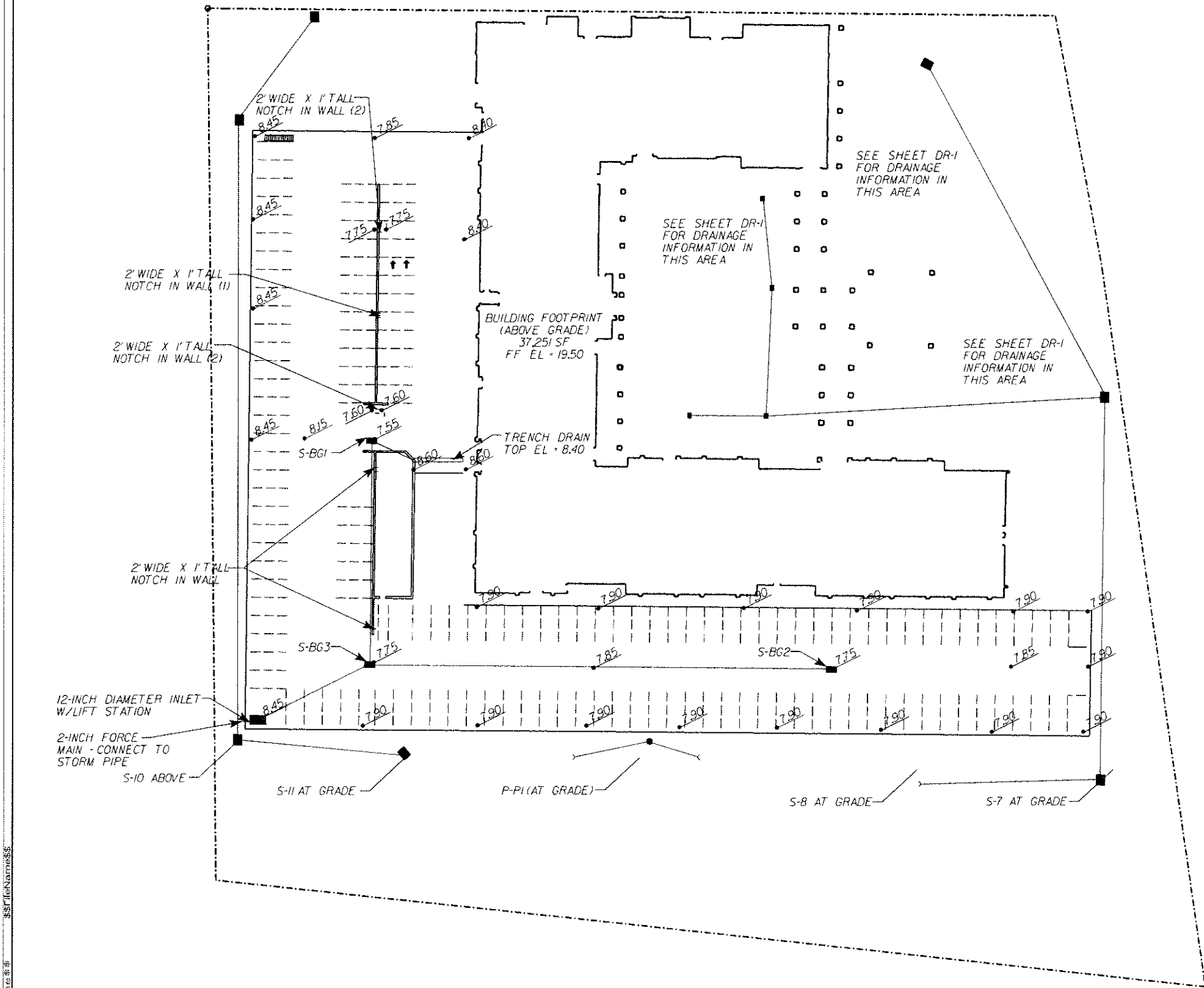
BELOW GRADE
GEOMETRY PLAN

LEGEND	
	PROPOSED PAVEMENT
	BUILDING FOOTPRINT
SHEET NO. GE-1 of 18	
DATE: DECEMBER 27, 2018	
DRAWN BY: NG	
CHECKED BY: BM	
APPROVED BY: AG	





STRUCTURE TABLE						
NAME	TYPE	TOP/ INLET ELEV.	INVERT ELEVATION			
			N	S	E	W
S-BG1	ZURN- Z536-C	7.55	TO BE SET BY CONTRACTOR			
S-BG2	ZURN- Z536-C	7.75				
S-BG3	ZURN- Z536-C	7.75				
LIFT	LIFT STATION	8.45				



NO.	BY	DATE	REVISION
1	NG	11-28-18	Revised to address City of FB&JRWMD comments

Scale:
Project Mgr: NG
Designed by: NG
Drawn by: BM
QA/QC: AG

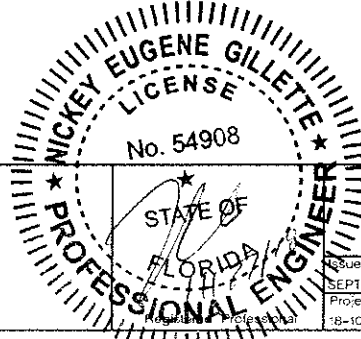
Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL 32034
Certificate of Authorization No. 9332
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FERNANDINA BEACH, FL 32034
PHONE (904) 261-8819
FAX (904) 261-9905

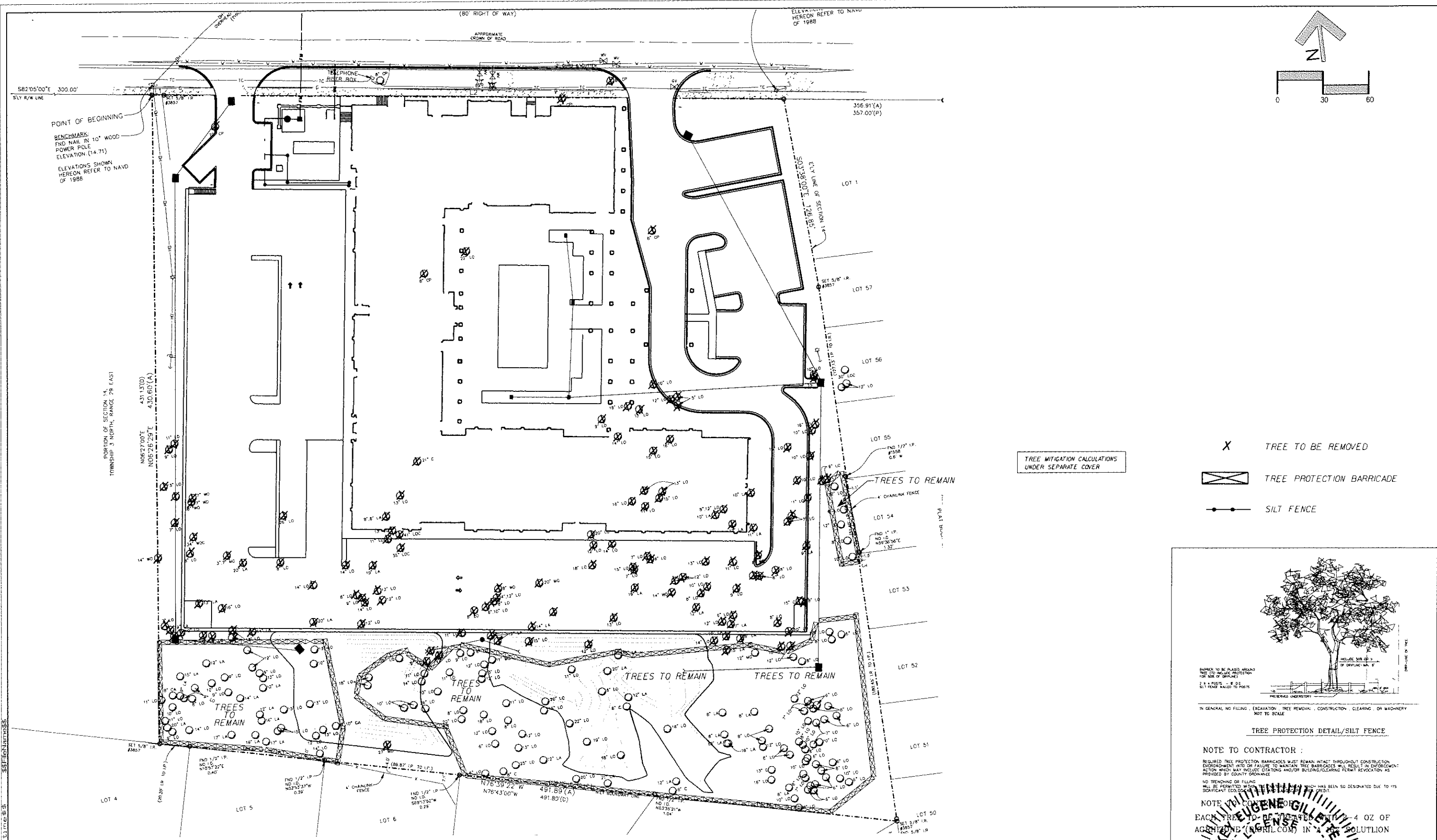
THE COMPASS
GROUP

COURTYARD/SPRINGHILL
DUAL BRAND

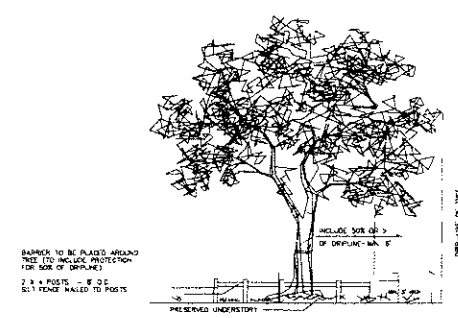
BELOW GRADE
DRAINAGE PLAN



SHEET NO.
DR-2
7 of 18
Issue Date
SEPTEMBER 27, 2018
Project No.
18-10.27



- X TREE TO BE REMOVED
- [Symbol] TREE PROTECTION BARRICADE
- [Symbol] SILT FENCE



IN GENERAL NO FILLING, EXCAVATION, TREE REMOVAL, CONSTRUCTION, CLEARING, OR MACHINERY NOT TO SCALE

TREE PROTECTION DETAIL/SILT FENCE

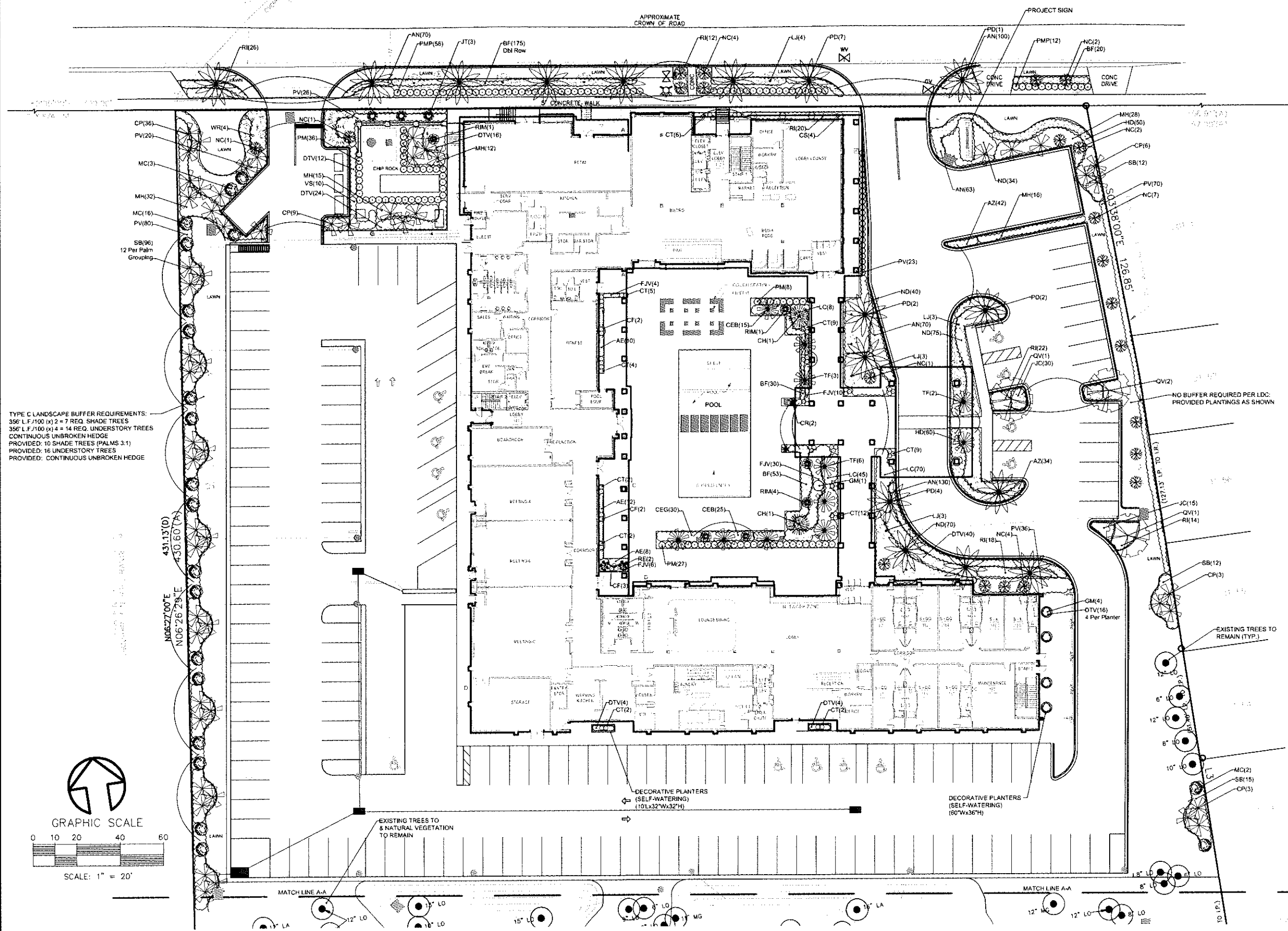
NOTE TO CONTRACTOR :
REQUIRED TREE PROTECTION BARRICADES MUST REMAIN INTACT THROUGHOUT CONSTRUCTION. ENCROACHMENT INTO OR FAILURE TO MAINTAIN TREE BARRICADES WILL RESULT IN ENFORCEMENT ACTION WHICH MAY INCLUDE CITATIONS AND/OR BUILDING/CLEARING PERMIT REVOCATION AS PROVIDED BY COUNTY ORDINANCE.

NO TRENCING OR FILLING WILL BE PERMITTED WITHIN THE TREE PROTECTION ZONE HAS BEEN SO DESIGNATED DUE TO ITS SIGNIFICANT ECOLOGICAL VALUE.

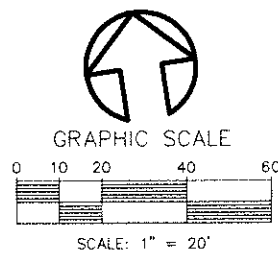
NOTE TO CONTRACTOR :
EACH TREE TO BE REMOVED WITHIN 4' OF AG MEASUREMENT (APRIL 2008) IN A SOLUTION

K			Scale: 1" = 30'			Gillette & Associates Inc. 20 South 4th Street Fernandina Beach, FL 32034			G & A ⁺ GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034			THE COMPASS GROUP			COURTYARD/SPRINGHILL DUAL BRAND			TREE REMOVAL TREE PROTECTION PLAN			SHEET NO. TREE-1		
L			Project Mgr: NG			Designed by: NG			Certificate of Authorization No. 8332			Phone: (904) 261-8819									No. 1		
M			Drawn by: BM			QA/QC: AG			DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"			Fax: (904) 261-9905									STATE OF FLORIDA		
N			A NG 11-26-18 Revised to address City of FBSJRWMD comments																		Issue Date SEPTEMBER 27, 2018		
O			By Date																		Project No. 18-10-27		
P																					PROFESSIONAL ENGINEER		
Q																							
R																							
S																							
T																							
U																							
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BN																							
BO																							
BP																							
BQ																							
BR																							
BS																							
BT																							

ATLANTIC AVENUE
(80' RIGHT OF WAY)



TYPE C LANDSCAPE BUFFER REQUIREMENTS:
356' L.F./100 (x) 2 = 7 REQ. SHADE TREES
356' L.F./100 (x) 4 = 14 REQ. UNDERSTORY TREES
CONTINUOUS UNBROKEN HEDGE
PROVIDED: 10 SHADE TREES (PALMS 3:1)
PROVIDED: 16 UNDERSTORY TREES
PROVIDED: CONTINUOUS UNBROKEN HEDGE



COURTYARD/SPRINGHILL
DUAL BRAND HOTEL
2700 ATLANTIC AVENUE
FERNANDINA BEACH, FLORIDA

REVISIONS:

NO.	DESCRIPTION

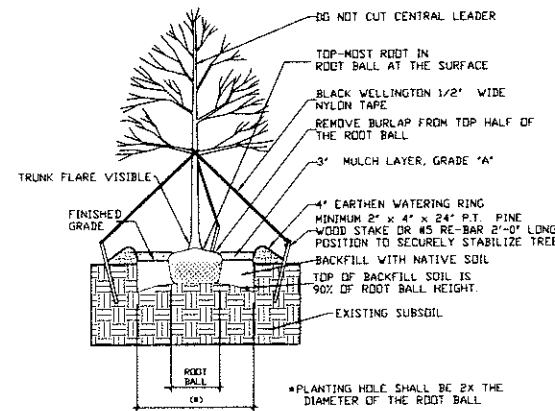
CULLIVER
DESIGN, INC.

LANDSCAPE ARCHITECTURE
173 SUNTREE COURT
ORMOND BEACH, FL 32174
TEL 386-871-6283
EMAIL: culliverdesign@att.net

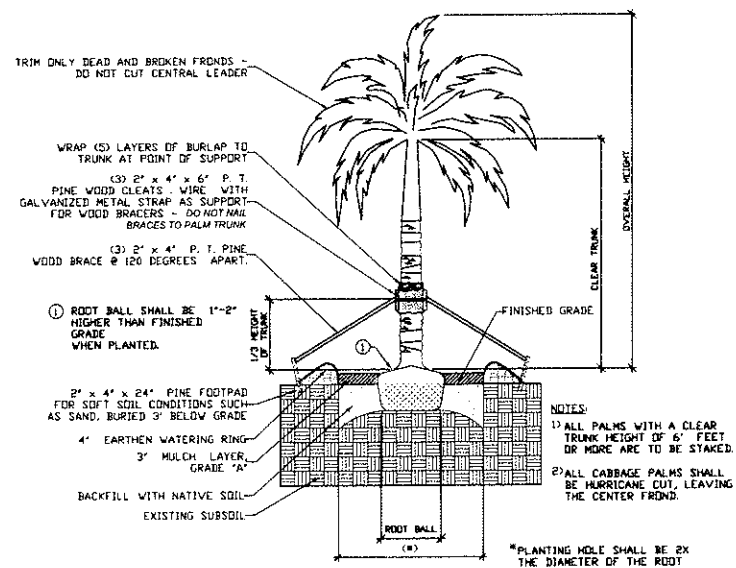
MEMBER: AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS

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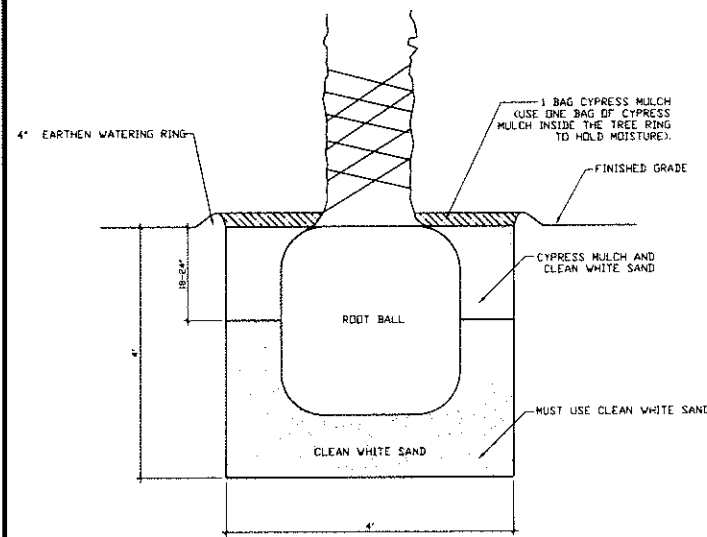
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DATE: 11/25/18
SCALE: 1"=20'-0"
DRAWING NO. 9 of 18



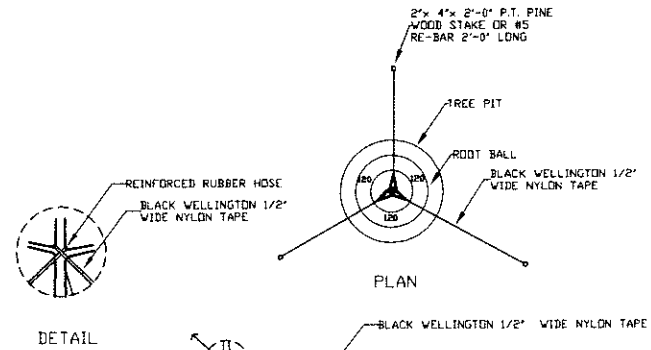
1 LARGE TREE PLANTING DETAIL



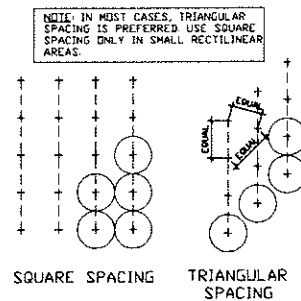
4 STRAIGHT TRUNK PALM DETAIL



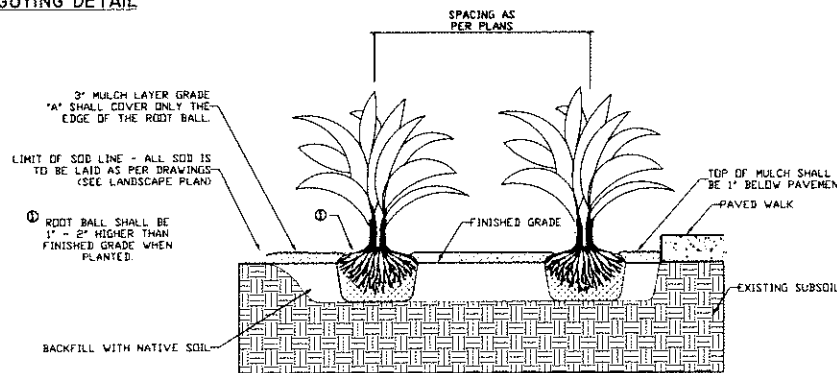
6 MEDJOOL PALM INSTALLATION SCHEMATIC
(INFORMATION BY GROUNDWORKS)



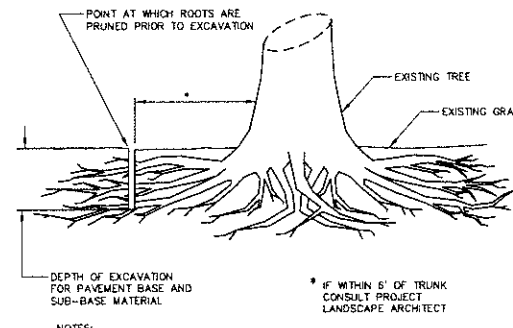
2 TYPICAL TREE GUYING DETAIL



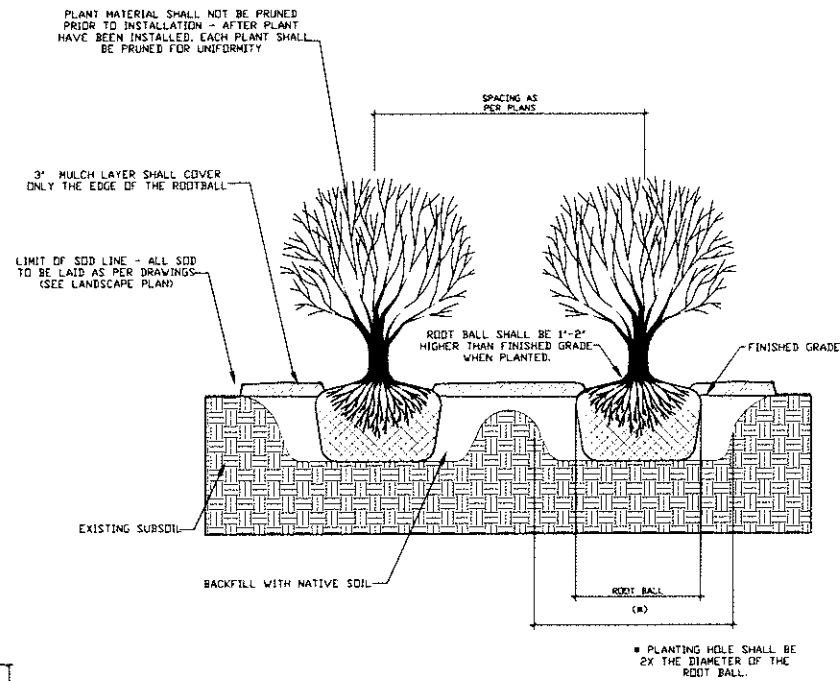
7 TYPICAL CONTAINER SPACING



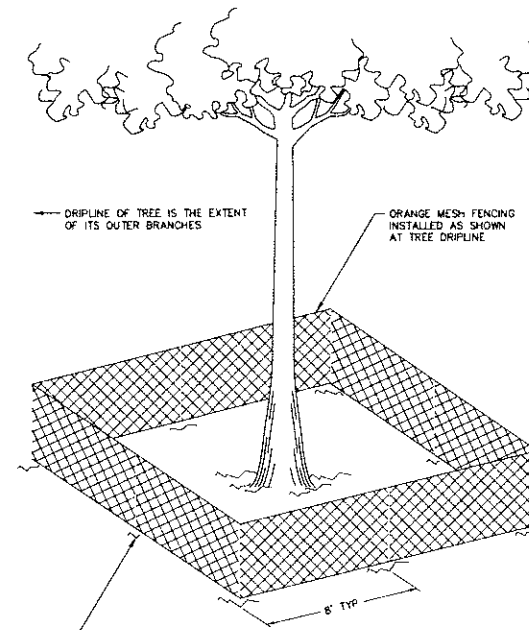
5 TYPICAL GROUND COVER PLANTING DETAIL



8 ROOT PRUNING DETAIL



3 TYPICAL SHRUB PLANTING DETAIL



9 TREE BARRICADE DETAIL

COURTYARD/SPRINGHILL
DUAL BRAND HOTEL
2700 ATLANTIC AVENUE
FERNANDINA BEACH, FLORIDA

REVISIONS:

CULLIVER
DESIGN, INC.

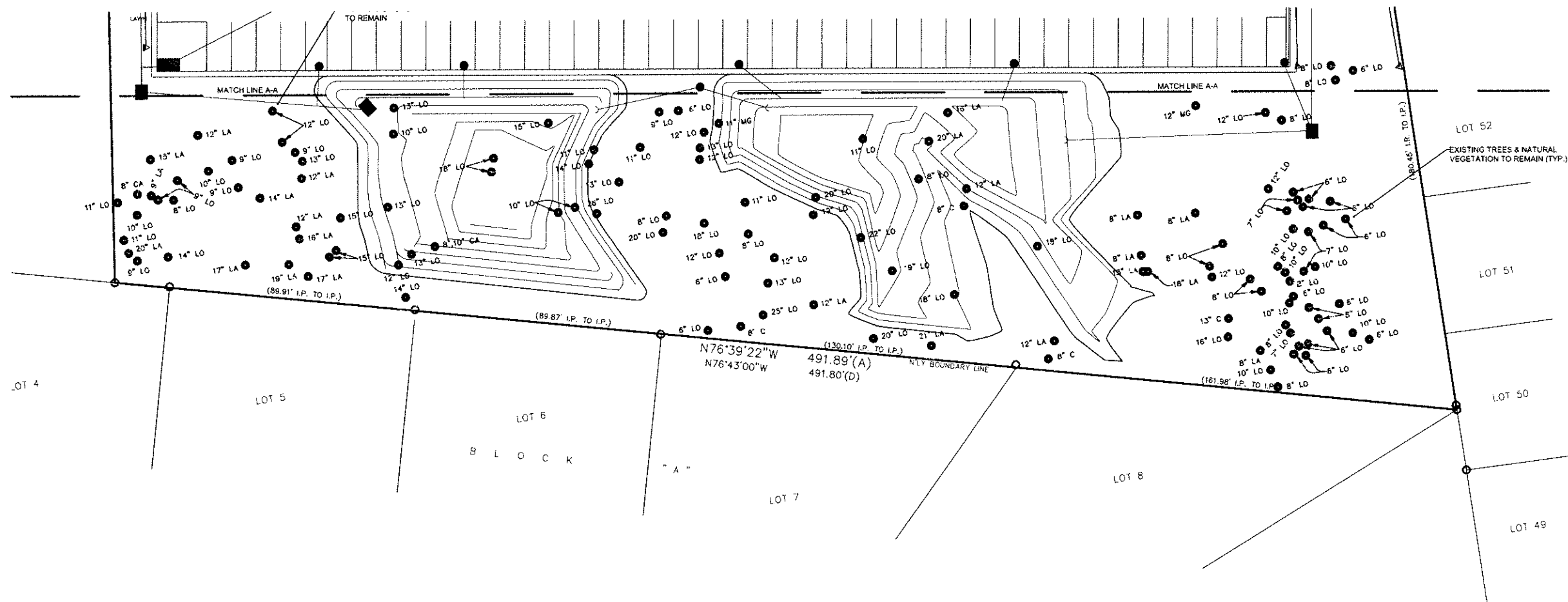
LANDSCAPE ARCHITECTURE
173 SUNTREE COURT
ORMOND BEACH, FL 32174
TEL 386-871-0293
EMAIL: culliverdesign@earthlink.net

TITLE: LANDSCAPE DETAILS

DATE: 03/28/18

SCALE: N.T.S.

DRAWING NO. 11 OF 18



IRRIGATION LEGEND:

TORO IRRIGATION 570Z PRX SERIES: MODEL# 570Z-6P-PRX (LAWN AREA) & 570Z-12P-PRX (PLANTING BEDS)

HEAD SYMBOL:	IRRIGATION DESCRIPTION:
	PROPOSED SPECIAL PATTERNS - 4-EST (4'x15')
	PROPOSED SPECIAL PATTERNS - 4-SST (4'x30')
	PROPOSED 8' SERIES - MODEL#: 8-Q & 8-H
	PROPOSED 10' SERIES - MODEL#: 10-Q, 10-T & 10-H
	PROPOSED 12' SERIES - MODEL#: 12-Q, 12-T & 12-H
	PROPOSED 15' SERIES - MODEL#: 15-Q, 15-T & 15-H
	HUNTER IRRIGATION - PRO-SPRAY: MODEL# PROS-06 (LAWN AREAS) & PROS-12 (PLANTING BEDS)
	PROPOSED END STRIP PATTERN NOZZLE - ES-515 (5'x15')
	PROPOSED SIDE STRIP PATTERN NOZZLE - SS-530 (5'x30')
	HUNTER IRRIGATION - PGJ ROTOR: MODEL# PGJ-ADJ (6" Pop-Up-LAWN AREAS) & PGJ-(12" Pop-Up-PLANTING BEDS)
	PROPOSED PGJ NOZZLE 1.0 (QUARTER) & NOZZLE 2.0 (HALF) (25'-30' Radius)
	HUNTER IRRIGATION - PGP ROTOR: MODEL# PGP-ADJ (4" Pop-Up-LAWN AREAS) & PGP-(12" Pop-Up-PLANTING BEDS)
	PROPOSED PGP NOZZLE 3 (QUARTER) & NOZZLE 6 (HALF) (25'-35' Radius)
	NEW SCHEDULE 40 PVC SLEEVE (SIZE AS NOTED)
	NEW SCHEDULE 160 PVC LATERAL 3/4" MIN. (SIZE AS NOTED)
	NEW SCHEDULE 40 PVC MAINLINE (2")
	PGV-101A & PGV-151 (REFER TO PLAN)
	HUNTER PRO-C CONTROLLER MODEL# PC-1200 & PCM-300; 7 Stations (Plastic Cabinet)
	PROPOSED 1.5" IRRIGATION METER TO TIE INTO EXISTING WATER MAIN
	PROPOSED REDUCED PRESSURE BACKFLOW PREVENTER (SIZE AS REQUIRED)
	HUNTER MINI CLICK RAIN SWITCH

IRRIGATION CONTRACTOR NOTES:

CONTRACTOR IS RESPONSIBLE TO CORRECTLY SIZE ALL IRRIGATION LATERALS FOR OPTIMUM SYSTEM PERFORMANCE. (MINIMUM 3/4" SCHEDULE PR 160 PVC)
 - ALL HEADS LOCATED IN LAWN AREAS SHALL BE A 6" POP UP, UNLESS OTHERWISE SPECIFIED.
 - ALL HEADS IN PLANTING AREAS SHALL BE A 12" POP UP.

COURTYARD/SPRINGHILL
 DUAL BRAND HOTEL
 2700 ATLANTIC AVENUE
 FERNANDINA BEACH, FLORIDA

REVISIONS:

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 DESIGN, INC.

LANDSCAPE ARCHITECTURE
 173 SUNTREE COURT
 ORMOND BEACH, FL 32174
 TEL 386-871-5293
 EMAIL: culliverdesign@icloud.com

MEMBER, AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS

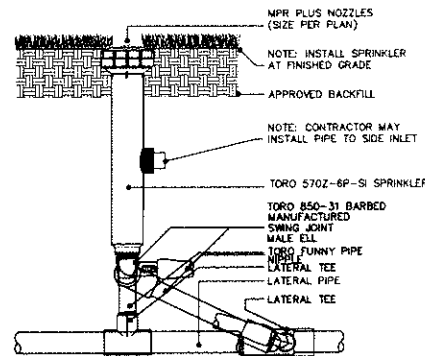
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TITLE: IRRIGATION PLAN

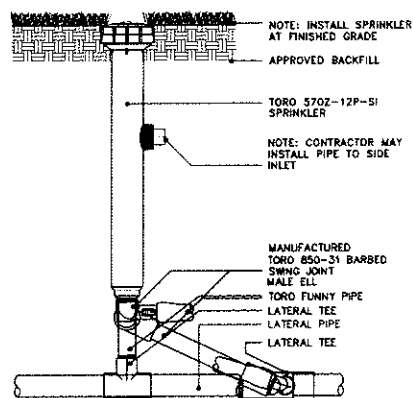
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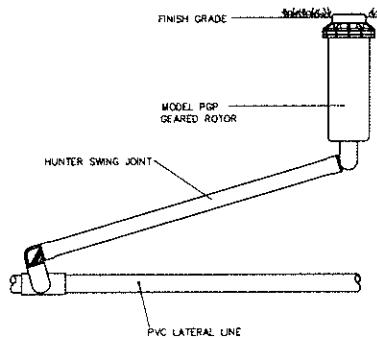
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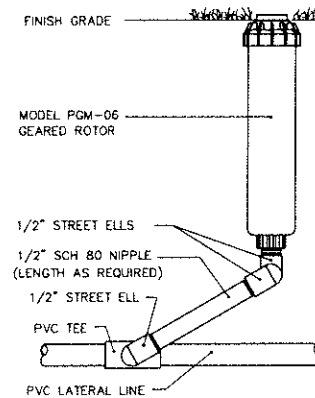
1 TORO 570Z-6P-SI



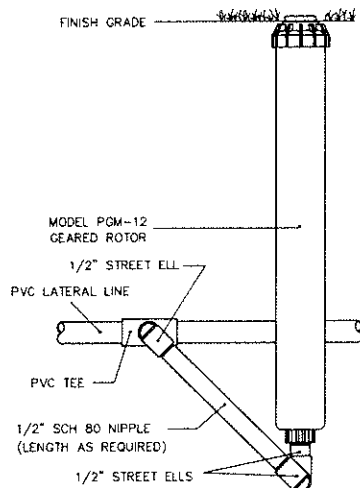
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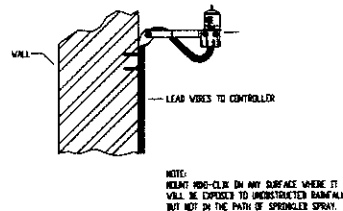
3 HUNTER PGP-SJ GEAR DRIVE



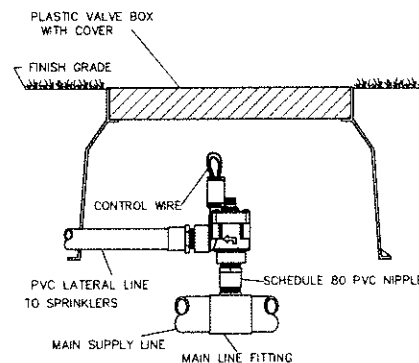
4 HUNTER PGJ-06 ROTOR



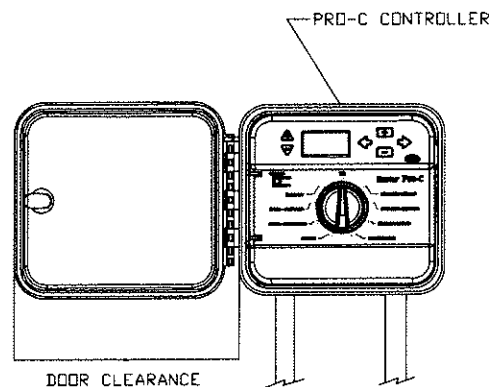
4A HUNTER PGJ-12 ROTOR



5 HUNTER MINI CLICK



6 HUNTER PGVI-ANGLE



7 HUNTER PRO-C CONTROLLER (OUTDOOR)

IRRIGATION NOTES:

1. THE ENTIRE SYSTEM TO BE INSTALLED IN COMPLIANCE WITH ALL LOCAL CODES. CONTRACTOR TO SUPPLY AND INSTALL ALL CHECK VALVES, BACKFLOW PREVENTERS, PERMITS ETC., AS REQUIRED BY THESE CODES. EVEN IF THESE ITEMS ARE NOT SHOWN ON THE DRAWING.
2. ALL MAIN LINE PIPING SHALL BE BURIED TO A MINIMUM DEPTH OF 18" OF COVER AND ALL LATERAL PIPING SHALL BE BURIED TO A MINIMUM DEPTH OF 12" OF COVER.
3. PIPE SIZES SHALL CONFORM TO THOSE ON THE DRAWINGS. THE SMALLEST LATERAL PIPE SIZE TO A SINGLE SPRAY OR ROTOR HEAD SHALL BE 3/4".
4. ALL REMOTE CONTROL VALVES, GATE VALVES AND QUICK COUPLERS SHALL BE INSTALLED IN VALVE BOXES.
5. ALL RISERS SHALL BE STAKED WITH A STEEL ANGLE AND SECURED WITH UV RESISTANT PLASTIC CABLE TIES. RISERS AND ANGLE SHALL BE PAINTED HUNTER GREEN.
6. ALL RISERS SHALL BE INSTALLED 12"-INCHES FROM ANY WALL AND A MINIMUM OF 18"-INCHES FROM ANY SIDEWALK, PATIO OR ROAD.
7. ALL LOW VOLTAGE WIRING TO BE #14 DIRECT BURIAL WIRE. COLORED RED FOR CONTROL WIRES AND WHITE FOR COMMON WIRES (UNLESS OTHERWISE INDICATED IN THE SPECIFICATIONS). NO CROSS CONNECTION BETWEEN CONTROLLERS SHALL BE ALLOWED. WIRE SPLICES SHALL BE MADE ONLY IN VALVE BOXES USING KING "ONE STEP" CONNECTORS AND SEALANT.
8. ANY PIPING OR VALVES SHOWN OUTSIDE THE PROPERTY LINE OR OUTSIDE OF A LANDSCAPE AREA IS SHOWN THERE FOR DESIGN CLARITY ONLY. ALL PIPING AND VALVES SHALL BE INSTALLED ON THE PROPERTY AND WITHIN LANDSCAPE AREAS.
9. FINAL LOCATION OF THE AUTOMATIC CONTROLLER(S) SHALL BE APPROVED BY THE OWNER'S AUTHORIZED REPRESENTATIVE PRIOR TO INSTALLATION.
10. ELECTRICAL SERVICE TO ALL EQUIPMENT SHALL BE PROVIDED TO A JUNCTION BOX AT THE EQUIPMENT LOCATION (BY OTHERS, NOT PART OF THIS CONTRACT). THE IRRIGATION CONTRACTOR SHALL BE RESPONSIBLE FOR THE FINAL CONNECTION FROM THE JUNCTION BOX TO ALL EQUIPMENT.
11. THE IRRIGATION CONTRACTOR SHALL FLUSH AND ADJUST ALL SPRINKLER HEADS AND VALVES TO PROVIDE OPTIMUM COVERAGE WITH MINIMAL OVERSPRAY ONTO WALKS, STREETS, WALLS ETC... IN ORDER TO ACCOMPLISH THIS, THE CONTRACTOR MAY SUBSTITUTE VARIABLE ARC NOZZLES IN PLACE OF THE SPECIFIED FIXED ARC NOZZLES WHERE NECESSARY. PRESSURE COMPENSATING SCREENS MAY ALSO BE USED TO REDUCE SPRAY DISTANCE.
12. ALL SPRINKLER EQUIPMENT NOT OTHERWISE DETAILED OR SPECIFIED SHALL BE INSTALLED AS PER MANUFACTURER'S SPECIFICATIONS.
13. ALL SLEEVES UNDER SIDEWALKS TO BE BURIED 6"-12" BELOW WALK SURFACE.
14. MAIN LINE MUST BE INSTALLED PER PLAN. OTHER LOCATIONS WILL NOT BE ACCEPTED.
15. ALL SPRAY HEADS OR NOZZLES IN PLANTING BEDS TO BE PLACED ON RISERS (SCH. 40) OF APPROPRIATE HEIGHT TO PLANTING AND RISERS TO BE PAINTED HUNTER GREEN.
16. ALL HEADS IN OPEN LAWN AREAS TO BE MOUNTED ON SWING JOINTS.
17. ALL MATERIALS TO BE NEW. ALL PIPE TO BE CUT SQUARELY AND BURRS REMOVED. ALL PVC JOINTS TO BE MADE WITH THE USE OF CLEANER, PRIMER AND CLEAN SOLVENT WELD.
18. ALL WORK TO BE DONE IN GOOD WORKMAN LIKE MANNER AND IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
19. ALL LINE LOCATIONS ARE APPROXIMATE. CONTRACTOR TO ADJUST TRENCHING IN THE FIELD FOR EXISTING CONDITIONS, PLANTINGS AND STRUCTURES.
20. HEAD LOCATIONS ARE APPROXIMATE. CONTRACTOR TO MAINTAIN 100% COVERAGE WITH 50% OVERLAP.
21. CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION AND EXISTENCE OF ALL UNDERGROUND UTILITIES. CONTRACTOR SHALL VERIFY UTILITIES IN THE FIELD. CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION AND MAINTENANCE AND SHALL BE RESPONSIBLE FOR DAMAGE TO EXISTING UTILITIES OR STRUCTURES.
22. THERE SHALL BE A ONE (1) YEAR GUARANTEE ON ALL PARTS AND LABOR FROM THE DATE OF FINAL ACCEPTANCE. AT THE END (1) YEAR FROM THE DATE OF FINAL ACCEPTANCE, THE CONTRACTOR SHALL CHECK AND ADJUST (IF NECESSARY) THE ENTIRE SYSTEM.
23. CONTRACTOR SHALL MARK STATIONS ON THE CONTROLLER PANEL TO CORRESPOND WITH THE ZONE NUMBERS ON THIS PLAN AND SET OPERATING SEQUENCE TO CORRESPOND TO THE NUMBERING.
24. ALL SLEEVES TO BE INSTALLED UNDER EXISTING ASPHALT OR WALKS TO BE INSTALLED BY DIRECTIONAL DRILL OR JACK & BORE UNDER THE PAVEMENT.
25. ALL VALVE BOXES SHALL BE FLUSH WITH FINISH GRADE AND HAVE A MINIMUM OF 3"-INCHES OF CHIP ROCK INSIDE THE BOX AROUND THE VALVE FOR POSITIVE DRAINAGE.

COURTYARD SPRINGHILL
DUAL BRAND HOTEL
2700 ATLANTIC AVENUE
FERNANDINA BEACH, FLORIDA

REVISIONS

CULLIVER
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LANDSCAPE ARCHITECTURE
173 SUNTREE COURT
DRMOND BEACH, FL 32174
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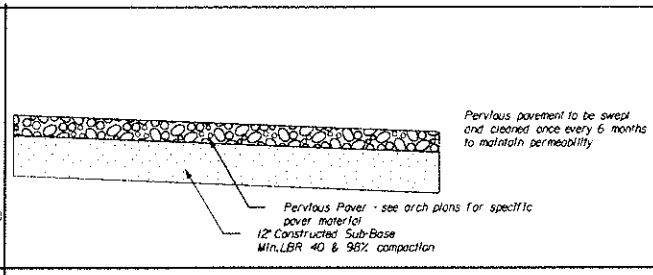
MEMBER: AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS

TITLE: IRRIGATION DETAILS

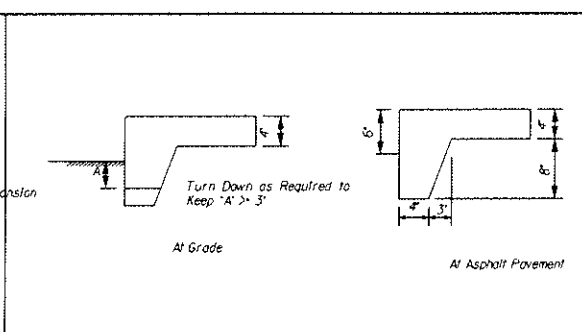
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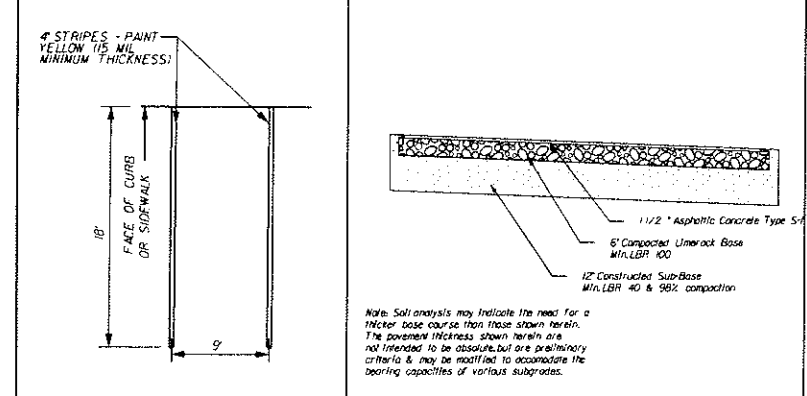
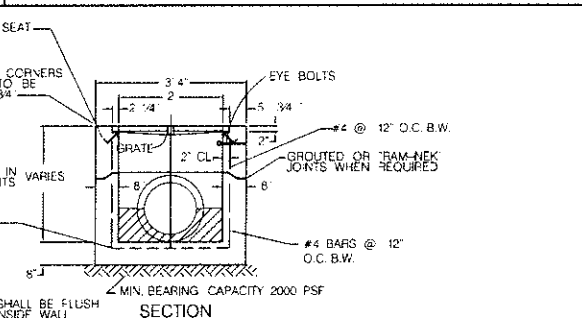
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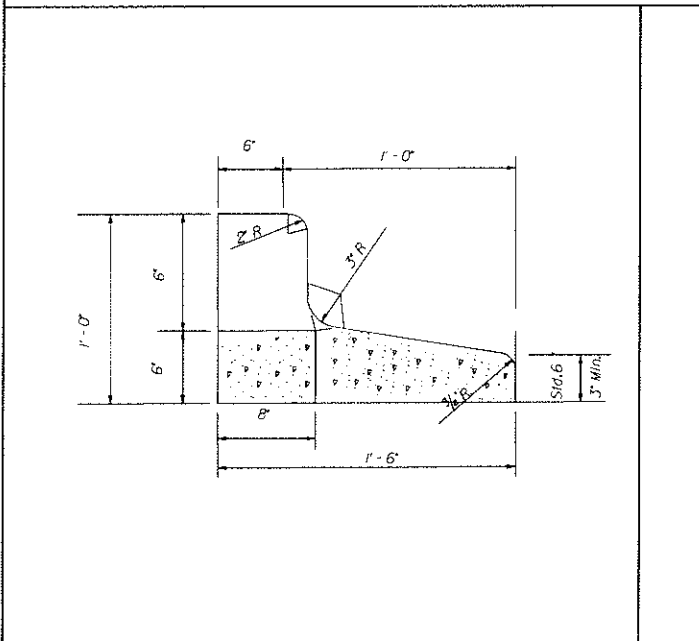
PERVIOUS PAVER DETAIL



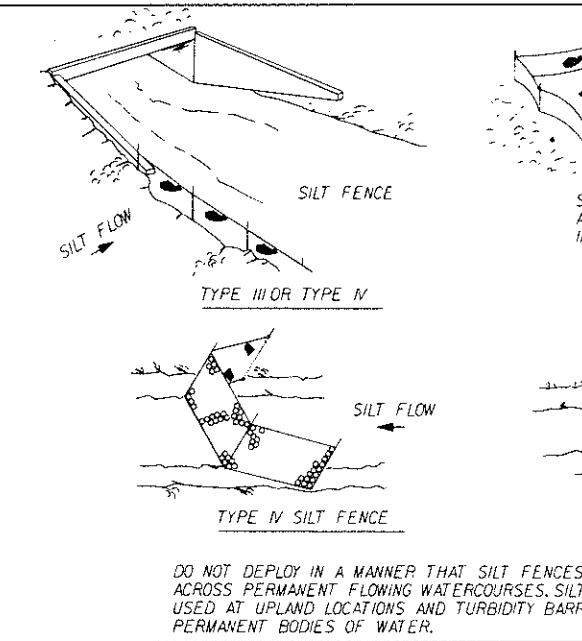
SIDEWALK TURNDOWN DETAILS



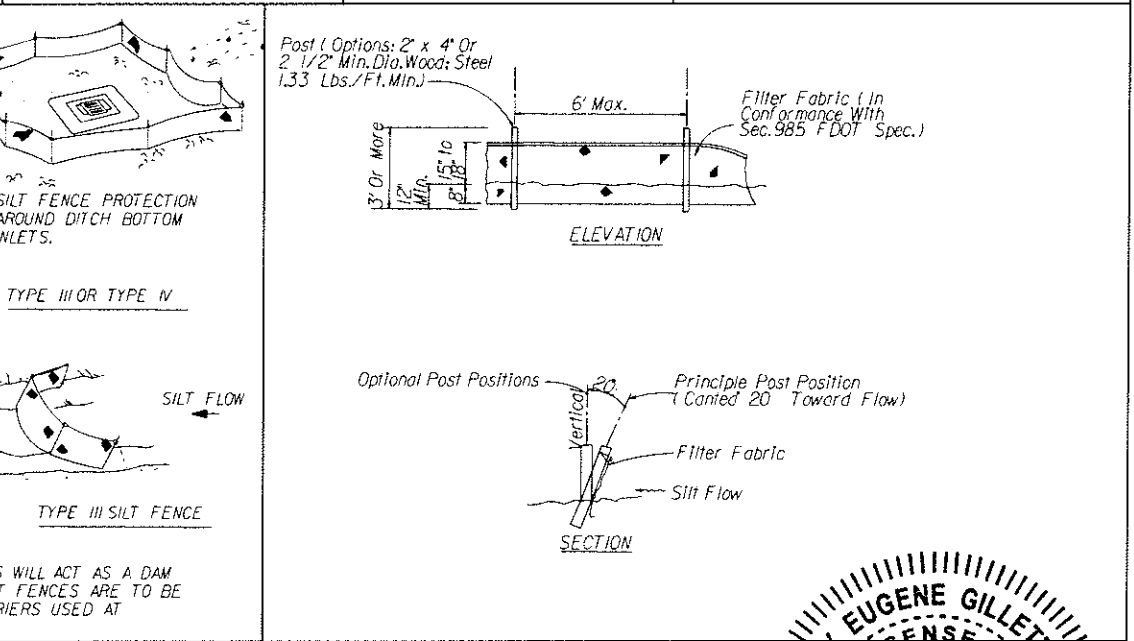
PAVEMENT DETAIL



CITY STANDARD CURB & GUTTER

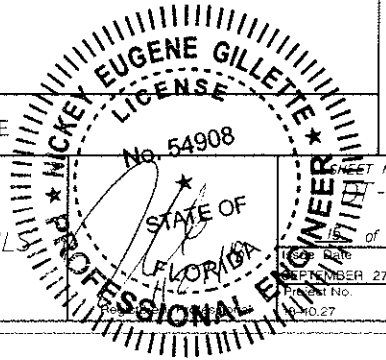


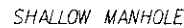
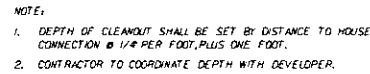
SILT FENCE DETAILS



TYPE III SILT FENCE

Scale: Project Mgr: NG Designed by: NG Drawn by: BM QA/QC: AG Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034 Certificate of Authorization No. 9332 DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34" G & A GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819 FAX: (904) 261-9905	THE COMPASS GROUP	COURTYARD/SPRINGHILL DUAL BRAND	MISCELLANEOUS DETAILS
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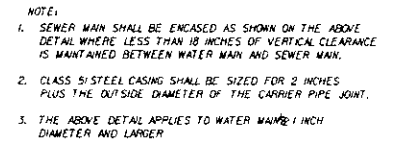
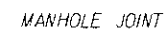
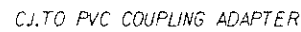
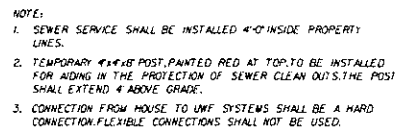




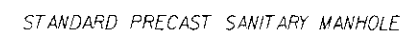
CLASS "B"

1. ALL MANHOLES SHALL BE PRECAST CONCRETE AND CONFORM TO ASTM C-498. STANDARDS. MANHOLE COVERS AND RISERS SHALL BE CONSTRUCTED OF 400 PSI TYPE II CEMENT WITH RIVER ROCK OR GRANITE AGGREGATE.
2. ALL PIPES SHALL BE CONNECTED TO THE MANHOLE USING A 10' OR JOINT FLEXIBLE MANHOLE SLEEVE (ASTM C-923) OR APPROVED EQUAL.
3. THE EXTERIOR AND INTERIOR OF THE MANHOLE SHALL BE COATED IN ACCORDANCE WITH THE TOP AND BOTTOM RECOMMENDATIONS WITH FIBERGLASS AS MANUFACTURED BY BURGESS BITUMINOUS PROTECTION COMPANY.
4. AN INSIDE PVC DROPPED MANHOLE (TYPE "B") SHALL BE REQUIRED FOR MANHOLES WHICH HAVE A DISTANCE OF 18 INCHES OR GREATER BETWEEN INFLUENT PIPE INVERT AND EFFLUENT PIPE INVERT. ALL OTHER DROPS SHALL HAVE A SLOPE CONSTRUCTED TO DIRECT THE FLOW INTO THE INVERT CHANNEL.
5. SEWER SERVICES SHALL BE CAST IRON IF 2 1/2 FEET OF GROUND COVER CAN NOT BE MAINTAINED OVER THE SERVICE LINE.
6. ALL DUCTILE IRON SEWER PIPE, WHERE SPECIFIED, SHALL BE CLASS 54 AND COMPLY WITH AASHTO C-692/CSO 402 AND ANSI/AWWA C-692/CSO 402. DUCTILE IRON PIPE SHALL BE PROVIDED WITH A COATING APPLIED THROUGH RIGIDITY TESTING IN ACCORDANCE WITH ANSI/AWWA C-692.
7. THE MAXIMUM NUMBER OF DIPS IS ONE INCH DIP PER 135 LF OF PIPE, NO MORE THAN ONE DIP PER 135 LF OF PIPE. A DIP IS DEFINED AS A CHANGE IN ELEVATION OF .05' OR GREATER.

TYPE "C" MANHOLE



INSIDE PVC DROP CONNECTION TO MANHOLE



NOTE: ONE COAT OF ASPHALTIC PAINT

TYPE "B" MANHOLE

TYPE "A" MANHOLE

Scale:	
Project Mgr:	NG
Designed by:	NG
Drawn by:	BM
QA/QC:	AG

Certificate of Authorization No. 9332

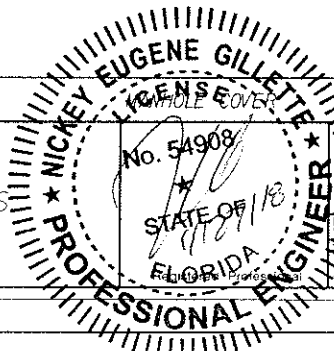
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G & A
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
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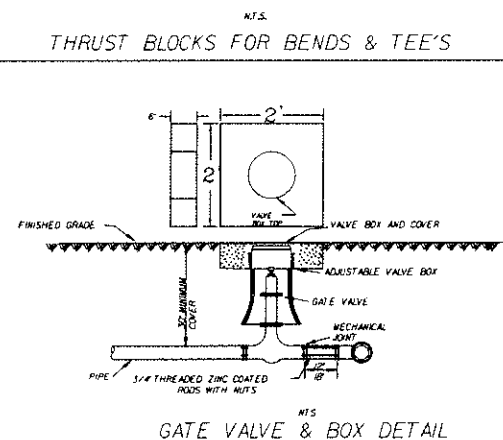
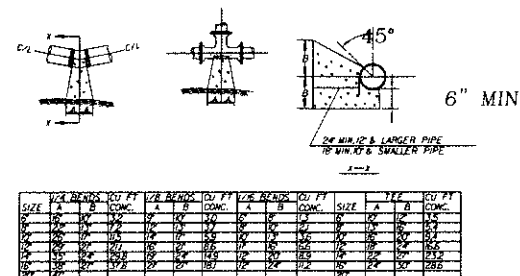
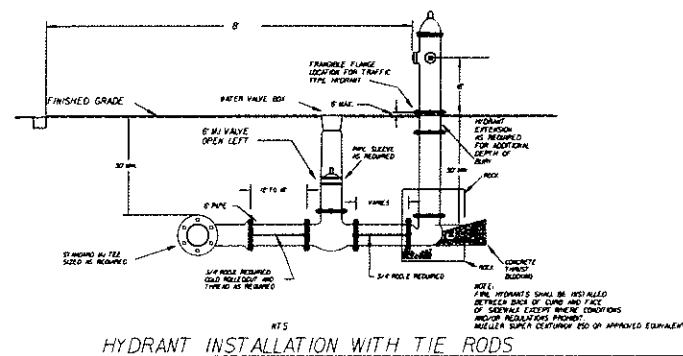
THE COMPASS
GROUP

COURTYARD/SPRINGHILL
DUAL BRAND

SEWER DETAILS

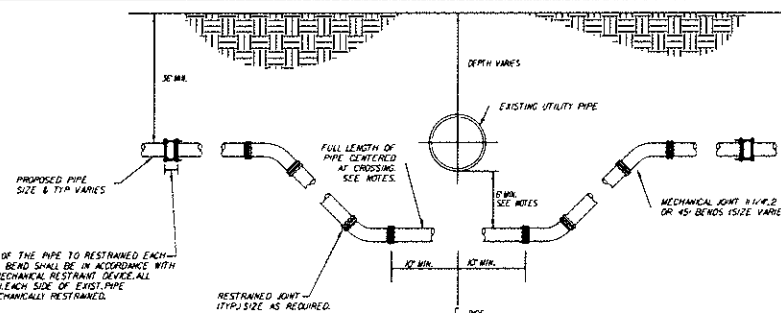


SHEET NO.
DT-2
16 of 18
Issue Date
SEPTEMBER 27, 2018
Project No.
18-10.27



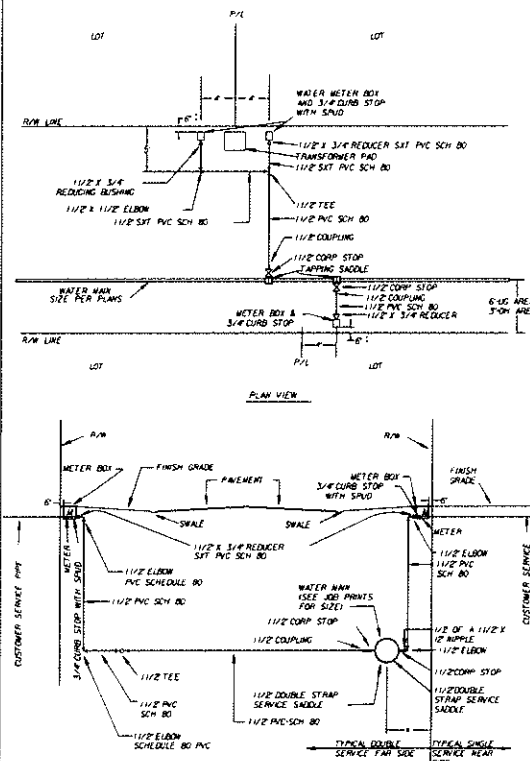
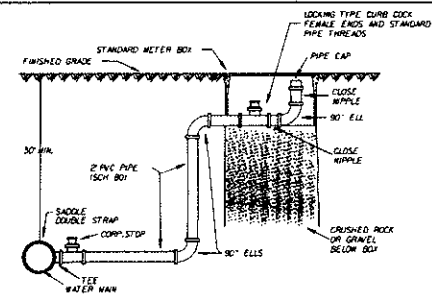
TIE RODS

TWO RODS ON ALL SIX INCH (6") PIPE
THREE RODS ON ALL EIGHT INCH (8") PIPE
FOUR RODS ON ALL TEN AND TWELVE INCH (10" & 12") PIPE

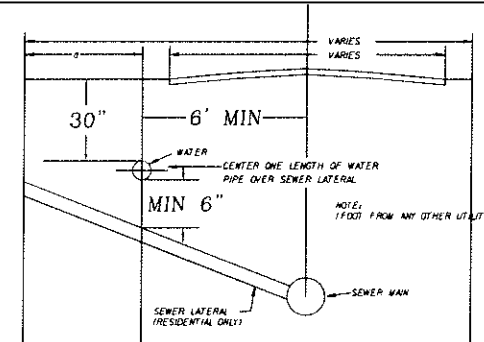


NOTES:

1. IF EXISTING UTILITY PIPE IS A WATER MAIN 6" MINUS OF SEPARATION IS REQUIRED.
2. IF EXISTING UTILITY PIPE IS A FORCE MAIN, SANITARY SEWER LINE OR DRAINAGE PIPE, 18" MINUS OF SEPARATION IS REQUIRED. IF THIS IS IMPOSSIBLE AND EXISTING UTILITY IS OVERHEADS OR DRAINAGE SEPARATION CAN BE REDUCED TO 6" MINUS, A FULL 20 FT. PIPE SHALL BE CENTERED UNDER EXISTING UTILITY MAIN TO PROVIDE MAXIMUM JOINT SPACING FOR BRANCH OR SERVICE LINES.
3. LOCATING WIRING RELOCATE CLOSE TO THE WALL.

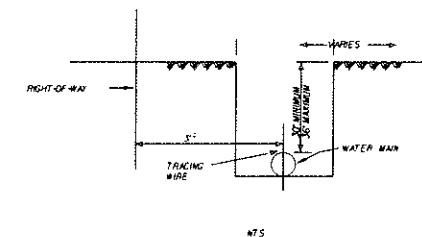
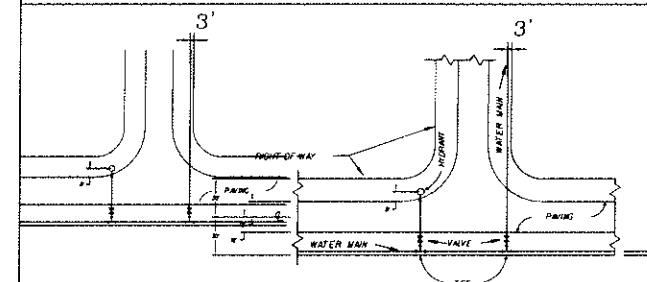


STATE ROAD-11/2" CARRIER PIPE-4" BLACK IRON
CASING FOR JACK AND BORE INSTALLATION.
COUNTY AND CITY-11/2" CARRIER PIPE-3" PVC
CASING FOR JACK AND BORE INSTALLATION.



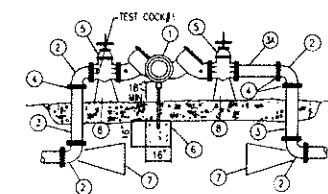
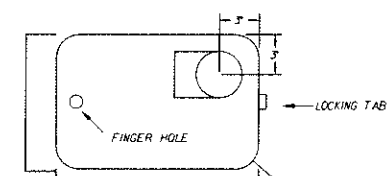
NOTES:

1. MAINTAIN MINIMUM 6' HORIZONTAL SEPARATION BETWEEN WATER AND WASTE WATER WAKNS WHERE THIS IS NOT POSSIBLE REPLACE WASTE WATER MAIN WITH DUCTILE IRON PIPE THE SITUATION IN WHICH THE 10' HORIZONTAL SEPARATION CANNOT BE MAINTAINED (BY OTHERS)
2. MAINTAIN MINIMUM 12" SEPARATION BETWEEN ALL WATER AND WASTE WATER MAINS.
3. MAINTAIN MINIMUM 12" SEPARATION BETWEEN ALL OTHER FACILITIES



WHITE PVC DR 25 C900 PIPE FOR ALL 4,6,8,10" AND 12" MAINS
*10' OF SLD TRACING WIRE TO BE INSTALLED ON TOP OF MAIN TAPED EVERY TEN FEET
PIPE LETTERING TO BE AT TOP OF PIPE WITH EDGE OF JOINT LINE VISIBLE AT BELL

MINIMUM LENGTH TO BE RESTRAINED ON EACH SIDE OF FITTING (FEET)								
PPE SIZE	11 1/4	22 1/2	45	90	REE	VALVE	CAP	REDUCER
4	2	4	7	18	20	39	39	N/A
6	2	5	10	25	20	55	55	28
8	3	6	13	32	20	72	72	30
10	4	8	16	38	20	86	86	29
12	4	9	19	45	20	102	102	50
16	6	11	23	57	20	131	131	30



M A T E R I A L S		
ITEM	QUANT.	DESCRIPTION
1	1	2 1/2" - 10" VALVE DOUBLE CHECK OR REDUCED PRESSURE
2	4	2 1/2" - 10" BEND - 90° F-T
3	2	2 1/2" - 10" D.I.P. (48" LONG) F-P
3A	1	2 1/2" - 10" D.I.P. (24" LONG) F-P (OPT.)
4	3	2 1/2" - 10" FLANGE, D.I.P.
5	2	2 1/2" - 10" VALVE, GATE, D.I. F-T, 6547 (RESILIENT SEAT)
6	1	2' IRON PIPE/CONCRETE FOUNDATION
7	2	THROST BLOCK
8	2	SUPPORT BLOCKS UNDER VALVES (6" AND LARGER SIZES)

LARGE WATER SERVICE
DOUBLE CHECK AND REDUCED PRESSURE
BACKFLOW PREVENTION
SINGLE SERVICE: 2 1/2" - 10"

N.T.S.
BACKFLOW PREVENTER

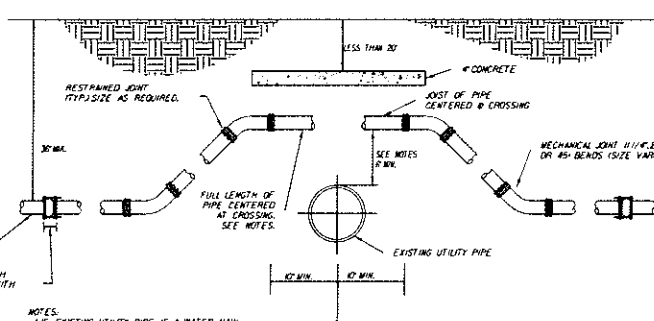
CONTRACTORS ARE REQUIRED TO PAINT NEW HYDRANTS SAFETY YELLOW AND MARK HYDRANTS. THE TOPS AND NOZZLE CAPS SHALL BE PAINTED WITH THE FOLLOWING CAPACITY INDICATOR COLOR SCHEME.

BLUE (SW4079) 1500GPM AND HIGHER
GREEN (SW4085) 1000-1499 GPM
ORANGE (SW4083) 500-999 GPM
RED (SW4081) LESS THAN 500 GPM

IT IS REQUIRED THAT ALL NEW AND RELOCATED WATER MAINS BE PRESSURE TESTED AND LEAKAGE TESTED IN ACCORDANCE WITH AWWA STANDARD C600.

IT IS REQUIRED THAT ALL NEW AND RELOCATED WATER MAINS BE DISINFECTED IN ACCORDANCE WITH AWWA STANDARD C651 AND RULE 62-555.345, F.A.C.

ALL WATER MAINS WILL BE CONSTRUCTED WITH BLUE C900-DR25 PIPE
ALL WATER SERVICES WILL CONSTRUCTED WITH WHITE PVC SCHEDULE
80 PIPE. BLUE SPRAY PAINT WILL BE APPLIED TO THE TOP SURFACE.
JOINT RESTRAINERS MAY BE USED IN PLACE OF THRUST BLOCKS ON C900-DR25 PIPE.
WITH APPROVAL OF THE CITY UTILITIES DEPARTMENT.



NOTES:

1. IF EXISTING UTILITY PIPE IS A WATER MAIN 6-INCHES OF SEPARATION IS REQUIRED.
2. IF EXISTING UTILITY PIPE IS A FORCE MAIN, SANITARY SEWER LINE OR DRAINAGE PIPE, 18 INCHES OF SEPARATION IS REQUIRED. IF THIS IS IMPOSSIBLE AND EXISTING UTILITY MAIN IS 3000.0000 OR D.M.P.P. SEPARATION CAN BE REDUCED TO 6-INCHES. A FULL 20 FT. PIPE SHALL BE CENTERED UNDER EXISTING UTILITY MAIN TO PROVIDE MAXIMUM JOINT SPACING FOR 6-INCH OF GROUND SEPARATION.
3. LOCATING WIRING REQUIRED SEE DETAIL W-44.

UTILITY CONFLICT DETAIL "A"

K			
J			
I			
H			
G			
F			
E			
D			
C			
B			
A			
NG	11-28-18	Revised to address City of F&B/JRWMD comments	
NO	By Date	Revision	

Scale:	
Project Mgr: NG	Gillette & Associates Inc. 20 South 4th Street Fernandina Beach, FL 32034
Designed by: NG	
Drawn by: BM	Certificate of Authorization No. 9332
QA/QC: AG	DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

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FERNANDINA BEACH, FL 32034
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THE COMPASS
GROUP

COURTYARD/SPRINGHILL
DUAL BRAND

WATER DETAILS



WATER SPECIFICATIONS

EUGENE GILLETTE
 1001 W. FERNANDO BEACH
 GAITHERSBURG, MD 20878
 301-251-1111
 301-251-1111

LICENSE
 No. 54998
 SHEET NO. 18
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 EXPIRES 12/27/2010
 12/27/2010



Civil & Environmental Engineering • Mechanical & Structural Engineering • Construction Management

TREE MITIGATION CALCULATIONS

Dual Brand Hotel - Atlantic Avenue Revised 11/9/18

X = REMOVE	TYPE	DIAMETER	COUNT	TOTAL INCHES
	LO	5	2	10
	LO	6	2	12
	LO	7	8	56
	LO	8	10	80
	LO	9	4	36
	LO	10	10	100
	LO	11	7	77
	LO	12	12	144
	LO	13	11	143
	LO	14	8	112
	LO	15	6	90
	LO	16	5	80
	LO	18	2	36
	LO	20	1	20
	LO	22	1	22
	LO	26	1	26
	LOC	35	1	35
	LOC	41	1	41
	LA	7	1	7
	LA	8	2	16
	LA	10	3	30
	LA	11	1	11
	LA	12	4	48
	LA	13	1	13
	LA	14	2	28
	LA	15	1	15
	LA	19	3	57
	LA	20	2	40
	CP	8	3	24
	CP	10	1	10
	CP	16	1	16
	CP	21	1	21
	MG	3	1	3
	MG	7	1	7

MG	20	1	20
MG	27	1	27
			0
WO	7	2	14
WO	8	2	16
WO	14	2	28
WO	18	1	18
WOC	34	1	34
		130	1623

O = KEEP	TYPE	DIAMETER	COUNT	Mitigation Value				<---- mitigation multiplier
				6"-7"	8"-13"	14-20"	>20"	
	LO	6	18	1	1.5	2	3	
	LO	7	5	108				
	LO	8	18	35				
	LO	9	10		216			
	LO	10	11		135			
	LO	11	5		165			
	LO	12	15		82.5			
	LO	13	7		270			
	LO	14	2		136.5			
	LO	15	3			56		
	LO	16	1			90		
	LO	18	5			32		
	LO	19	2			180		
	LO	20	3			76		
	LO	22	1			120		
	LO	25	1				66	
	LO	26	1				75	
	LO	30	1				78	
	LOC						90	
	LA	8	4		48			
		9	1		13.5			
		12	6		108			
		13	1		19.5			
		14	2			56		
		15	2			60		
		16	2			64		
		17	2			68		

	18	1			36		
	19	1			38		
	20	3			120		
	21	1				63	
CA	8	2		24			
CA	10	1		15			
C	8	3		36			
C	13	1		19.5			
	15	1			30		
CP	8	1		12			
MG	12	1		18			
Totals		145		144	1320	1028	375
					total mitigation Value		
					2867		
					1244	EXCESS MITIGATION	



Civil & Environmental Engineering • Mechanical & Structural Engineering • Construction Management

LIFT STATION DESIGN

PROJECT NAME: Courtyard Hotel
 PROJECT NO.: 2012-1020
 DATE: #####

COMPUTATION OF AVERAGE FLOW

(SEE CHARTS ON "FLOW CALULATTIONS" PAGE FOR ADDITIONAL INFORMATION)

TYPE OF DEVELOPMENT	* NO. UNITS	AVERAGE FLOW-GPD	PER UNIT	TOTAL G.P.D.	GPM.
SINGLE FAMILY RESIDENTIAL	0	350	UNIT	0	0.00
HOTEL W/ RESTAURANT	239	100	RESTROOM	23900	16.60
SHOPPING CENTER	0	0.10	Sq.Ft	0	0.00
OFFICE	0	0.14	Sq.Ft	0	0.00
WAREHOUSES	0	60	UNIT	0	0.00
INDUSTRIAL	0	20	SQ. FT.	0	0.00
CHURCH	0	3	PER SEAT	0	0.00
OVERNIGHT PARK	0	150	UNIT	0	0.00
FIXTURE UNITS	0	13.9	PER DAY	0	0.00
METER READING		0	PER DAY	0	0.00

AVERAGE DAILY & PEAK FLOW CALCULATION TOTAL SYSTEM FLOW = 17 GPM

(TOTAL GPD/MIN. IN DAY = GPM FLOW)

* RUN OFF TIME= 18 HRS

26.03 G.P.M

(AMOUNT OF HOURS PER DAY FLOW IS EXPECTED)

PEAK FACTOR (CAL.)= 4.50

*USE PEAK FACTOR= 4

TOTAL PEAK FLOW (TPF)= 26.03 X 4.00 104 G.P.M.

*USE DESIGN FLOW=Q 100 G.P.M. (EACH STATION)

DISCHARGE LINE SIZE:

MAXIMUM FLOW IN PIPE:	180 G.P.M.x	Cft.	Min.	=CF/SEC.
		.48 Gals	60 Sec	-
MAXIMUM FLOW IN PIPE:	0.401 CF/SEC			

PIPE DIAMETER			CROSS-SECT. AREA	FLOW VELOCITY	FLOW	
*1.	4.00	Inch	Pipe	0.087 Sq.Ft	4.59 Ft./Sec.	0.401
2.	6	Inch	Pipe	0.196 Sq.Ft	2.04 Ft./Sec.	0.401
3.	8	Inch	Pipe	0.349 Sq.Ft	1.15 Ft./Sec.	0.401
4.	10	Inch	Pipe	0.546 Sq.Ft	0.74 Ft./Sec.	0.401

*USE STATION PIPING OF DIAMETER 4 INCHES

*USE STATION FORCE MAIN PIPING OF DIAMETER 4 Inches

DESIGN OF WET WELL:

* USE A CYCLE TIME OF 'T' = 12 Minutes
TPF (Peak Flow) 104 G.P.M..

STORAGE REQUIRED = - $T \times TPF = 12 \times 104 = 1249.37 / 7.48 = 167.0$ CU. FT.
GAL. GAL. PER CU. FT.

CALCULATION OF STORAGE VOLUME

DIAMETER OF WET WELL (FT.)	3.00	4.00	5.00	6.00	8.00	10.00
STORAGE HEIGHT (FT.)	23.62	13.29	8.50	5.91	3.32	2.13

* USE WET WELL DIAMETER: 8.00 Feet
STORAGE HEIGHT 3.32 Feet

EL. (FT.)	* GRADE	* DISC.	* INFLU.	* FLOAT SEP.	* PUMP HT.	BOT. **	MIN. DEPTH REQ. "
	17.5	14.5	13	0.5 FT.	2 FT.	6.18	11.82
** DEFAULT IS 2' BELOW 'ALL OFF'						* USE BASIN DEPTH OF (FT.): <u>12.00</u>	

DIM. (IN)	* TOP	* SIDES	* BOT.	* FLANGE
	0.01	0.01	12	12
* TOP ABOVE GRADE(FT)	0.50			

TOP EL. 18.00

GRADE EL. 17.50 vVwv/ \vvVv

(OUTLINE OF
WET WELL)

EFFLUENT (F.M.) EL. 14.50

INFLUENT EL.. 13.00

ALARM EL. 12.50

"LAG PUMP ON" EL. (FT.) 12.00

"LEAD PUMP ON" EL. (FT.) 11.50

"PUMPS OFF" EL. (FT.) 8.18

FLANGE WIDTH (IN.) 9 "

BOTTOM EL. (FT.) 6.00

BOTTOM PAD THICKNESS: 12.00 "->

11 ' 0 "

0 DEG. ANGLE

CHECK STABILITY OF WET WELL:

ASSUME :	Unit Weight of soil	=	90	LBS. PER CU. FT.
	Unit Weight of Concrete/F/G	=	150	LBS. PER CU. FT.
	Ground Water Elevation at Wet Well	=	17.50	(DEFAULT GRADE)

WET WELL :	Thickness of Top Cover	8 "	=	0.67 Ft.
	Thickness of Wall	9 "	=	0.75 Ft.
	Thickness of Bottom Pad	12 "	=	1.00 Ft.
	Height of Wet Well		=	11.50 Ft.
*	Thickness of Insert	0 "	=	0.00 Ft.

SELF WEIGHT OF WET WELL

Top Cover	OD	9.50	FLANGE	9.00	IN.	150	=	8252	LBS
Walls	Pi x (	9.50	) x	0.75	x	11.50	x	150	= 38612 LBS
Bottom Pad	Pi x (	5.50	)^2 x	1.00	x	150	=	14255	LBS
Soil Over xtns:	r=	5.50	R=	5.5000	h=	11.50	=		
Vol. over xtns.	1082 cu.ft	Vol. of wet well	815	x	90 LBS			24013	INCLUDES ANGLE CELL X47
Insert	n x (	4.00	)^2 x	0.00	x	150	=	0	
Total Downward Pressure								=	85131 LBS.

UPLIFT FORCE

Uplift Force	Pi x (	4.75	)^2 x	62.4	x	12.50	=	55288	Lbs.
--------------	--------	------	-------	------	---	-------	---	-------	------

FACTOR OF SAFETY

		<u>Downward Pressure</u>	85131		
Factor of Safety	=	<u>Uplift Force</u>	55288	=	1.54

REQUIRED WEIGHT IN CU. FT.. OF CONCRETE FOR 1.1 SAFETY FACTOR=

0 cu. ft

FORCE MAIN ELEVATIONS AND CONNECTIONS PRESSURES

STATION DISCHARGE EL.	14.50 FEET	MAXIMUM ELEVATION	14.50
*HIGHEST POINT IN LINE	14.50 FEET	*#CONNECTION POINT PRESSURE (FT.)	7.00
		(STATION ONLY)	
*CONNECTION POINT EL.	11.00 FEET	*#CONNECTION POINT PRESSURE (FT.)	58.00
		(ALL STATIONS ON)	

#INCLUDED IN TOTAL STATIC HEAD CALCULATION

STATIC HEAD:	STATION ONLY	STATIC HEAD:	ALL STATIONS ON
MAXIMUM ELEVATION IN LINE=	14.50	MAXIMUM ELEVATION IN LINE=	14.50
PUMP OFF ELEVATION=	8.18	PUMP OFF ELEVATION=	8.18
CONNECTION POINT PRESSURE=	7.00	CONNECTION POINT PRESSURE=	58.00
TOTAL STATIC HEAD=	13.32	TOTAL STATIC HEAD=	64.32

LIFT STATION FOR :
PROJECT No. :

Courtyard Hotel
2012-1020
SYSTEM PROFILE
(TO BE DRAWN BY HAND)

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- - -

COMPUTATION OF EQUIVALENT PIPE LENGTH

LINE LENGTH:	STATION			
PIPE DIA. (IN.)=	4	*HAZEN-WILLIAMS "C" VALUE =		140
4 " Piping @ C =	140			100
2 x Std. 90 elbow		@ 11.00 =		22.00
1 x Gate Valve open		@ 2.30 =		2.30
1 x Swing Check Valve		@ 33.60 =		33.60
1 x Tee - Branch Flow		@ 22.00 =		22.00
VERTICAL PIPE + 7' FOR HORZ. RUN		@ 13.32157 =		13.32
TOTAL STATION :	4 " Pipe Equivalent @	C = 140 =		193.22 L/F

*** SECTION 1:**

PIPE DIAMETER (IN.)=	12	*HAZEN-WILLIAMS "C" VALUE =		140
12" Piping @ C =	140	@ =		0.00*
* 0 x STANDARD ELBOW 90		@ 32 =		0.00
* 0 x MED RADIUS ELBOW 90		@ 26 =		0.00
* 0 x LONG RADIUS ELBOW		@ 20 =		0.00
* 0 x STANDARD ELBOW 45		@ 15 =		0.00
* 0 x TEE- BRANCH FLOW		@ 66 =		0.00
* 0 x TEE- THRU FLOW		@ 16.7 =		0.00
* 0 x GATE VALVE OPEN		@ 6.7 =		0.00
* 0 x SWING CHECK VALVE		@ 49.7 =		0.00
TOTAL SECT. 1 FORCE MAIN	12 " Pipe Equivalent @	C = 140 =		0.00 L/F

***SECTION 2:**

*PIPE DIAMETER (IN.)	16	*HAZEN-WILLIAMS "C" VALUE=		150
16" Piping @ C =	150	@ =		0.00
* 0 x STANDARD ELBOW 90		@ 32 =		0.00 *
* 0 x MED RADIUS ELBOW 90		@ 26 =		0.00
* 0 x LONG RADIUS ELBOW		@ 20 =		0.00
* 0 x STANDARD ELBOW 45		@ 15 =		0.00
* 0 x TEE- BRANCH FLOW		@ 66 =		0.00
* 0 x TEE- THRU FLOW		@ 16.7 =		0.00
* 0 x GATE VALVE OPEN		@ 6.7 =		0.00
* 0 x SWING CHECK VALVE		49.7 =		0.00
TOTAL SECT. 2 FORCE MAIN	16 " Pipe Equivalent @	C = 150 =		0.00 L/F

***SECTION 3:**

*PIPE DIAMETER (IN.)	20	*HAZEN-WILLIAMS "C" VALUE=		130
20 " Piping @ C =	130	@ =		0.00*
* 0 x STANDARD ELBOW 90		@ 32 =		0.00
* 0 x MED RADIUS ELBOW 90		@ 26 =		0.00
* 0 x LONG RADIUS ELBOW		@ 20 =		0.00
* 0 x STANDARD ELBOW 45		@ 15 =		0.00
* 0 x TEE- BRANCH FLOW		@ 66 =		0.00
* 0 x TEE- THRU FLOW		@ 16.7 =		0.00
* 0 x GATE VALVE OPEN		@ 6.7 =		0.00
* 0 x SWING CHECK VALVE		@ 49.7 =		0.00
TOTAL SECT. 3 FORCE MAIN	20 "Pipe Equivalent @	C= 130		0 L/F

***ADDITIONAL LOSSES NOT ACCOUNTED FOR ABOVE**

0.00 Feet

LIFT STATION FOR:

Courtyard Hotel

PROJECT No.

2012-1020

PAGE 6 OF 6

* G.P.M. Increment

5

SYSTEM CURVE:

* PUMP MC

HEAD LOSS TABLE

NP 3127

PD. (IN)	PIPE FRICTION LOSSES:				STATION ONLY		ALL STATIONS ON		* CURVE #
	STATION 4	F.M. 1 12	F.M. 2 16	F.M. 3 20	STATIC HEAD (FEET)	TOTAL HEAD (FEET)	STATIC HEAD	TOTAL HEAD (FEET)	
PIPE (FT.)	193	0	0	0					
GPM									* TDH @
0	0.0	0.0	0.0	0.0	13.3	13.3	64.3	64.3	76.0
5	0.0	0.0	0.0	0.0	13.3	13.3	64.3	64.3	
10	0.0	0.0	0.0	0.0	13.3	13.3	64.3	64.3	
15	0.0	0.0	0.0	0.0	13.3	13.4	64.3	64.4	
20	0.1	0.0	0.0	0.0	13.3	13.4	64.3	64.4	
25	0.1	0.0	0.0	0.0	13.3	13.4	64.3	64.4	
30	0.1	0.0	0.0	0.0	13.3	13.5	64.3	64.5	
35	0.2	0.0	0.0	0.0	13.3	13.5	64.3	64.5	
40	0.2	0.0	0.0	0.0	13.3	13.6	64.3	64.6	
45	0.3	0.0	0.0	0.0	13.3	13.6	64.3	64.6	
50	0.4	0.0	0.0	0.0	13.3	13.7	64.3	64.7	72.0
55	0.4	0.0	0.0	0.0	13.3	13.7	64.3	64.7	
60	0.5	0.0	0.0	0.0	13.3	13.8	64.3	64.8	
65	0.6	0.0	0.0	0.0	13.3	13.9	64.3	64.9	
70	0.7	0.0	0.0	0.0	13.3	14.0	64.3	65.0	
75	0.8	0.0	0.0	0.0	13.3	14.1	64.3	65.1	
80	0.8	0.0	0.0	0.0	13.3	14.2	64.3	65.2	
85	0.9	0.0	0.0	0.0	13.3	14.3	64.3	65.3	
90	1.1	0.0	0.0	0.0	13.3	14.4	64.3	65.4	
95	1.2	0.0	0.0	0.0	13.3	14.5	64.3	65.5	
100	1.3	0.0	0.0	0.0	13.3	14.6	64.3	65.6	68.1
105	1.4	0.0	0.0	0.0	13.3	14.7	64.3	65.7	
110	1.5	0.0	0.0	0.0	13.3	14.9	64.3	65.9	
115	1.7	0.0	0.0	0.0	13.3	15.0	64.3	66.0	
120	1.8	0.0	0.0	0.0	13.3	15.1	64.3	66.1	
125	1.9	0.0	0.0	0.0	13.3	15.3	64.3	66.3	
130	2.1	0.0	0.0	0.0	13.3	15.4	64.3	66.4	
135	2.2	0.0	0.0	0.0	13.3	15.6	64.3	66.6	
140	2.4	0.0	0.0	0.0	13.3	15.7	64.3	66.7	
145	2.5	0.0	0.0	0.0	13.3	15.9	64.3	66.9	
150	2.7	0.0	0.0	0.0	13.3	16.0	64.3	67.0	65.0
155	2.9	0.0	0.0	0.0	13.3	16.2	64.3	67.2	
160	3.1	0.0	0.0	0.0	13.3	16.4	64.3	67.4	
165	3.2	0.0	0.0	0.0	13.3	16.6	64.3	67.6	
170	3.4	0.0	0.0	0.0	13.3	16.7	64.3	67.7	
175	3.6	0.0	0.0	0.0	13.3	16.9	64.3	67.9	
180	3.8	0.0	0.0	0.0	13.3	17.1	64.3	68.1	
185	4.0	0.0	0.0	0.0	13.3	17.3	64.3	68.3	
190	4.2	0.0	0.0	0.0	13.3	17.5	64.3	68.5	
195	4.4	0.0	0.0	0.0	13.3	17.7	64.3	68.7	
200	4.6	0.0	0.0	0.0	13.3	17.9	64.3	68.9	61.0

CONDITIONS AT DESIGN POINT

100	1.3	0.0	0.0	0.0	13.3	14.6	64.3	65.6
-----	-----	-----	-----	-----	------	------	------	------

TYPE OF ESTABLISHMENT	ESTIMATED FLOW PER 24 HOUR PERIOD	FIXTURE UNIT CALCULATION
SINGLE FAMILY HOMES		B/ROOM GRP.(LAV., TUB/SHOW. B/ROOM GRP.(LAV., TUB/SHOW. BATHTUB W/1-1/2"TRAP BATHTUB W/2"TRAP BIDET W/1-1/2" TRAP DENTAL UNIT DRINKING FOUNTAIN D/WASHER HOME KITCHEN SINK HOME K/SINK HOME W/DISPOSAL LAVATORY WITH 1-1/2" PLUG LAVATORY BARBER/BEAUTY SI
a) Two bedrooms	350	
b) Three bedrooms	450	
c) Four bedrooms	500	
d) Five bedrooms	550	
APARTMENT HOUSE		
a) One bedroom	200	
b) Two bedroom	300	
c) Three bedroom	400	
MOBILE HOME PARK, per unit	175 (250 possible)	LAUNDRY TRAY 2- COMP. SHOWER STALL SHOWER GROUP (PER HEAD) SINK (DIRECT FLUSH VALVE)
MOTELS, per unit	50	
MOTEL W/RESTAURANT	100	
LAUNDRIES (COIN OPERATED), per machine	400	SINK (SERVICE TYPE WITH FLOW
OFFICE BUILDING, per employee	20	SINK (SCULLERY) SINK (SURGEONS) URINAL (WITH FLUSH VALVE) URINAL (FLUSH TANK) WATER CLOSET(FLUSH VALVE) WATER CLOSET(FLUSH TANK) SWIMMING POOL (PER 1000 GAL UNLISTED FIXTURE (1-1/4" TRAI UNLISTED FIXTURE (1-1/2" TRAI UNLISTED FIXTURE (2" TRAP) UNLISTED FIXTURE (2-1/2" TRAI UNLISTED FIXTURE (3" TRAP) UNLISTED FIXTURE (4" TRAP)
SCHOOLS, per pupil		
Day, cafeteria and showers	20	
Day, cafeteria or lunch room	15	
Boarding	75	
SERVICE STATIONS		
First bay	1000	
Each additional bay	500	
CHURCHES, per sanctuary seat	7	WATER SOFTENER (DOMESTIC) WASHING MACHINE
RESTAURANTS, (including toilets)		
a) 24 hr., per seat	50	
b) Not 24 hr., per seat	35	
c) Bars and cocktail lounge, per seat	35	
d) Drive-ins, per car space	50	
FAST FOOD	30	
HOSPITALS, per bed	200	AVERAGE DAILY FLOW FROM METER READING CU FT DAYS IN BILLING CYCLE GALS PER DAY

NURSING AND REST HOMES, per person	100
CAMPS	
a) Day (no meals), per person	20
b) Luxury, private bath, per person	100
c) Labor, per person	75
d) Youth and recreation, per person	50
VACATION COTTAGES	50
INSTITUTIONS, per resident	100
THEATERS	
Indoor, per seat	3
Outdoor, per seat	5
SHOPPING CENTERS	
a) Per square foot - floor space (No food service or laundry)	0.1
FACTORIES, per person per shift	15
PICNIC PARKS (with floor toilets), per person	10
AIRPORT, per passenger	3

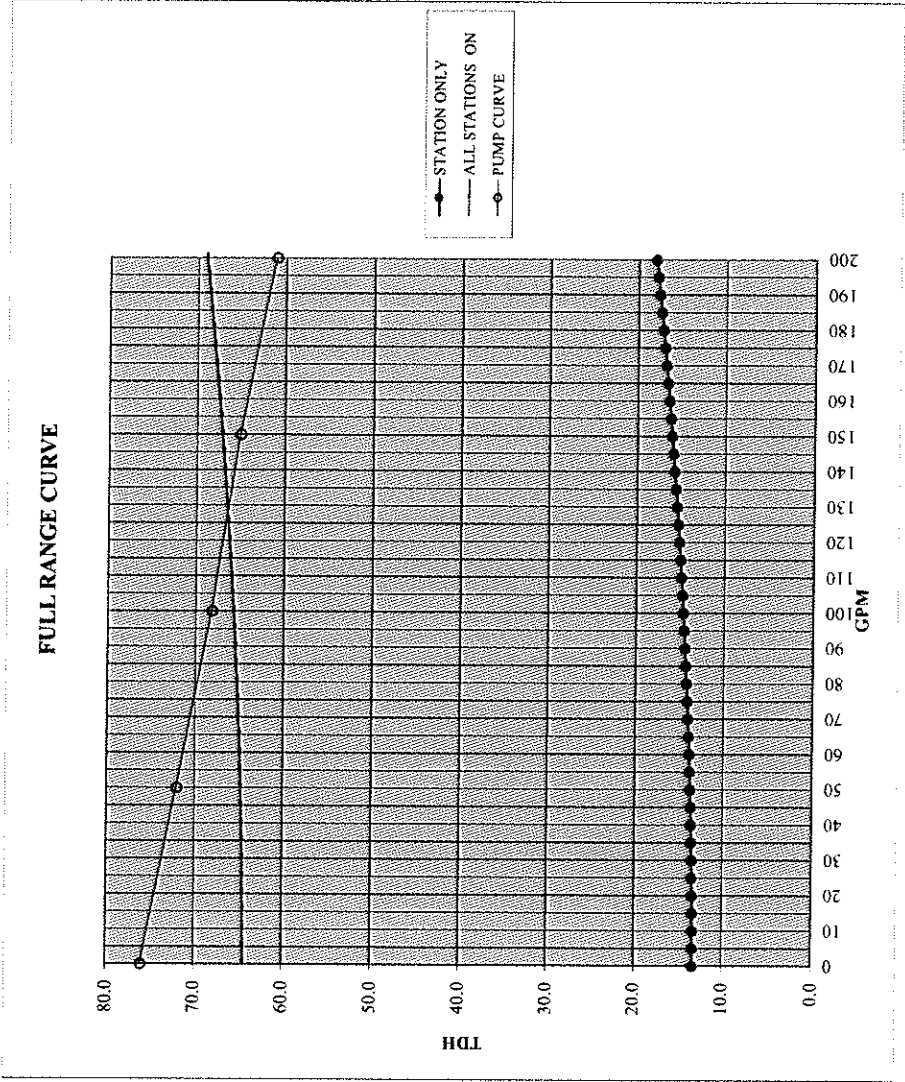
	QTY	UNIT VAL.	TOTAL
, FLUSH W/C)	0	8	0
, TANK W/C)	0	6	0
	15	2	30
	0	3	0
	0	3	0
	0	1	0
	0	0.5	0
	2	2	4
	15	2	30
	0	3	0
	15	1	15
HOP	0	2	0
	0	2	0
	0	2	0
	0	3	0
	0	8	0
OR DRAIN)	0	3	0
	0	4	0
	0	3	0
	0	8	0
	0	4	0
	0	8	0
	15	4	60
, CAPACITY)	0	1	0
)	0	1	0
)	0	2	0
	0	3	0
)	0	4	0
	0	5	0
	0	6	0
	0	4	0
	2	2	4
TRANSFER TOTAL TO G24		TOTAL	143

WATER METER

100
28

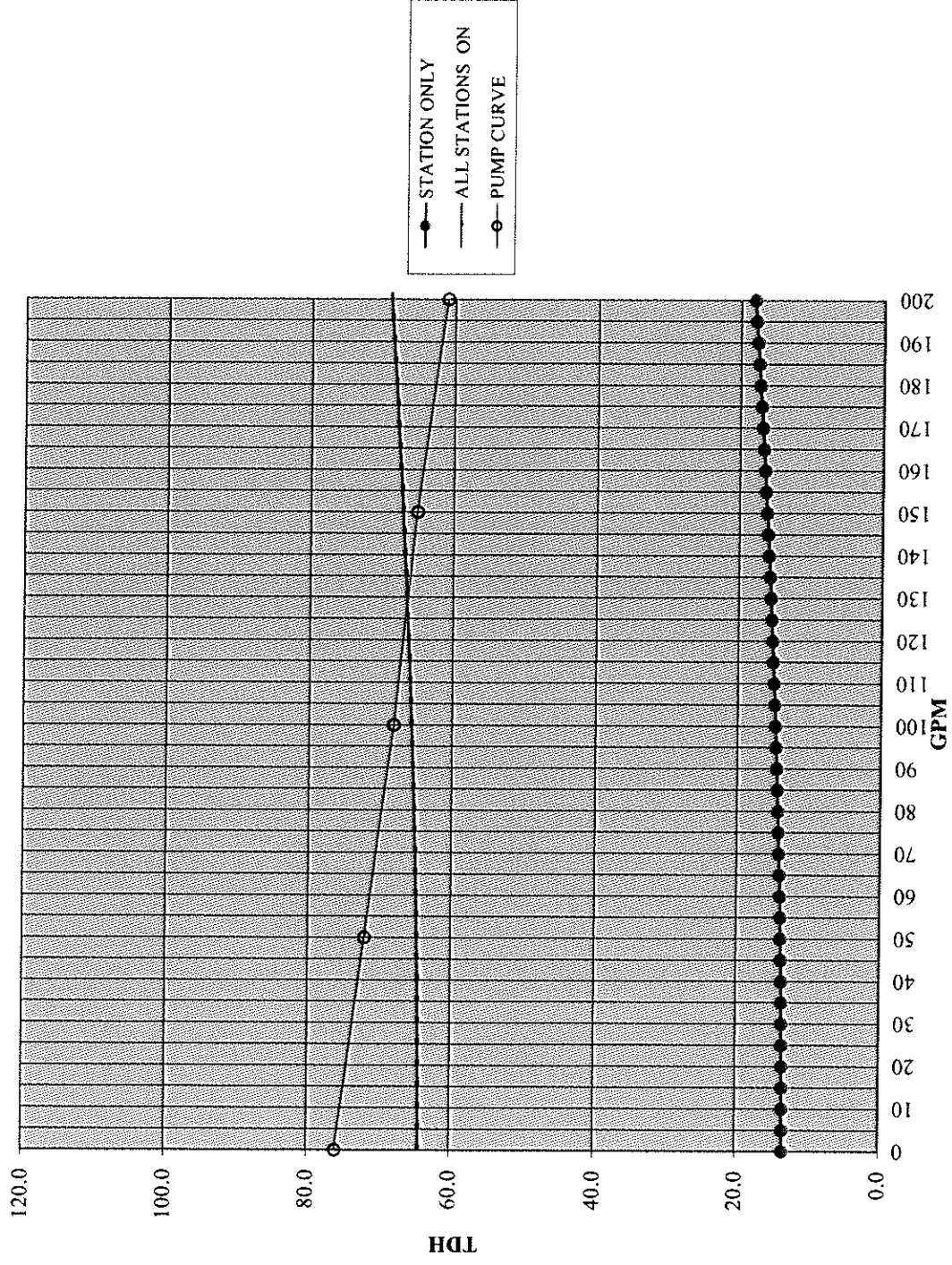
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LIFT STATION DESIGN

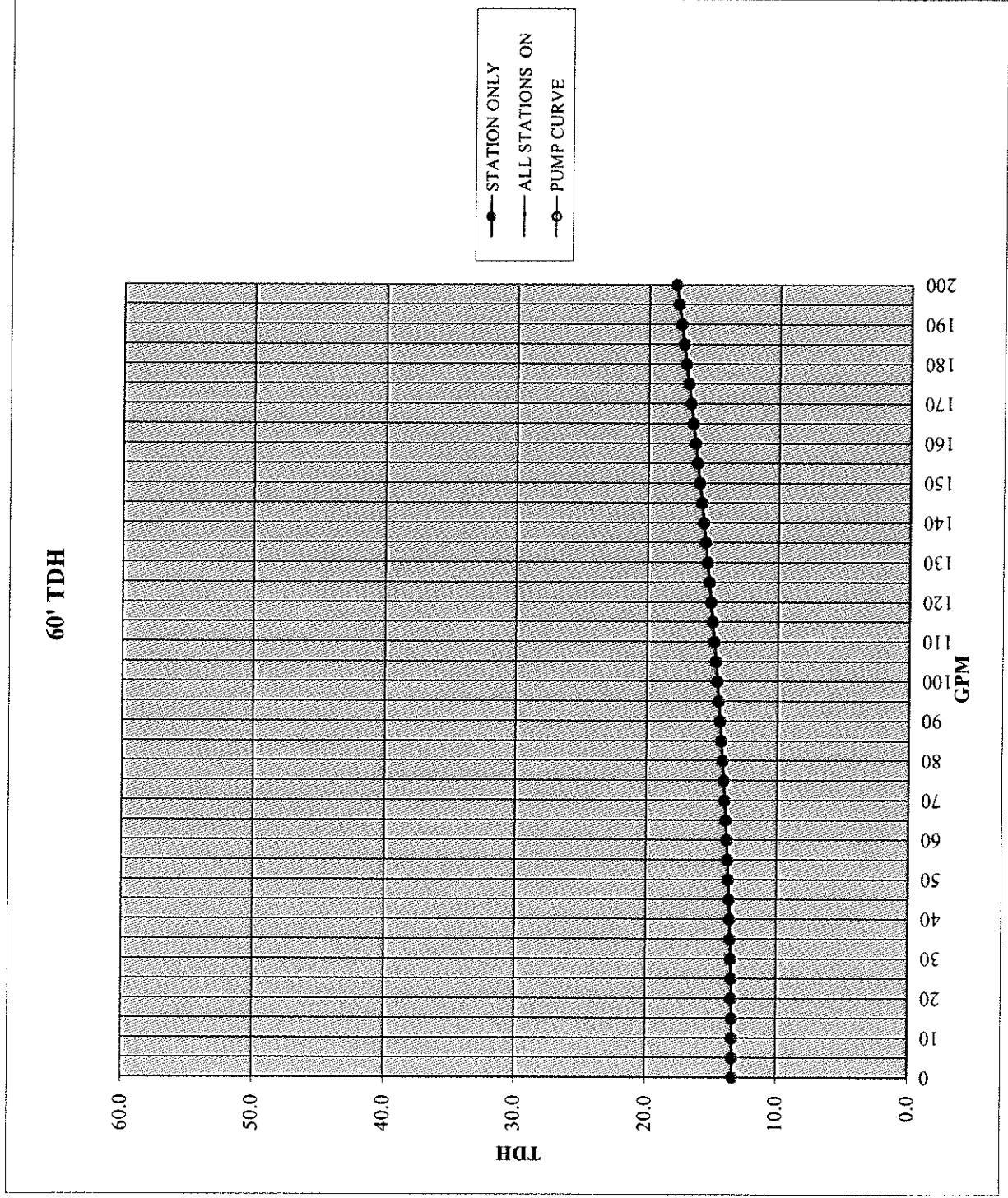


LIFT STATION DESIGN

120' TDH

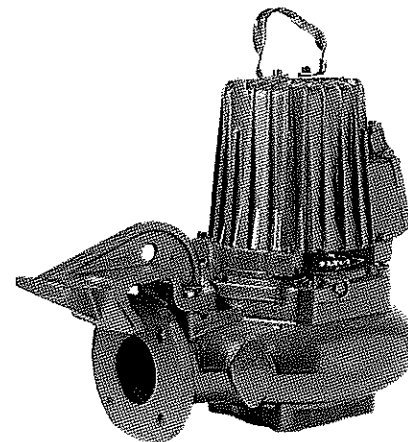
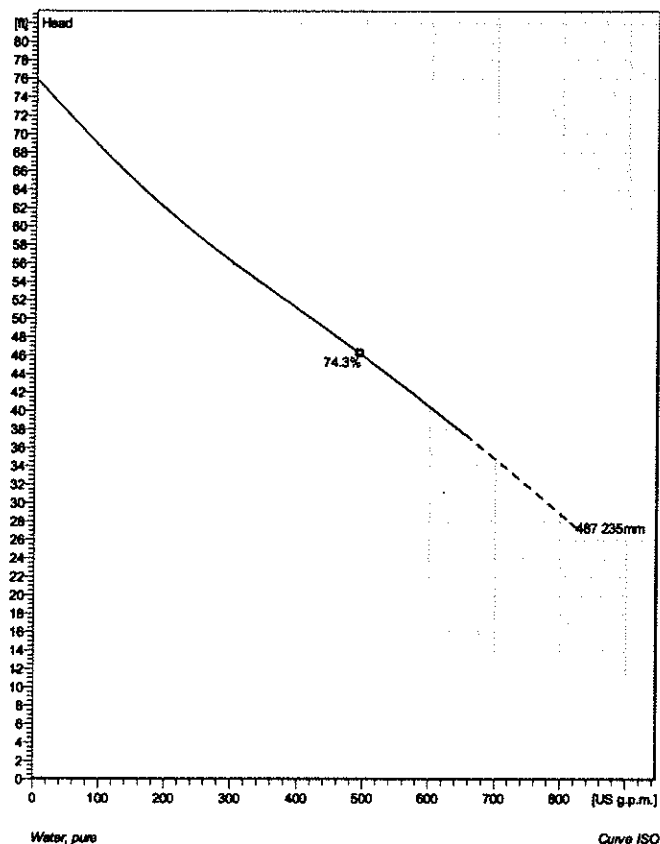


LIFT STATION DESIGN



NP 3127 HT 3~ Adaptive 487

Technical specification



Note: Picture might not correspond to the current configuration.

General

Patented self cleaning semi-open channel impeller, ideal for pumping in waste water applications. Possible to be upgraded with Guide-pin® for even better clogging resistance. Modular based design with high adaptation grade.

Impeller

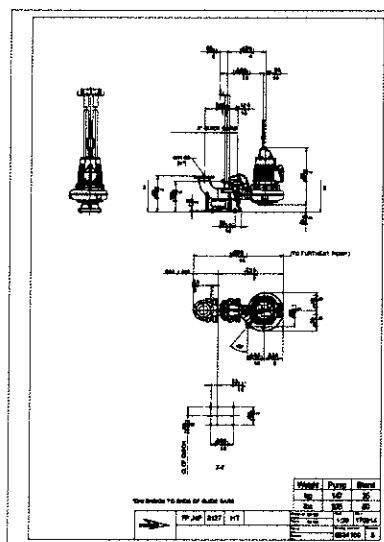
Impeller material	Grey cast iron
Discharge Flange Diameter	3 15/16 inch
Suction Flange Diameter	3 15/16 inch
Impeller diameter	235 mm
Number of blades	2

Motor

Motor #	N3127.901 21-12-4AS-W IE3 6.5KW
Stator variant	Standard
Frequency	50 Hz
Rated voltage	200 V
Number of poles	4
Phases	3~
Rated power	8.7 hp
Rated current	22 A
Starting current	144 A
Rated speed	1500 rpm
Power factor	
1/1 Load	0.92
3/4 Load	0.90
1/2 Load	0.82
Motor efficiency	
1/1 Load	92.0 %
3/4 Load	92.1 %
1/2 Load	90.8 %

Configuration

Installation: P - Semi permanent, Wet



NP 3127 HT 3~ Adaptive 487

Performance curve



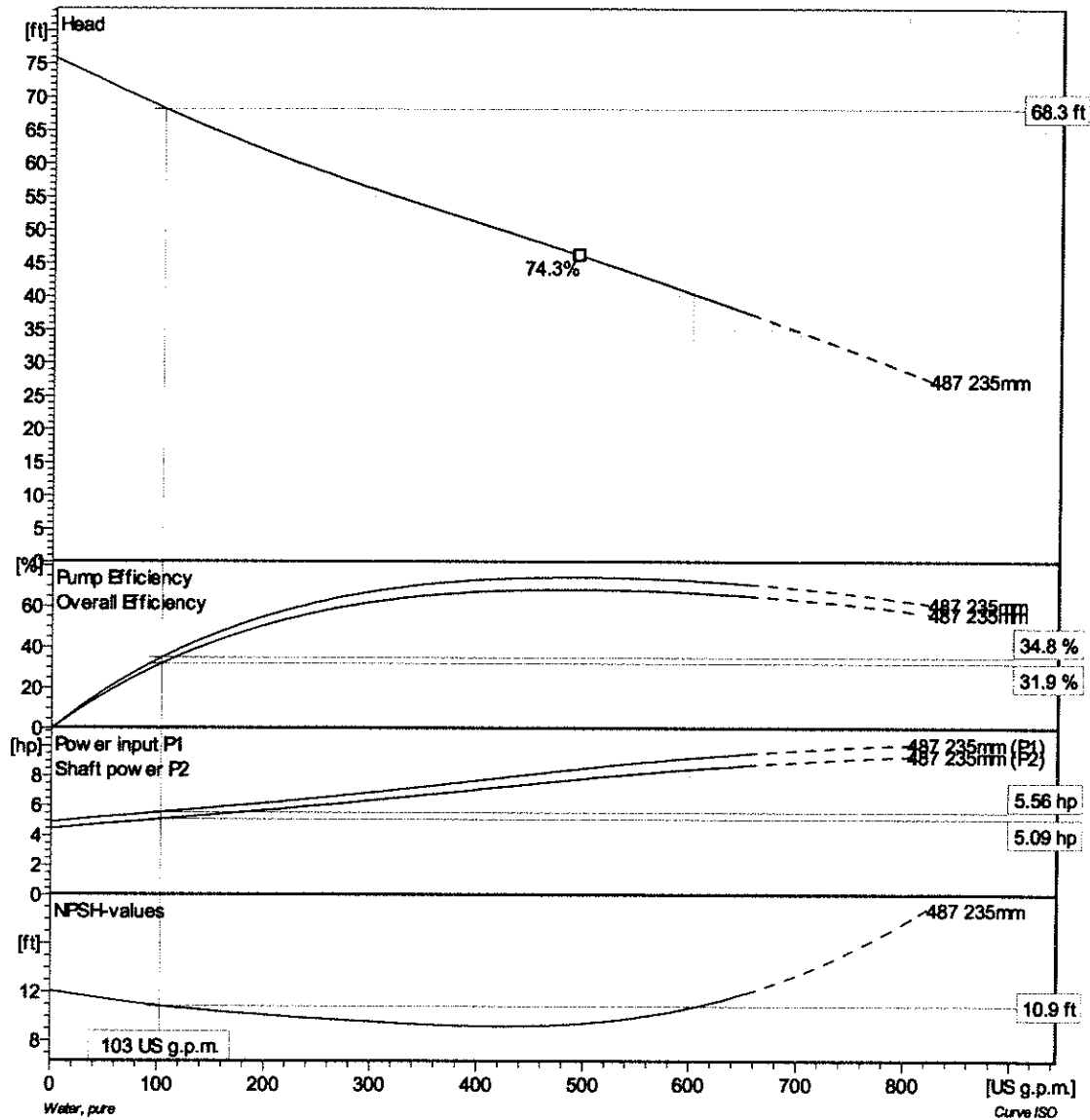
Pump

Discharge Flange Diameter 3 15/16 inch
Suction Flange Diameter 100 mm
Impeller diameter 9 1/4"
Number of blades 2

Motor

Motor # N3127.901 21-12-4AS-W IE3 6.5KW
Stator variant 29
Frequency 50 Hz
Rated voltage 200 V
Number of poles 4
Phases 3~
Rated power 8.7 hp
Rated current 22 A
Starting current 144 A
Rated speed 1500 rpm

Power factor
1/1 Load 0.92
3/4 Load 0.90
1/2 Load 0.82
Motor efficiency
1/1 Load 92.0 %
3/4 Load 92.1 %
1/2 Load 90.8 %



Project

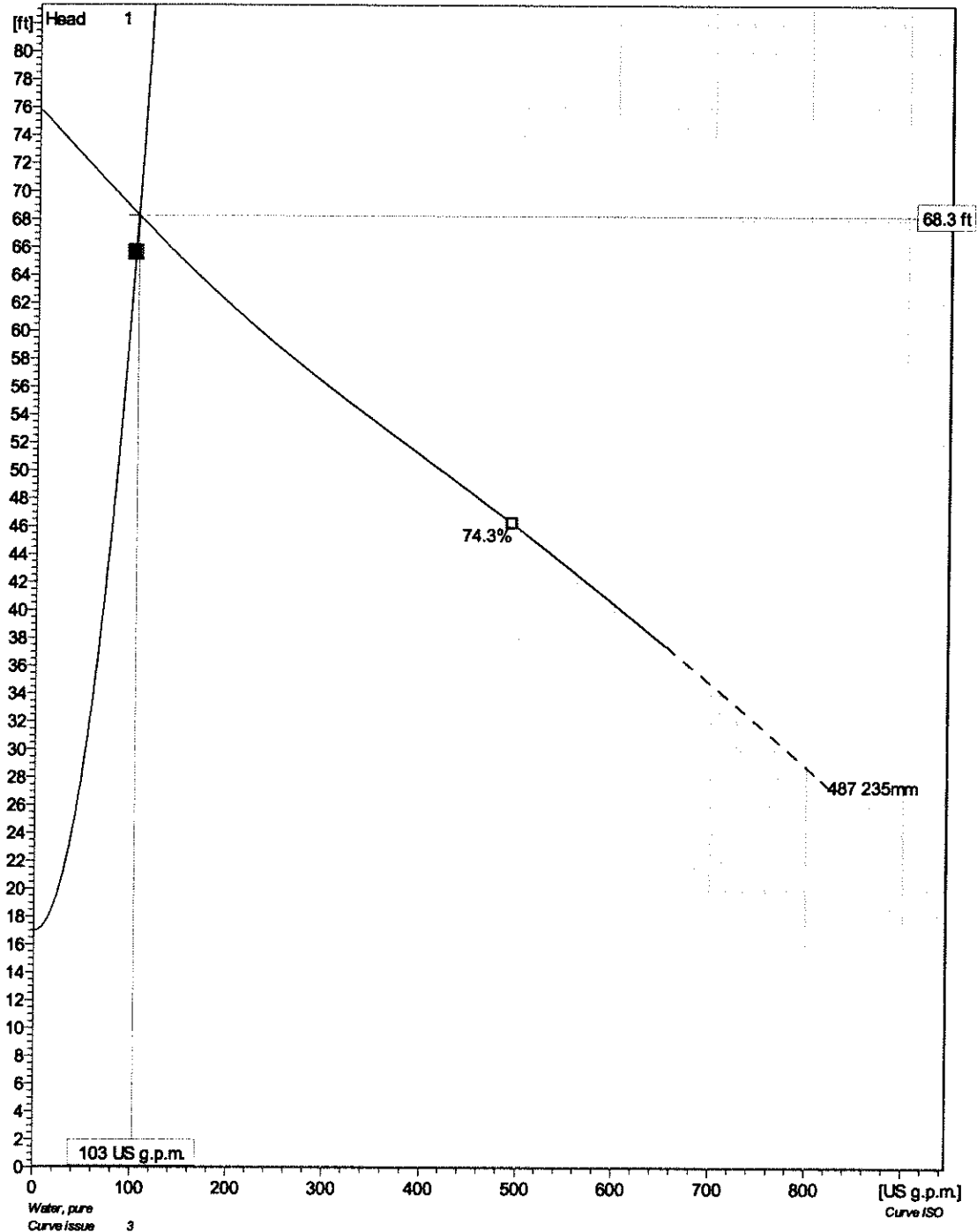
Project ID

Created by

Created on
11/12/2018

Last update

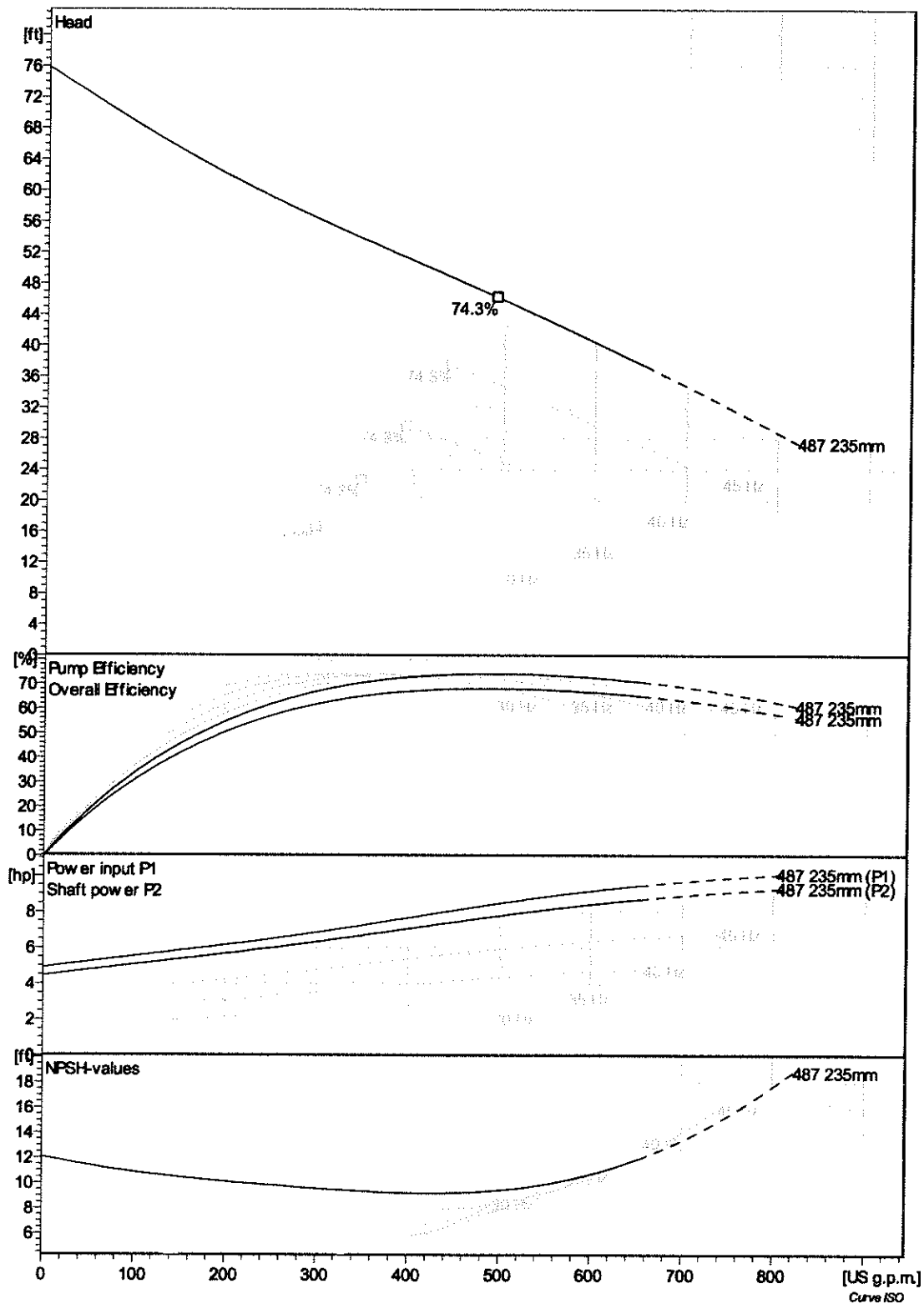
NP 3127 HT 3~ Adaptive 487 Duty Analysis



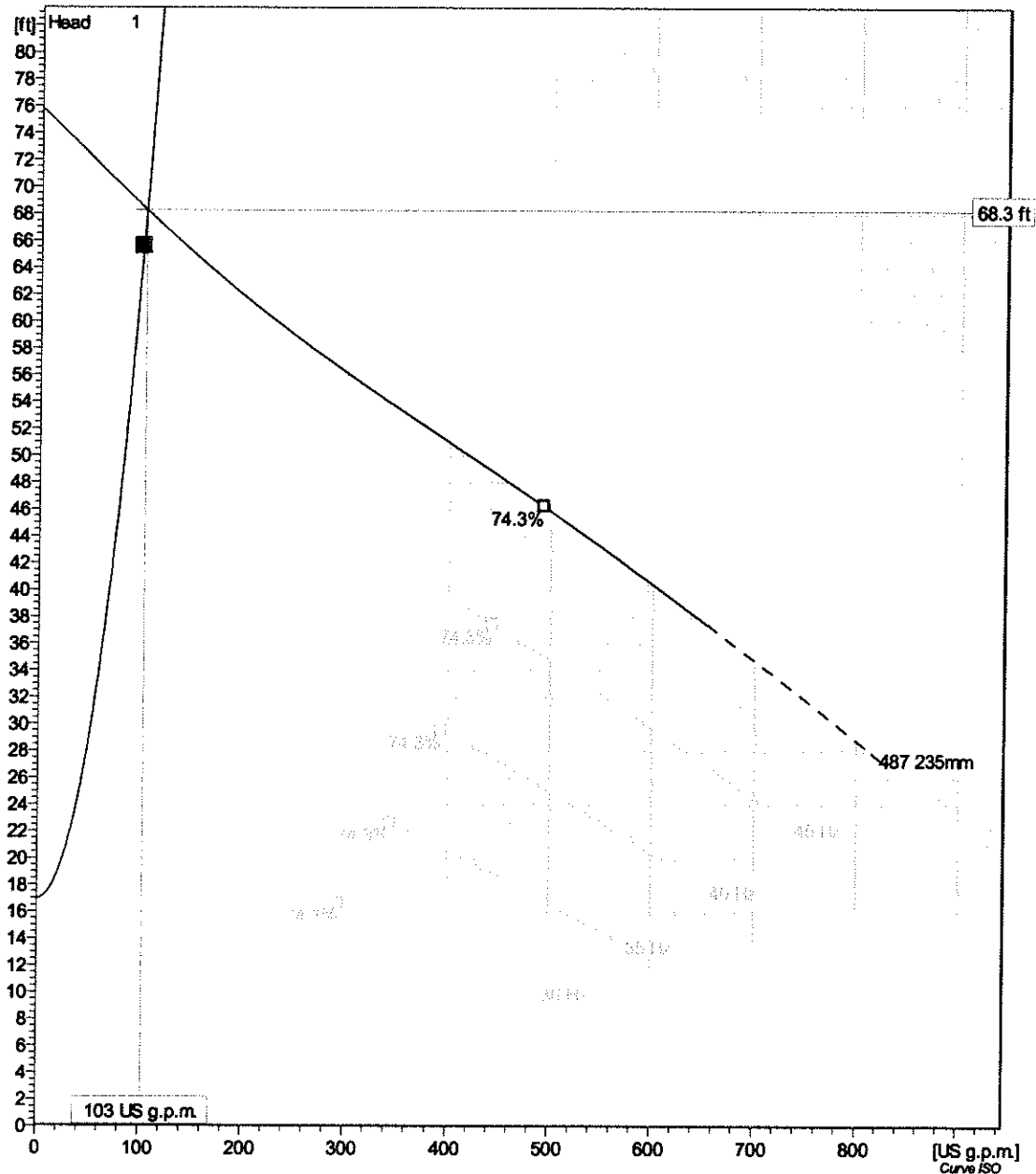
Pumps running /System	Individual pump			Total					NPSH _{re}
	Flow	Head	Shaft power	Flow	Head	Shaft power	Pump eff.	Specific energy	
1	103 US g.p.m.	68.3 ft	5.09 hp	103 US g.p.m.	68.3 ft	5.09 hp	34.8 %	672 kWh/US MG	10.9 ft

Project: XXXXXXXXXX Project ID: XXXXXXXXXX Created by: XXXXXXXXXX Created on: 11/12/2018 Last update: XXXXXXXXXX

NP 3127 HT 3~ Adaptive 487 VFD Curve



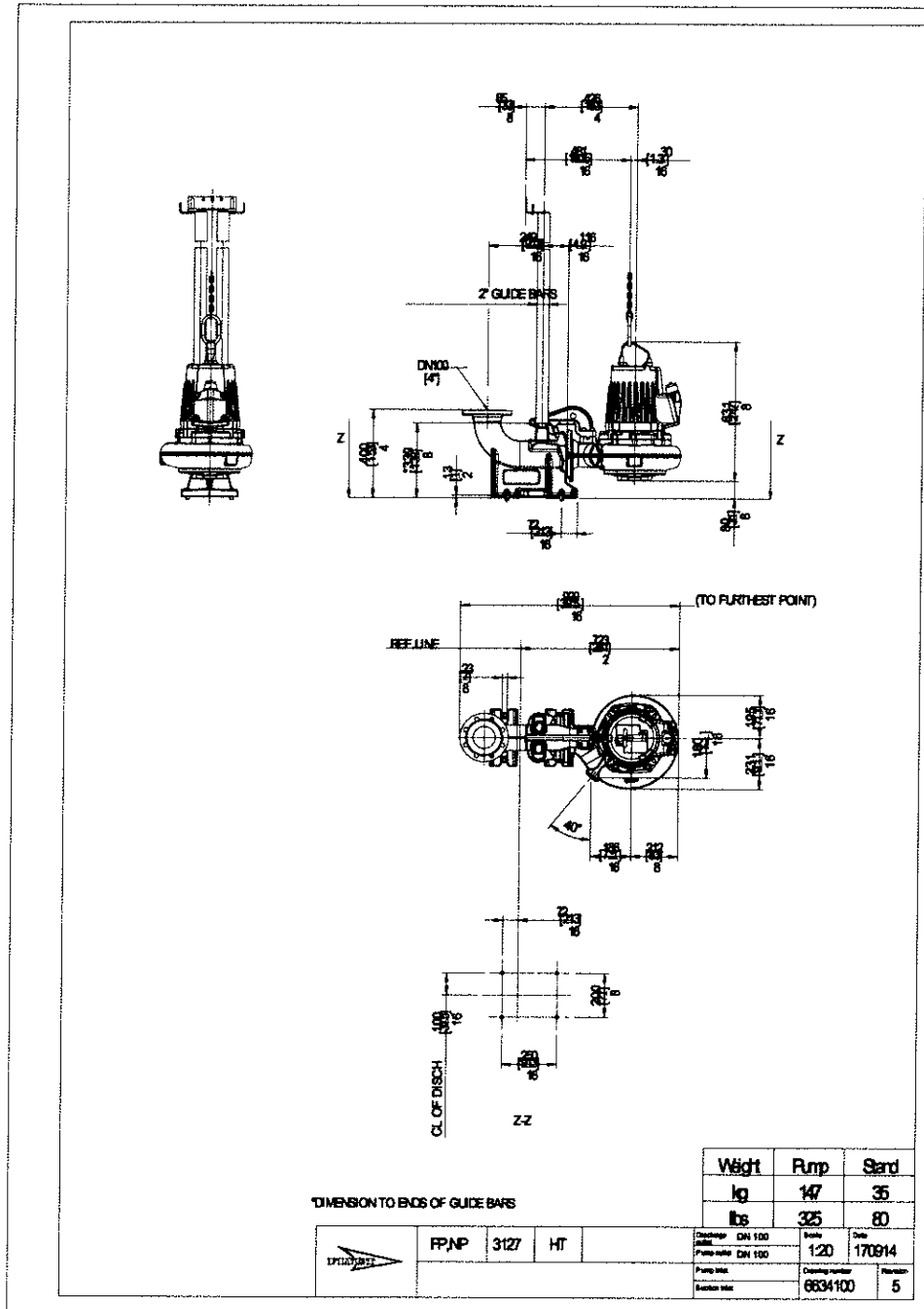
NP 3127 HT 3~ Adaptive 487 VFD Analysis



Pumps running /System	Frequency	Flow	Head	Shaft power	Flow	Head	Shaft power	Hyd. eff.	Specific energy	NPSH _r
1	50 Hz	103 US g.p.m.	68.3 ft	5.08 hp	103 US g.p.m.	68.3 ft	5.08 hp	34.8 %	672 kWh/US MG	10.9 ft
1	45 Hz	89 US g.p.m.	55.5 ft	3.69 hp	89 US g.p.m.	55.5 ft	3.69 hp	33.8 %	577 kWh/US MG	9.21 ft
1	40 Hz	74.7 US g.p.m.	44.1 ft	2.58 hp	74.7 US g.p.m.	44.1 ft	2.58 hp	32.3 %	497 kWh/US MG	7.66 ft
1	35 Hz	59.3 US g.p.m.	34.1 ft	1.71 hp	59.3 US g.p.m.	34.1 ft	1.71 hp	29.9 %	437 kWh/US MG	6.24 ft
1	30 Hz	41.7 US g.p.m.	25.4 ft	1.06 hp	41.7 US g.p.m.	25.4 ft	1.06 hp	25.4 %	421 kWh/US MG	4.95 ft

NP 3127 HT 3~ Adaptive 487

Dimensional drawing



Prepared for:



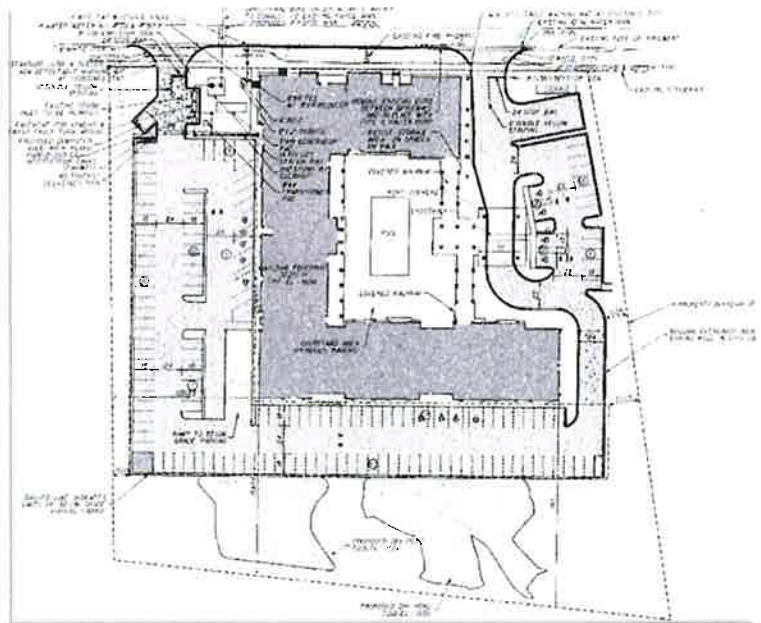
&



Prepared by:



Chindalur Traffic Solutions, Inc.
8833 Perimeter Park Boulevard, Suite 103
Jacksonville, FL 32216
904.619.3368 || 904.422.6923



Atlantic Avenue Hotel FDOT Traffic Study

**City of
Fernandina Beach
Nassau County
Florida**

Project # 1075-180-037
Date: 11/28/2018

PROFESSIONAL ENGINEER CERTIFICATE

I, Rajesh Ramn K. Chindalur, PE #77285, certify that I currently hold an active license in the state of Florida and am competent through education or experience to provide engineering services in the civil discipline contained in this plan, print, specification, or report.

PROJECT:	Atlantic Avenue Hotel – FDOT Traffic Study
LOCATION:	City of Fernandina Beach, Nassau County, Florida
CLIENT:	Compass Group, Inc.

I further certify that this plan, print, specification, or report was prepared by me or under my responsible charge as defined in Chapter 61G15-18.001 F.A.C. Moreover, if offered by a corporation, partnership, or through a fictitious name, I certify that the company offering the engineering services, Chindalur Traffic Solutions, Inc., 8515 Baymeadows Road, Suite 401, Jacksonville, Florida 32256, holds an active certificate of authorization #30806 to provide engineering service.

**Rajesh
Chindalur, P.E.,
PTOE**

Digitally signed by Rajesh Chindalur,
P.E., PTOE
DN: cn=Rajesh Chindalur, P.E., PTOE,
o=Chindalur Traffic Solutions, Inc., ou,
email=Chindalur@ctrfficsolutions.com,
c=US
Date: 2018.11.28 07:38:19 -05'00'

Signature:

Name:

P.E. No.:

Date:

Rajesh Ramn K. Chindalur, P.E., PTOE
77285
11/28/2018

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Introduction:

A hotel development that is anticipated to include 239 rooms and a 2,101 SF restaurant are proposed for construction on the south side of Atlantic Avenue just west of Tarpon Avenue in the City of Fernandina Beach, Nassau County, Florida. **Figure 01** shows the location of the proposed development.

Access to the proposed development will be provided via two driveways on Atlantic Avenue. The western driveway is anticipated to be located just east of Fort Clinch Road and the eastern driveway is anticipated to be located across from Tarpon Avenue. Atlantic Avenue is a Florida Department of Transportation (FDOT) roadway. Hence, a traffic study confirming to FDOT's requirements determining the impact of project traffic on Atlantic Avenue at the proposed project driveways would be required.

Existing Conditions:

Atlantic Avenue is a two-lane roadway with left turn lanes at major intersections/driveways has a posted speed limit of 35 miles per hour (mph). **Figure 02** shows the existing conditions on Atlantic Avenue at the proposed project driveway locations.

Trip Generation:

The Daily, AM and PM Peak trip generation for the proposed development were estimated using equations included in the ITE Trip Generation Manual, 10th Edition. ITE land use code 311 – All Suites Hotel and 932 – High-Turnover Sit-Down Restaurant were used to estimate the trip generation. **Table 01** summarizes the estimated trip generation from the proposed development. As shown in this table, 1,345 daily trips that include 112 AM peak trips (60 entering and 52 exiting) and 113 PM peak trips (57 entering and 56 exiting) are anticipated to be generated by the proposed development.

Data Collection:

The AM and PM peak traffic counts at the following intersections on Atlantic Avenue were obtained on September, 19th 2018 and November, 6th 2018. These turning movements were further adjusted for seasonal variation by applying a season factor of 1.09. These season factors were obtained from the Florida Department of Transportation (FDOT) traffic counts online portal. **Figure 03** shows these peak hour turning movements. A copy of these counts and the FDOT season factors are included as **Attachment B**.

Year 2021 Background Traffic Projections:

The proposed development is anticipated to be built-out by the year 2021. As such, the year 2021 traffic volumes on Atlantic Avenue at the study intersections were estimated by applying a growth factor of 1.030 (1.00% per year for 3 years) to the existing year 2018 peak hour traffic volumes. This growth factor was estimated by performing trends analysis of the historical AADT on Atlantic Avenue. The historical AADT was obtained from the FDOT traffic counts online portal. The trends analysis revealed a negative growth. Hence, a minimum of 1% growth rate was used in this analysis. A copy of the historical AADT and the trends analysis plot are included as **Attachment C**. The year 2021 background conditions AM peak and PM peak hour turning movement volumes at the study intersections are shown in **Figure 04**.

Project Traffic Distribution and Assignment:

Project traffic distribution for the proposed development was estimated based on the existing traffic patterns on Atlantic Avenue. **Figure 05** shows the project traffic distribution percentages on Atlantic Avenue. As shown in this figure, following is a summary of the project traffic distribution percentages:

- 50% oriented to and from the west, 45% oriented to and from the east and 5% oriented to and from the north during the AM peak period
- 55% oriented to and from the west, 40% oriented to and from the east and 5% oriented to and from the north during the PM peak period

The AM peak and PM peak trip generation as shown in **Table 01** was multiplied by the project traffic distribution to determine the AM peak and PM peak traffic assignment on Atlantic Avenue at the project access driveways. Previously stated **Figure 05** shows the AM peak and PM peak hour project traffic assignment on Atlantic Avenue at the project access driveways.

Total Traffic Projections:

The year 2021 total traffic volumes on Atlantic Avenue at the project access driveways include the year 2021 background conditions traffic volumes and the project traffic assignment from the proposed development. **Figure 06** shows the AM peak and PM peak total traffic volumes on Atlantic Avenue at the project access driveways. **Attachment D** includes details of these calculations.

Right Turn Lane Evaluation:

The evaluation to determine the need for eastbound right turn lanes on Atlantic Avenue at both the project access driveways was performed using the criteria and guidance included in the “*Driveway Information Guide*” published by FDOT based on the research published in NCHRP 420 entitled “*Impacts of Access Management Techniques*”. For a two-lane roadway with a posted speed limit of 45 mph or less, 125 right turns per hour are required to warrant an exclusive right turn lane. A copy of this criteria and guidance are included as **Attachment E**. As shown in **Figure 06**, about 15 AM peak & 16 PM peak eastbound right turns are anticipated on Atlantic Avenue at both the project access driveways are anticipated under the build-out conditions of the proposed development. Hence, an eastbound right turn lane on Atlantic Avenue at both the project access driveways is not anticipated to be warranted under the build-out conditions of the proposed development.

Westbound Left Turn Lanes:

As shown in previously stated **Figure 02**, Atlantic Boulevard currently provides for eastbound left turn lanes at Tarpon Avenue and Tarpon Street. Tarpon Avenue and Tarpon Street are closely spaced intersections. Providing westbound left turn lanes on Atlantic Avenue into the proposed project access driveways will require reducing the length of the existing eastbound left turn lanes. However, this reduction will result in left turn lanes with inadequate storage and deceleration distances. Hence, it is recommended that the existing eastbound left turn lanes on Atlantic Avenue be re-striped to provide for two-way-left-turn lanes between western project access driveway and Tarpon Street. **Figure 07** shows the recommended two-way-left-turn lane striping on Atlantic Avenue.

Intersection Capacity Analysis:

The year 2021 build-out conditions delay and LOS on both the project access driveways were determined by performing unsignalized intersection capacity analysis of the project access driveways on Atlantic Avenue. The unsignalized intersection capacity analysis was performed using the Synchro 9 software which uses the HCM 2010 methodologies and procedures in estimating delay and level of service (LOS). **Table 02** summarizes the peak hour delay and LOS at on the proposed project access driveways on Atlantic Avenue under the year 2021 build-out conditions. As summarized in this table, all approaches at the study intersections are anticipated to operate at LOS E or better under the year 2021 build-out conditions of the proposed development. **Attachment F** includes a copy of the HCM worksheets.

Summary and Conclusions:

A hotel development that is anticipated to include 239 rooms and a 2,101 SF restaurant are proposed for construction on the south side of Atlantic Avenue just west of Tarpon Avenue in the City of Fernandina Beach, Nassau County, Florida.

Access to the proposed development will be provided via two driveways on Atlantic Avenue. The western driveway is anticipated to be located just east of Fort Clinch Road and the eastern driveway is anticipated to be located across from Tarpon Avenue.

The proposed development is anticipated to generate 1,345 daily trips that include 112 AM peak trips (60 entering and 52 exiting) and 113 PM peak trips (57 entering and 56 exiting).

The proposed development is anticipated to be built-out by the year 2021. As such, the year 2021 traffic volumes on Atlantic Avenue at the study intersections were estimated by applying a growth factor of 1.030 (1.00% per year for 3 years) to the existing year 2018 peak hour traffic volumes.

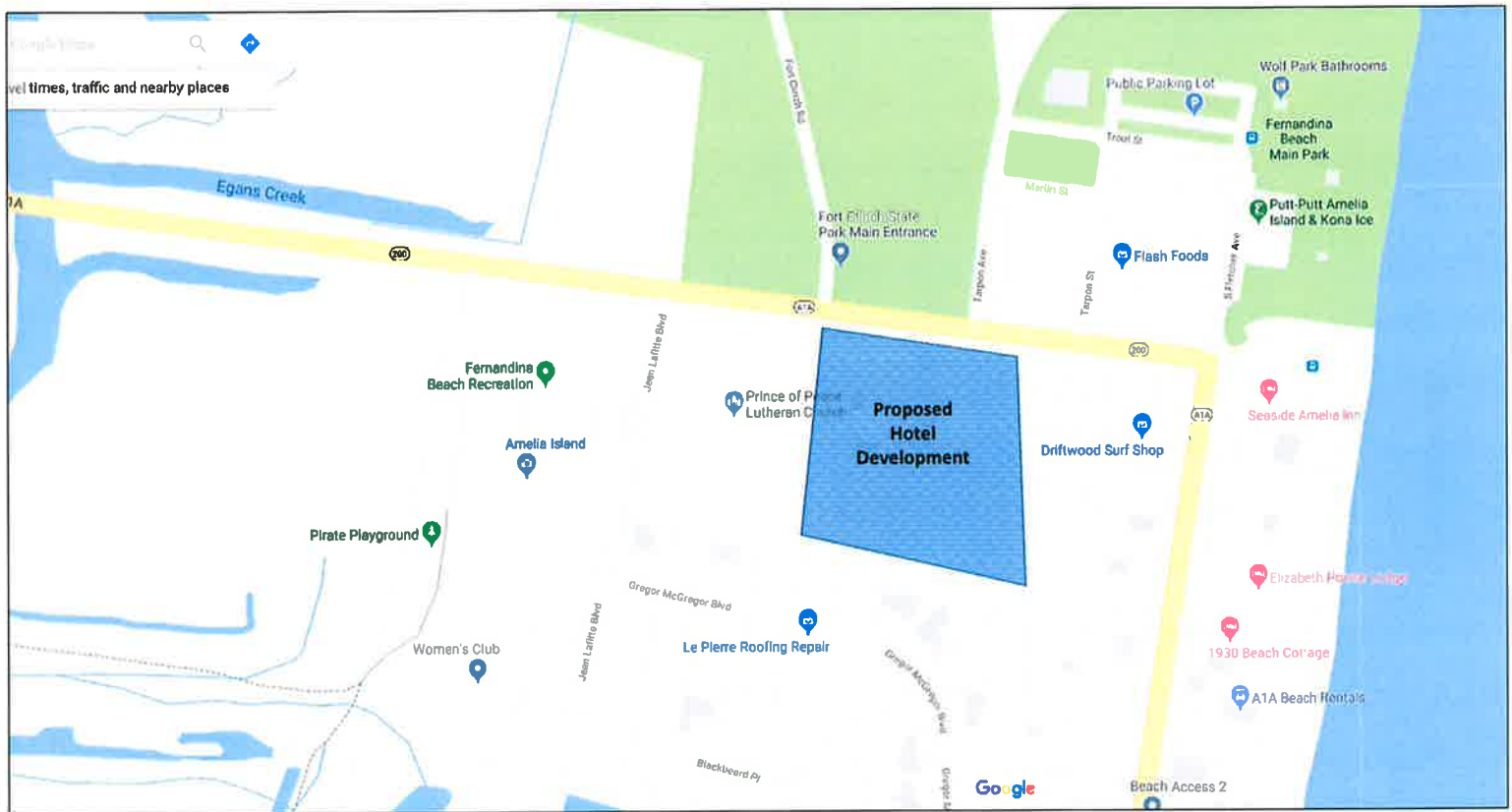
Project traffic distribution for the proposed development was estimated based on the existing traffic patterns on Atlantic Avenue.

- 50% oriented to and from the west, 45% oriented to and from the east and 5% oriented to and from the north during the AM peak period
- 55% oriented to and from the west, 40% oriented to and from the east and 5% oriented to and from the north during the PM peak period

The evaluation to determine the need for eastbound right turn lanes on Atlantic Avenue at both the project access driveways was performed using the criteria and guidance included in the *“Driveway Information Guide”* published by FDOT based on the research published in NCHRP 420 entitled *“Impacts of Access Management Techniques”*. An eastbound right turn lane on Atlantic Avenue at both the project access driveways is not anticipated to be warranted under the build-out conditions of the proposed development.

Tarpon Avenue and Tarpon Street are closely spaced intersections. Atlantic Boulevard currently provides for eastbound left turn lanes at Tarpon Avenue and Tarpon Street. Providing westbound left turn lanes on Atlantic Avenue into the proposed project access driveways will require reducing the length of the existing eastbound left turn lanes. However, this reduction will result in left turn lanes with inadequate storage and deceleration distances. Hence, it is recommended that the existing eastbound left turn lanes on Atlantic Avenue be re-stripped to provide for two-way-left-turn lanes between western project access driveway and Tarpon Street.

The unsignalized intersection capacity analysis of the project access driveways on Atlantic Avenue shows that the all approaches at the study intersections are anticipated to operate at LOS E or better under the year 2021 build-out conditions of the proposed development.



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Figure 01 – Location Map
 Atlantic Avenue Hotel – FDOT Traffic Study
 Nassau County, Florida

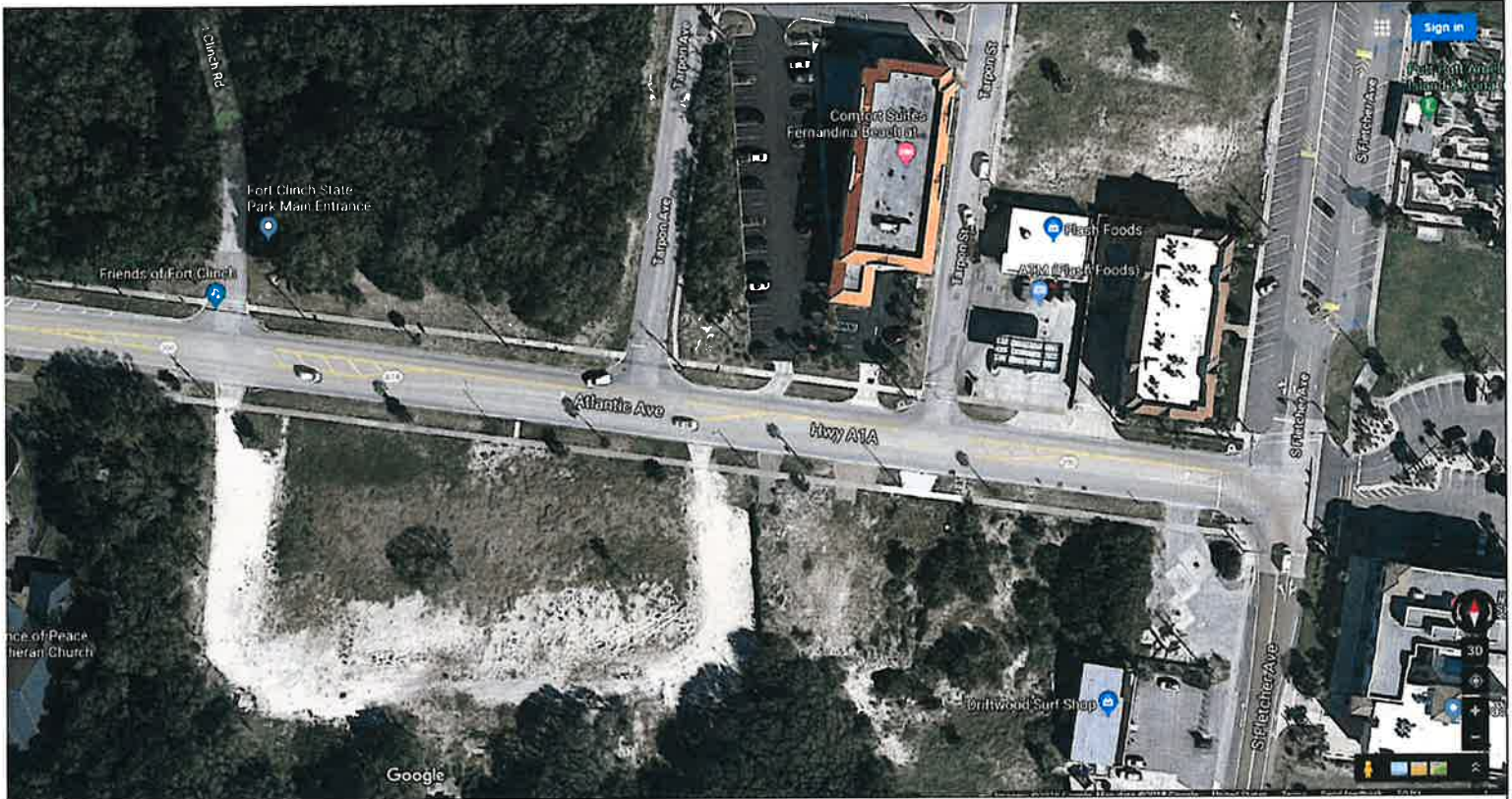
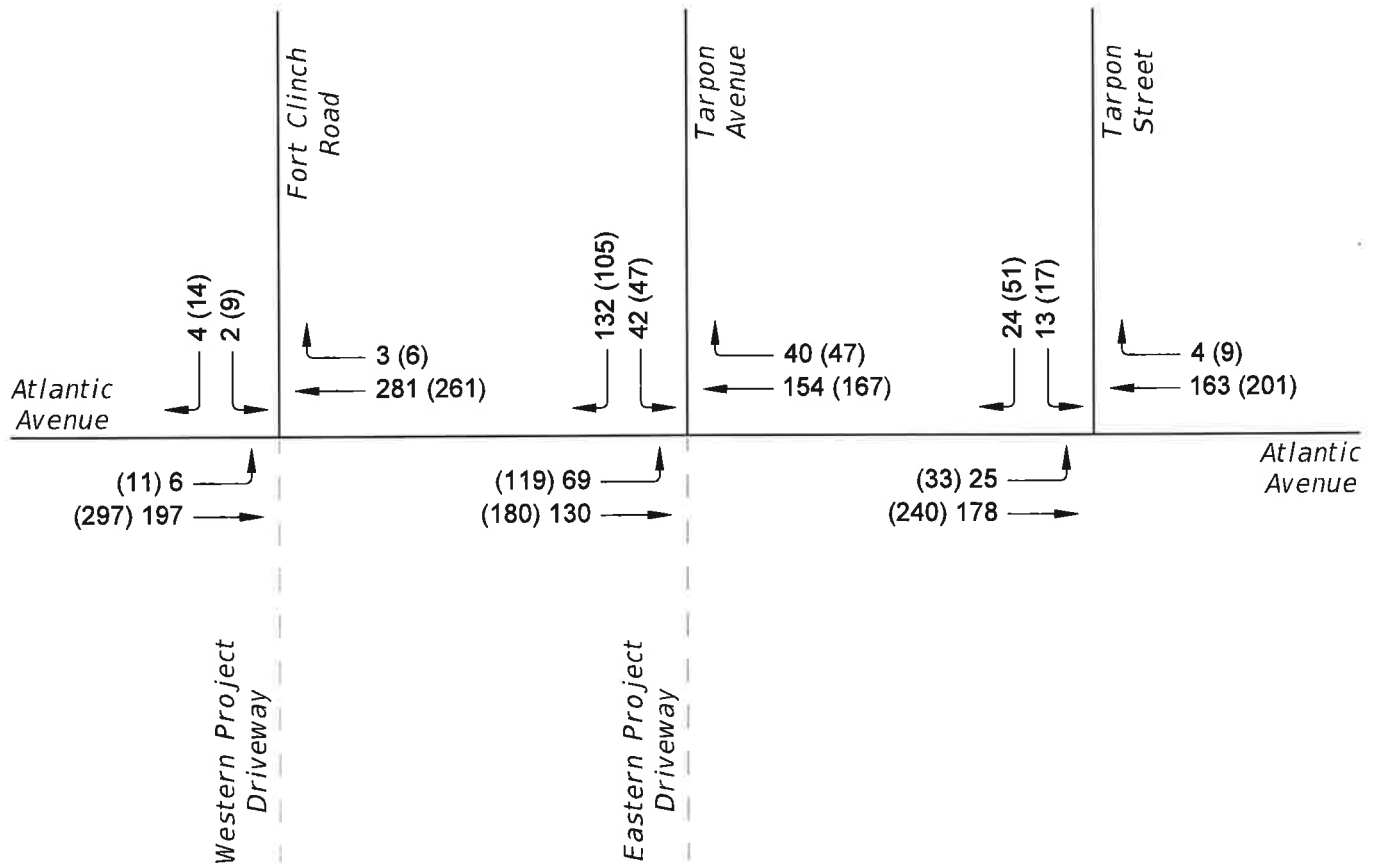


Figure 02 – Existing Conditions on Atlantic Avenue
Atlantic Avenue Hotel – FDOT Traffic Study
Nassau County, Florida



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LEGEND:

154 - AM Peak Hour Traffic
 (167) - PM Peak Hour Traffic

YEAR 2018 TRAFFIC VOLUMES

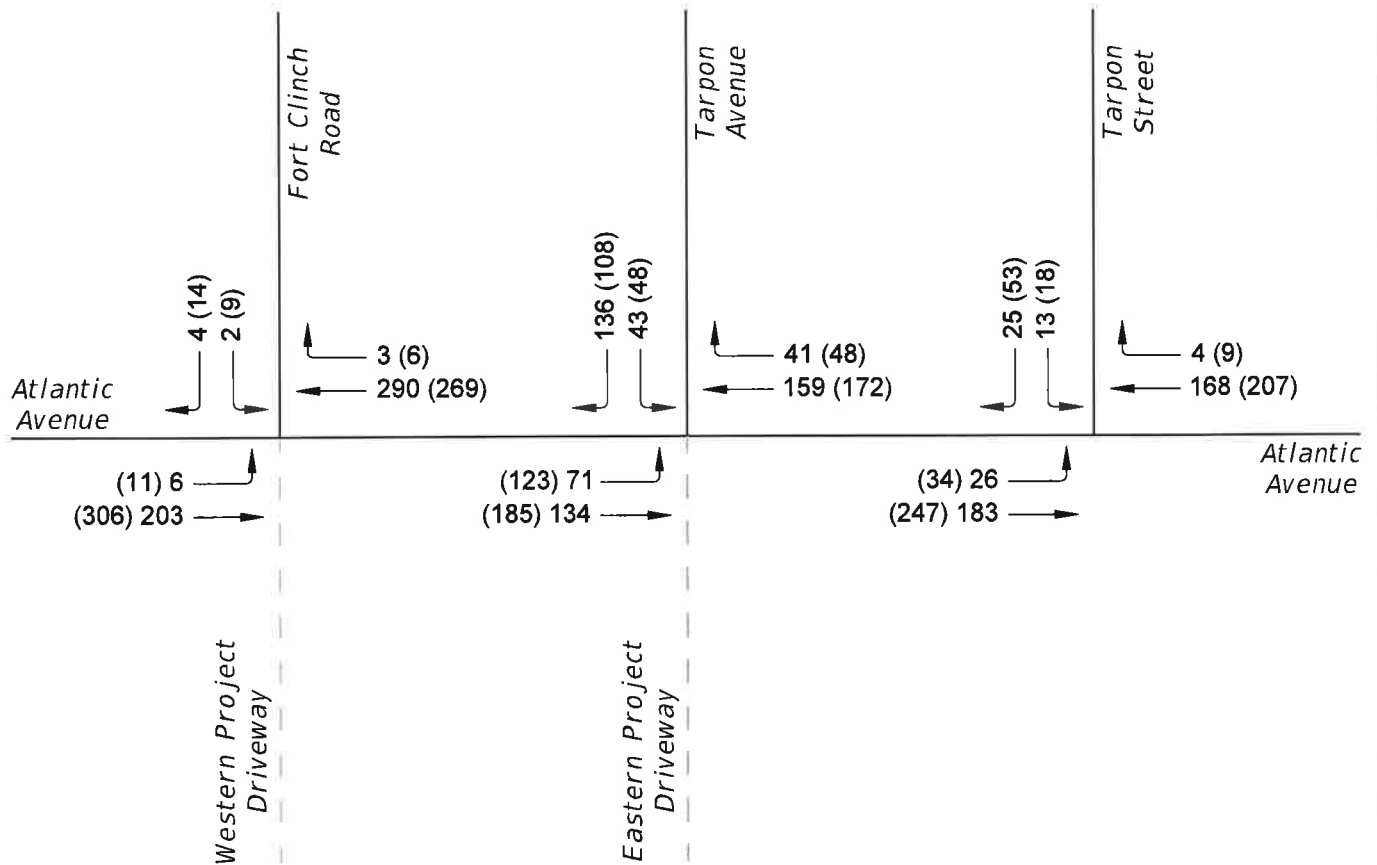


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Atlantic Avenue Hotel
 FDOT Traffic Study
 City of Fernandina Beach

FIGURE

03



LEGEND:

159 - AM Peak Hour Traffic
 (172) - PM Peak Hour Traffic

YEAR 2021 BACKGROUND CONDITIONS TRAFFIC VOLUMES

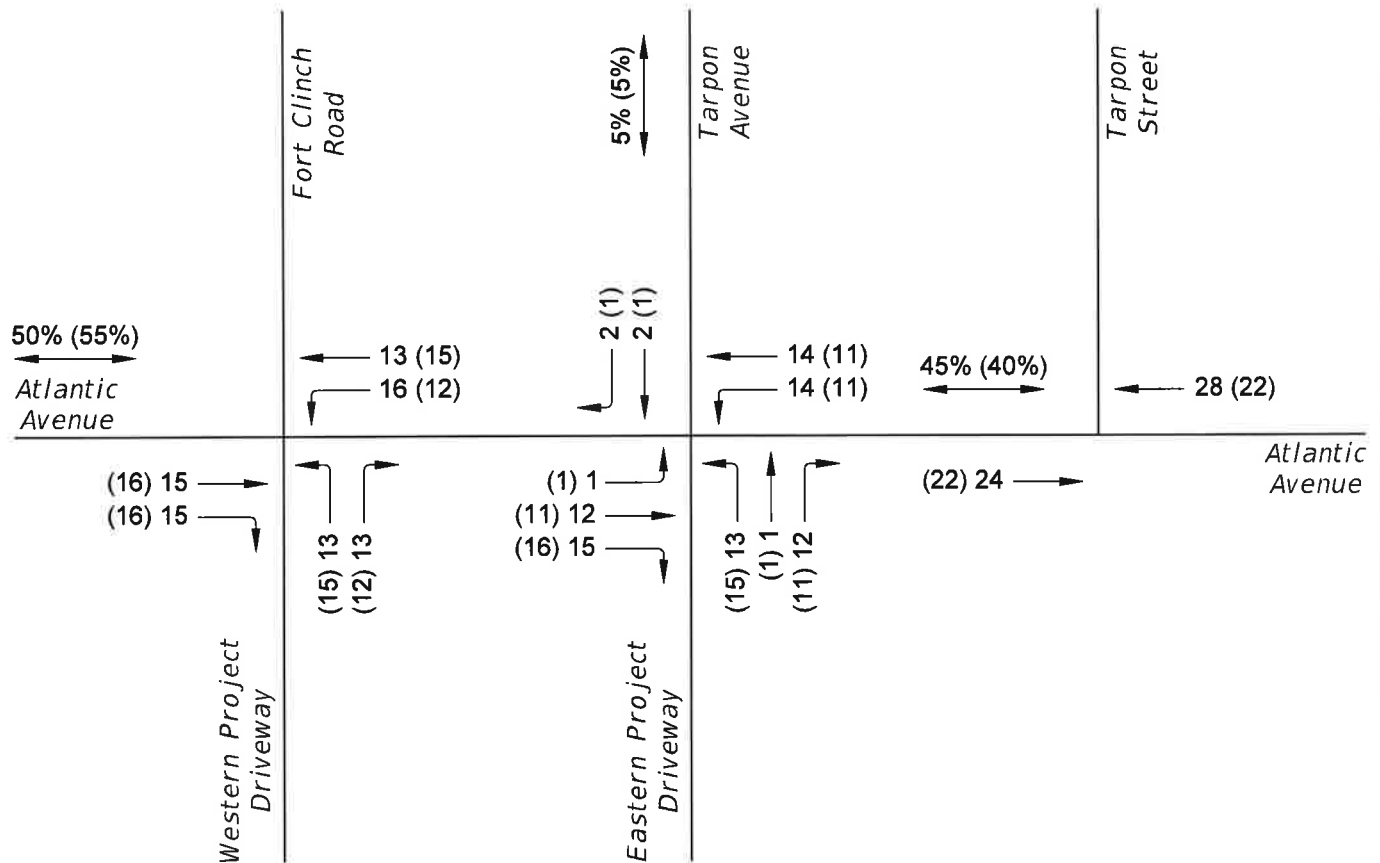


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Atlantic Avenue Hotel
 FDOT Traffic Study
 City of Fernandina Beach

FIGURE

04



PROJECT TRAFFIC DISTRIBUTION & ASSIGNMENT

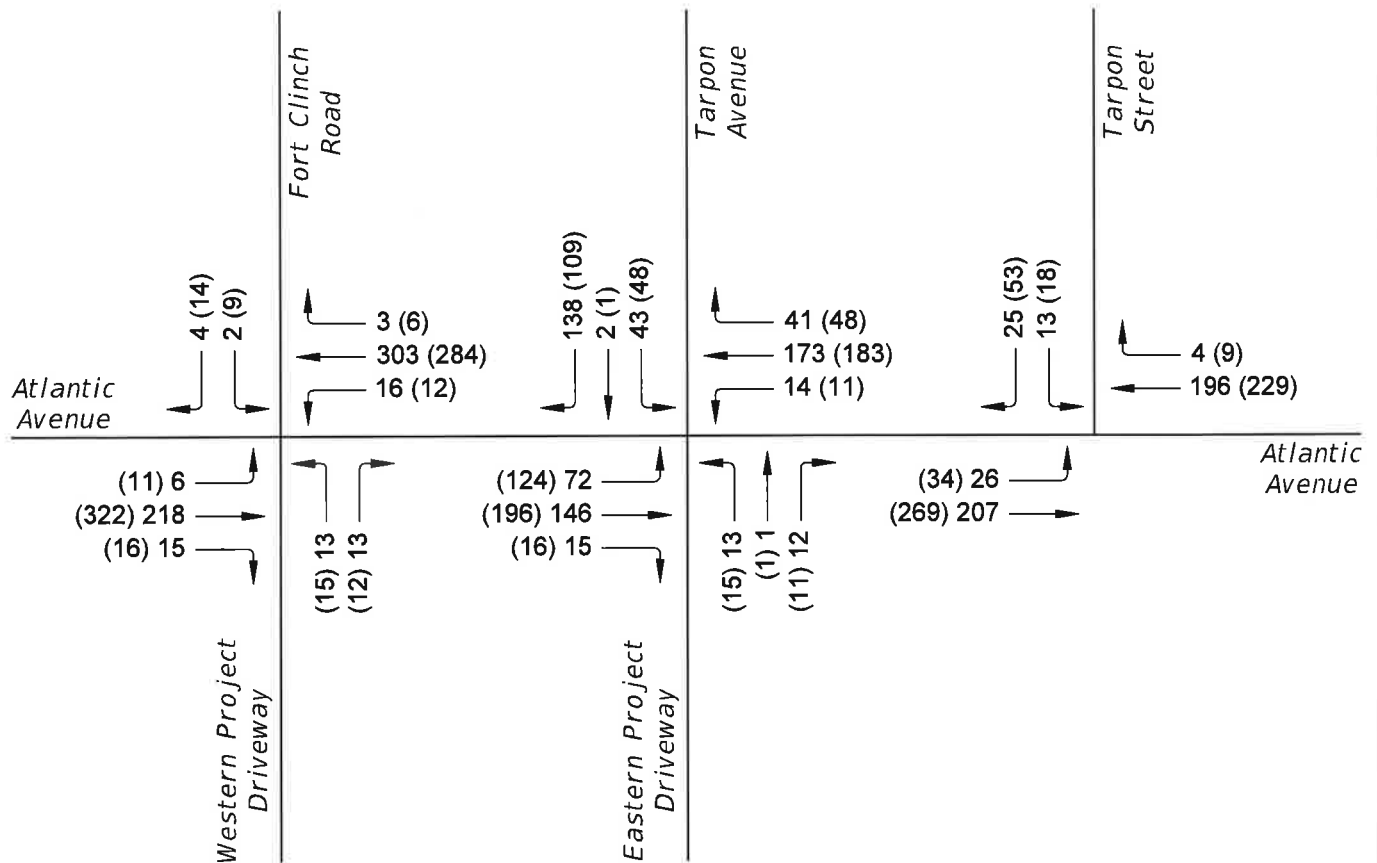


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FIGURE

05



LEGEND:

173 - AM Peak Hour Traffic
 (183) - PM Peak Hour Traffic

YEAR 2021 BUILD-OUT CONDITIONS TRAFFIC VOLUMES



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 Email: Chindalur@ctrfficsolutions.com

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 FDOT Traffic Study
 City of Fernandina Beach

FIGURE

06



	8833 Promenade Park Blvd Jacksonville, FL 32216 Phone: (904) 422-0924 Email: Chandana@ctsinc.com	Atlantic Avenue Hotel FDOT Traffic Study City Of Fernandina Beach
		RECOMMENDED IMPROVEMENTS
		PG 01 06 07

Table 01
Trip Generation
Atlantic Avenue Hotel - City of Fernandian Beach, FL

ITE Land Use Code	Description	Proposed Quantity	Units	Time Period	Equation	Proposed New Trips	Percent		Total Trips	
							Entering	Exiting	Entering	Exiting
311	All Suites Hotel	236	Rooms	Daily	T = 5.20(X) - 119.26	1,108	50%	50%	554	554
				AM Peak	T = 0.51(X) - 29.37	91	53%	47%	48	43
				PM Peak	T = 0.45(X) - 14.07	92	48%	52%	44	48
932	High-Turnover Sit-Down Restaurant	2,101	SF	Daily	T = 112.81 (X/1,000)	237	50%	50%	119	118
				AM Peak	T = 9.94 (X/1,000)	21	55%	45%	12	9
				PM Peak	T = 9.77 (X/1,000)	21	62%	38%	13	8
				Daily						
				AM Peak		1,345			673	672
				PM Peak		112			60	52
						113			57	56

Source: Trip Generation Manual, 10th Edition, Institute of Transportation Engineers (ITE)

Table 02
Year 2021 Build-Out Conditions Intersection Capacity Analysis - Delay and LOS
Atlantic Avenue Hotel - City of Fernandian Beach, FL

Intersection	Approach	Signal Control	AM Peak		PM Peak	
			Delay	LOS	Delay	LOS
Atlantic Avenue at Western Project Driveway	NB	Stop	12.30	B	14.10	B
	WBL	Yield	7.80	A	8.10	A
Atlantic Avenue at Tarpon Avenue/Eastern Project Driveway	NB	Stop	13.80	B	16.70	C
	EBL	Yield	7.90	A	8.10	A
	WBL	Yield	7.60	A	7.70	A
	SB	Stop	12.80	B	15.00	C
Atlantic Avenue at Tarpon Street	EBL	Yield	7.70	A	7.70	A
	SB	Stop	10.60	B	10.60	B

Source: Attachment F HCM Worksheets

Attachment A

Site Plan

Source: Gillette and Associates,
Inc.

Attachment B

Traffic Counts and FDOT Season Factors

2017 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL
 CATEGORY: 7400 NASSAU COUNTYWIDE

MOCF: 0.94

WEEK	DATES	SF	PSCF
1	01/01/2017 - 01/07/2017	1.07	1.14
2	01/08/2017 - 01/14/2017	1.12	1.19
3	01/15/2017 - 01/21/2017	1.17	1.24
4	01/22/2017 - 01/28/2017	1.13	1.20
5	01/29/2017 - 02/04/2017	1.10	1.17
6	02/05/2017 - 02/11/2017	1.07	1.14
7	02/12/2017 - 02/18/2017	1.04	1.11
8	02/19/2017 - 02/25/2017	1.02	1.09
9	02/26/2017 - 03/04/2017	1.00	1.06
10	03/05/2017 - 03/11/2017	0.97	1.03
*11	03/12/2017 - 03/18/2017	0.95	1.01
*12	03/19/2017 - 03/25/2017	0.95	1.01
*13	03/26/2017 - 04/01/2017	0.94	1.00
*14	04/02/2017 - 04/08/2017	0.94	1.00
*15	04/09/2017 - 04/15/2017	0.93	0.99
*16	04/16/2017 - 04/22/2017	0.93	0.99
*17	04/23/2017 - 04/29/2017	0.93	0.99
*18	04/30/2017 - 05/06/2017	0.93	0.99
*19	05/07/2017 - 05/13/2017	0.93	0.99
*20	05/14/2017 - 05/20/2017	0.93	0.99
*21	05/21/2017 - 05/27/2017	0.94	1.00
*22	05/28/2017 - 06/03/2017	0.95	1.01
*23	06/04/2017 - 06/10/2017	0.96	1.02
24	06/11/2017 - 06/17/2017	0.97	1.03
25	06/18/2017 - 06/24/2017	0.96	1.02
26	06/25/2017 - 07/01/2017	0.95	1.01
27	07/02/2017 - 07/08/2017	0.94	1.00
28	07/09/2017 - 07/15/2017	0.94	1.00
29	07/16/2017 - 07/22/2017	0.95	1.01
30	07/23/2017 - 07/29/2017	0.97	1.03
31	07/30/2017 - 08/05/2017	0.98	1.04
32	08/06/2017 - 08/12/2017	1.00	1.06
33	08/13/2017 - 08/19/2017	1.01	1.07
34	08/20/2017 - 08/26/2017	1.04	1.11
35	08/27/2017 - 09/02/2017	1.07	1.14
36	09/03/2017 - 09/09/2017	1.10	1.17
37	09/10/2017 - 09/16/2017	1.13	1.20
38	09/17/2017 - 09/23/2017	1.10	1.17
39	09/24/2017 - 09/30/2017	1.08	1.15
40	10/01/2017 - 10/07/2017	1.05	1.12
41	10/08/2017 - 10/14/2017	1.03	1.10
42	10/15/2017 - 10/21/2017	1.01	1.07
43	10/22/2017 - 10/28/2017	1.01	1.07
44	10/29/2017 - 11/04/2017	1.01	1.07
45	11/05/2017 - 11/11/2017	1.02	1.09
46	11/12/2017 - 11/18/2017	1.02	1.09
47	11/19/2017 - 11/25/2017	1.03	1.10
48	11/26/2017 - 12/02/2017	1.05	1.12
49	12/03/2017 - 12/09/2017	1.06	1.13
50	12/10/2017 - 12/16/2017	1.07	1.14
51	12/17/2017 - 12/23/2017	1.10	1.17
52	12/24/2017 - 12/30/2017	1.13	1.20
53	12/31/2017 - 12/31/2017	1.17	1.24

* PEAK SEASON

02-MAR-2018 15:35:05

830UPD

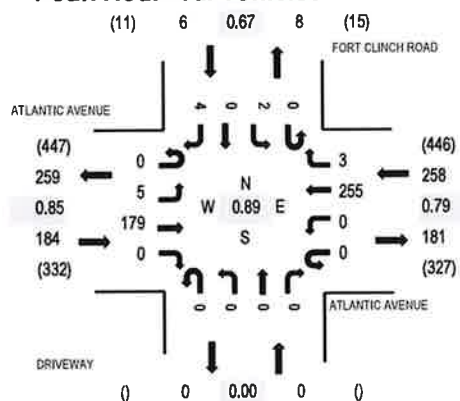
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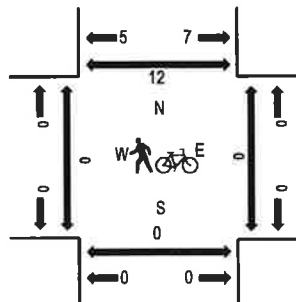
Location: 1 DRIVEWAY & ATLANTIC AVENUE AM
Date and Start Time: Wednesday, September 19, 2018
Peak Hour: 08:00 AM - 09:00 AM
Peak 15-Minutes: 08:30 AM - 08:45 AM

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.

Peak Hour - Pedestrians/Bicycles in Crosswalk



Traffic Counts

Interval Start Time	ATLANTIC AVENUE Eastbound				ATLANTIC AVENUE Westbound				DRIVEWAY Northbound				FORT CLINCH ROAD Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
7:00 AM	0	1	25	0	0	0	0	35	0	0	0	0	0	1	0	0	62	341	0	0	0	7
7:15 AM	0	2	28	0	0	0	0	49	1	0	0	0	0	1	0	0	81	373	0	0	0	3
7:30 AM	0	0	47	0	0	0	0	55	0	0	0	0	0	0	0	0	102	395	0	0	0	3
7:45 AM	0	1	44	0	0	0	0	46	2	0	0	0	0	0	0	0	96	419	0	0	0	7
8:00 AM	0	1	41	0	0	0	0	51	0	0	0	0	0	0	0	0	94	448	0	0	0	5
8:15 AM	0	0	45	0	0	0	0	54	1	0	0	0	0	2	0	1	103		0	0	0	2
8:30 AM	0	2	41	0	0	0	0	81	1	0	0	0	0	0	0	0	126		0	0	0	0
8:45 AM	0	2	52	0	0	0	0	69	1	0	0	0	0	0	0	0	125		0	0	0	4

Peak Rolling Hour Flow Rates

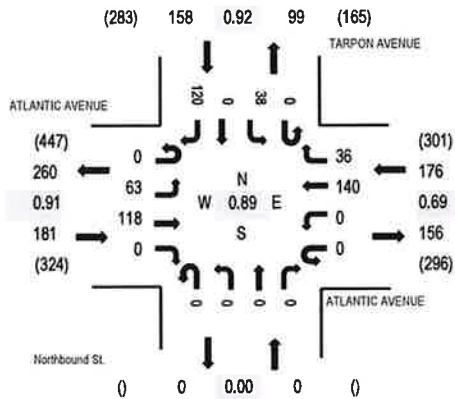
Vehicle Type	Eastbound				Westbound				Northbound				Southbound				Total
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
Articulated Trucks	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	3
Lights	0	5	173	0	0	0	248	3	0	0	0	0	0	2	0	4	435
Mediums	0	0	3	0	0	0	7	0	0	0	0	0	0	0	0	0	10
Total	0	5	179	0	0	0	255	3	0	0	0	0	0	2	0	4	448



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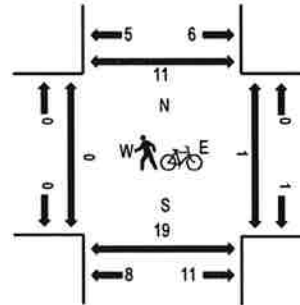
Location: 2 Northbound St. & ATLANTIC AVENUE AM
Date and Start Time: Wednesday, September 19, 2018
Peak Hour: 08:00 AM - 09:00 AM
Peak 15-Minutes: 08:30 AM - 08:45 AM

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.

Peak Hour - Pedestrians/Bicycles in Crosswalk



Traffic Counts

Interval Start Time	ATLANTIC AVENUE Eastbound				ATLANTIC AVENUE Westbound				Northbound St. Northbound				TARPON AVENUE Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
7:00 AM	0	4	22	0	0	0	21	4	0	0	0	0	0	4	0	13	68	393	0	0	4	4
7:15 AM	0	6	23	0	0	0	28	3	0	0	0	0	0	8	0	24	92	437	0	0	2	2
7:30 AM	0	12	34	0	0	0	27	10	0	0	0	0	0	9	0	27	119	466	0	0	2	3
7:45 AM	0	18	24	0	0	0	23	9	0	0	0	0	0	16	0	24	114	492	0	1	4	7
8:00 AM	0	13	31	0	0	0	22	9	0	0	0	0	0	7	0	30	112	515	0	0	4	5
8:15 AM	0	12	35	0	0	0	30	7	0	0	0	0	0	11	0	26	121		0	0	2	2
8:30 AM	0	16	24	0	0	0	49	15	0	0	0	0	0	10	0	31	145		0	1	4	0
8:45 AM	0	22	28	0	0	0	39	5	0	0	0	0	0	10	0	33	137		0	0	6	2

Peak Rolling Hour Flow Rates

Vehicle Type	Eastbound				Westbound				Northbound				Southbound				Total
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
Articulated Trucks	0	1	2	0	0	0	0	1	0	0	0	0	0	1	0	0	5
Lights	0	61	115	0	0	0	136	26	0	0	0	0	0	28	0	118	484
Mediums	0	1	1	0	0	0	4	9	0	0	0	0	0	9	0	2	26
Total	0	63	118	0	0	0	140	36	0	0	0	0	0	38	0	120	515



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Peak 15-Minutes: 08:45 AM - 09:00 AM

Vehicle Type	Eastbound				Westbound				Northbound				Southbound				Total
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
Articulated Trucks	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
Lights	0	23	160	0	0	0	142	4	0	0	0	0	0	12	0	22	363
Mediums	0	0	2	0	0	0	4	0	0	0	0	0	0	0	0	0	6
Total	0	23	162	0	0	0	148	4	0	0	0	0	0	12	0	22	371



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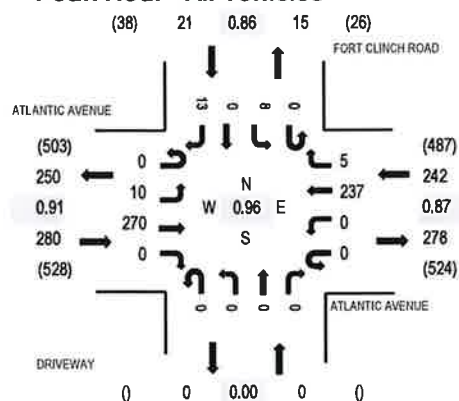
Location: 1 DRIVEWAY & ATLANTIC AVENUE PM

Date and Start Time: Wednesday, September 19, 2018

Peak Hour: 04:30 PM - 05:30 PM

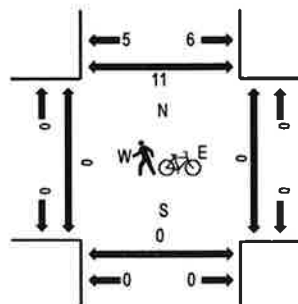
Peak 15-Minutes: 04:45 PM - 05:00 PM

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.

Peak Hour - Pedestrians/Bicycles in Crosswalk



Traffic Counts

Interval Start Time	ATLANTIC AVENUE Eastbound				ATLANTIC AVENUE Westbound				DRIVEWAY Northbound				FORT CLINCH ROAD Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
4:00 PM	0	1	69	0	0	0	62	1	0	0	0	0	0	2	0	1	136	534	0	0	0	1
4:15 PM	0	0	47	0	0	0	68	0	0	0	0	0	2	0	3	120	538	0	0	0	1	
4:30 PM	0	3	74	0	0	0	52	1	0	0	0	0	3	0	4	137	543	0	0	0	3	
4:45 PM	0	2	73	0	0	0	56	4	0	0	0	0	4	0	2	141	541	0	0	0	2	
5:00 PM	0	3	73	0	0	0	58	0	0	0	0	0	1	0	5	140	519	0	0	0	2	
5:15 PM	0	2	50	0	0	0	71	0	0	0	0	0	0	0	2	125		0	0	0	1	
5:30 PM	0	4	70	0	0	0	58	0	0	0	0	0	1	0	2	135		0	0	0	0	
5:45 PM	0	4	53	0	0	0	55	1	0	0	0	0	2	0	4	119		0	0	0	0	

Peak Rolling Hour Flow Rates

Vehicle Type	Eastbound				Westbound				Northbound				Southbound				Total
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
Articulated Trucks	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	2
Lights	0	10	266	0	0	0	0	233	5	0	0	0	0	8	0	12	534
Mediums	0	0	3	0	0	0	0	3	0	0	0	0	0	0	0	1	7
Total	0	10	270	0	0	0	0	237	5	0	0	0	0	8	0	13	543



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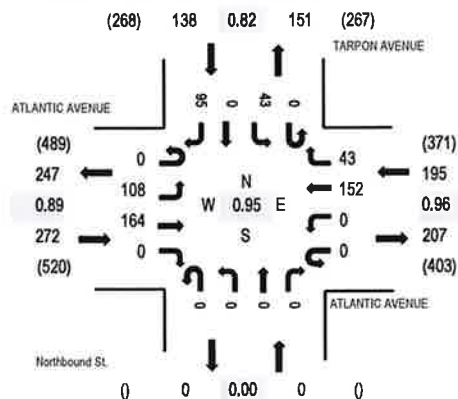
Location: 2 Northbound St. & ATLANTIC AVENUE PM

Date and Start Time: Wednesday, September 19, 2018

Peak Hour: 04:45 PM - 05:45 PM

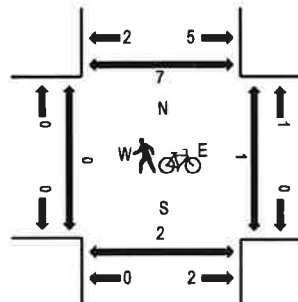
Peak 15-Minutes: 04:45 PM - 05:00 PM

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.

Peak Hour - Pedestrians/Bicycles in Crosswalk



Traffic Counts

Interval Start Time	ATLANTIC AVENUE				ATLANTIC AVENUE				Northbound St.				TARPON AVENUE				Total	Rolling Hour	Pedestrian Crossings				
	Eastbound				Westbound				Northbound				Southbound						Pedestrian Crossings				
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North	
4:00 PM	0	24	44	0	0	0	36	12	0	0	0	0	0	11	0	27	154	586	0	0	0	1	
4:15 PM	0	21	29	0	0	0	42	11	0	0	0	0	0	10	0	25	138	583	0	0	0	0	
4:30 PM	1	22	52	0	0	0	26	4	0	0	0	0	0	4	0	26	135	581	0	0	1	3	
4:45 PM	0	35	43	0	0	0	35	13	0	0	0	0	0	9	0	24	159	605	0	0	1	2	
5:00 PM	0	23	51	0	0	0	38	11	0	0	0	0	0	8	0	20	151	573	0	0	0	2	
5:15 PM	0	22	28	0	0	0	44	7	0	0	0	0	0	9	0	26	136		0	0	0	0	
5:30 PM	0	28	42	0	0	0	35	12	0	0	0	0	0	17	0	25	159		0	0	0	0	
5:45 PM	0	17	38	0	0	0	40	5	0	0	0	0	0	8	0	19	127		0	0	0	0	

Peak Rolling Hour Flow Rates

Vehicle Type	Eastbound				Westbound				Northbound				Southbound				Total
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
Articulated Trucks	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
Lights	0	108	163	0	0	0	150	43	0	0	0	0	0	42	0	94	600
Mediums	0	0	1	0	0	0	1	0	0	0	0	0	0	1	0	1	4
Total	0	108	164	0	0	0	152	43	0	0	0	0	0	43	0	95	605



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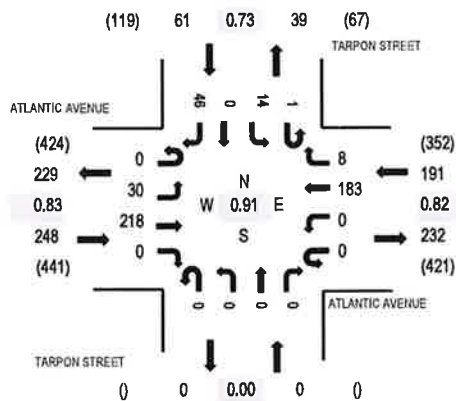
Location: 1 TARPON STREET & ATLANTIC AVENUE PM

Date and Start Time: Tuesday, November 06, 2018

Peak Hour: 04:00 PM - 05:00 PM

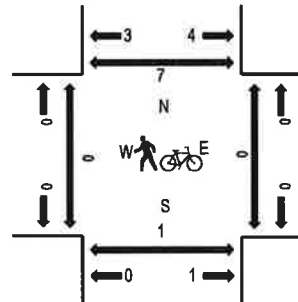
Peak 15-Minutes: 04:30 PM - 04:45 PM

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.

Peak Hour - Pedestrians/Bicycles in Crosswalk



Traffic Counts

Interval Start Time	ATLANTIC AVENUE Eastbound				ATLANTIC AVENUE Westbound				TARPON STREET Northbound				TARPON STREET Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
4:00 PM	0	5	56	0	0	0	56	2	0	0	0	0	0	3	0	9	131	500	0	0	0	1
4:15 PM	0	8	44	0	0	0	46	3	0	0	0	0	0	2	0	12	115	495	0	0	0	1
4:30 PM	0	8	69	0	0	0	45	3	0	0	0	0	0	1	0	11	137	486	0	0	1	3
4:45 PM	0	9	49	0	0	0	36	0	0	0	0	0	1	8	0	14	117	445	0	0	0	2
5:00 PM	0	7	61	0	0	0	40	1	0	0	0	0	0	6	0	11	126	412	0	0	0	3
5:15 PM	0	8	41	0	1	0	43	1	0	0	0	0	0	2	0	10	106		0	0	0	4
5:30 PM	0	8	38	0	0	0	34	1	0	0	0	0	0	6	0	9	96		0	0	0	0
5:45 PM	0	1	29	0	0	0	39	1	0	0	0	0	0	5	0	9	84		0	0	0	5

Peak Rolling Hour Flow Rates

Vehicle Type	Eastbound				Westbound				Northbound				Southbound				Total
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Lights	0	30	215	0	0	0	179	7	0	0	0	0	1	14	0	44	490
Mediums	0	0	3	0	0	0	4	1	0	0	0	0	0	0	0	2	10
Total	0	30	218	0	0	0	183	8	0	0	0	0	1	14	0	46	500

Attachment C

Historic AADT and Trends Analysis Plot

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2017 HISTORICAL AADT REPORT

COUNTY: 74 - NASSAU

SITE: 0128 - SR A1A .2 MI. W. OF JEAN LAFITTE ST.

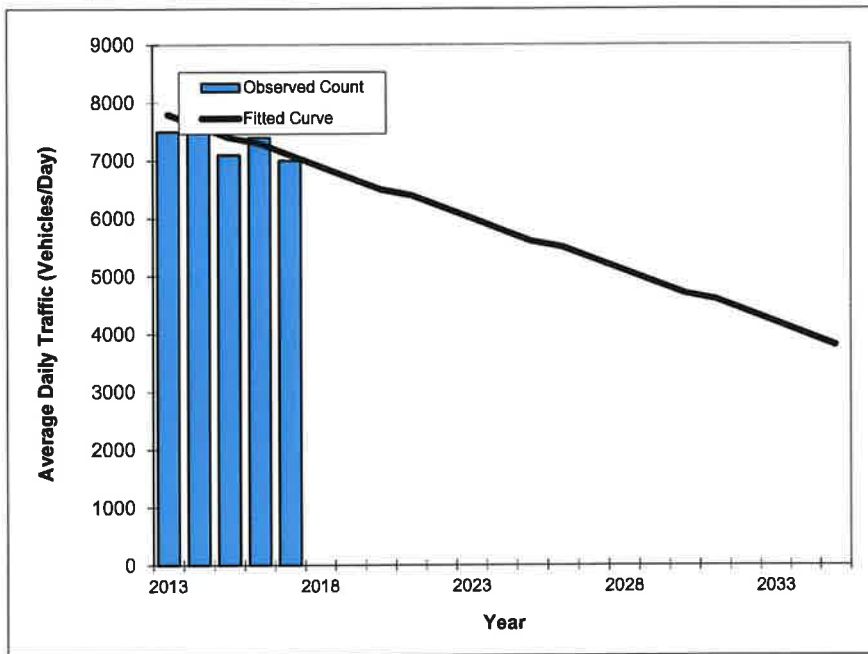
YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2017	7000 C	E 3500	W 3500	9.00	55.10	6.50
2016	7400 C	E 3700	W 3700	9.00	56.00	6.70
2015	7100 C	E 3500	W 3600	9.00	55.30	6.40
2014	8200 C	E 4100	W 4100	9.00	55.10	6.30
2013	7500 C	E 3700	W 3800	9.00	56.90	7.20
2012	7300 C	E 3600	W 3700	9.00	54.70	6.30
2011	8000 C	E 4000	W 4000	9.00	55.80	6.40
2010	7700 C	E 3800	W 3900	12.04	58.48	6.80
2009	7300 C	E 3700	W 3600	11.44	57.12	7.10
2008	7800 C	E 3900	W 3900	10.08	59.26	7.10
2007	7700 C	E 3700	W 4000	11.16	57.15	6.00
2006	7200 C	E 3600	W 3600	11.41	58.30	7.20
2005	8500 C	E 4200	W 4300	11.70	59.30	4.50
2004	7800 C	E 3800	W 4000	11.50	58.30	9.10
2003	7600 C	E 3800	W 3800	11.00	57.60	8.00
2002	8300 C	E 4100	W 4200	11.90	60.00	8.30

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

Traffic Trends - V2.0
SR 200 -- W. OF JEAN LAFITTE ST.

PIN#	973215-1
Location	1

County:	Nassau (74)
Station #:	0
Highway:	SR 200



Year	Traffic (ADT/AADT)	
	Count*	Trend**
2013	7500	7800
2014	8200	7600
2015	7100	7400
2016	7400	7300
2017	7000	7100
2018 Opening Year Trend		
2018	N/A	6900
2019 Mid-Year Trend		
2019	N/A	6700
2020 Design Year Trend		
2020	N/A	6500
TRANPLAN Forecasts/Trends		

** Annual Trend Increase:	-180
Trend R-squared:	36.32%
Trend Annual Historic Growth Rate:	-2.24%
Trend Growth Rate (2017 to Design Year):	-2.82%
Printed:	23-Oct-18
Straight Line Growth Option	

*Axle-Adjusted

Attachment D
TMC Projections

Year 2018 Traffic Volumes without Season Factors
Fort Clinch Road

	5	4	2	3
50.42%	179			255

Western Project Driveway

355

Atlantic Avenue

Tarpon Avenue

	63	120	38	36
	118			140

Eastern Project Driveway

49.58% Atlantic Avenue

Tarpon Street

	23	22	12	4
	162			148

Year 2018 Traffic Volumes with Season Factors
SF = 1.10

Fort Clinch Road

	6	4	2	3
50.38%	197			281

Western Project Driveway

391

Atlantic Avenue

Tarpon Avenue

	69	132	42	40
	130			154

Eastern Project Driveway

49.62% Atlantic Avenue

Tarpon Street

	25	24	13	4
	178			163

Year 2020 Background Conditions Traffic Volumes
GF = 1.030

Fort Clinch Road

	6	4	2	3
	203			290

Western Project Driveway

Atlantic Avenue

Tarpon Avenue

	71	136	43	41
	134			159

Eastern Project Driveway

Atlantic Avenue

Tarpon Street

	26	25	13	4
	183			168

Project Traffic Distribution and Assignment

Entering 60
Exiting 52

Fort Clinch Road

	15			13
50%	15			16
		13	13	

Western Project Driveway

Atlantic Avenue

Tarpon Avenue

	1	2	2	
	12			14
	15			14
		13	1	12

Eastern Project Driveway

45% Atlantic Avenue

Tarpon Street

	24			
				28

Year 2020 Build-Out Conditions Traffic Volumes
Fort Clinch Road

	6	4	2	3
	218			303
6.28%	15			16
		13	13	

Western Project Driveway

Atlantic Avenue

Tarpon Avenue

	72	138	2	43
	146			41
	15			173
		13	1	12

Eastern Project Driveway

6.44%

Atlantic Avenue

Tarpon Street

	26	25	0	13
	207			4
	0			196
		0	0	0

Year 2018 Traffic Volumes Without Season Factors
Fort Clinch Road

		13		8		
58.06%	10				5	
	270				237	

Western Project Driveway

465

Atlantic Avenue

Tarpon Avenue

		95		43		
	108				43	
	164				152	

Eastern Project Driveway

41.94% Atlantic Avenue

Tarpon Street

		46		15		
	30				8	
	218				183	

Year 2018 Traffic Volumes with Season Factors
SF = 1.10

Fort Clinch Road

		14		9		
58.12%	11				6	
	297				261	

Western Project Driveway

511

Atlantic Avenue

Tarpon Avenue

		105		47		
	119				47	
	180				167	

Eastern Project Driveway

41.88% Atlantic Avenue

Tarpon Street

		51		17		
	33				9	
	240				201	

Year 2020 Background Conditions Traffic Volumes
GF = 1.030

Fort Clinch Road

		14		9		
	11				6	
	306				269	

Western Project Driveway

Atlantic Avenue

Tarpon Avenue

		108		48		
	123				48	
	185				172	

Eastern Project Driveway

Atlantic Avenue

Tarpon Street

		53		18		
	34				9	
	247				207	

Project Traffic Distribution and Assignment

Entering 57
Exiting 56

Fort Clinch Road

55%	16				35	
	16				12	
		15		12		

Western Project Driveway

Atlantic Avenue

Tarpon Avenue

		1		1		
	1					
	11				11	
	16				11	
		15		11		

Eastern Project Driveway

40% Atlantic Avenue

Tarpon Street

	22				22	

Year 2020 Build-Out Conditions Traffic Volumes
Fort Clinch Road

		14		9		
	11				6	
	322				284	
	16				12	
4.58%		15		12		

Western Project Driveway

Atlantic Avenue

Tarpon Avenue

		109		48		
	124				48	
	196				183	
	16				11	
4.76%		15		11		

Eastern Project Driveway

Atlantic Avenue

Tarpon Street

		53		0		18
	34				9	
	269				229	
	0				0	
		0		0		0

Attachment E

FDOT Right Turn Lane Criteria

7.2

WHEN SHOULD WE BUILD RIGHT TURN LANES?

Exhibit 44
Recommended Guidelines
for Exclusive Right Turn
Lanes to Unsignalized*
Driveway

Roadway Posted Speed Limit	Number of Right Turns Per Hour
45 mph or less	80-125 (see note 1)
Over 45 mph	35-55 (see note 2)

*May not be appropriate for signalized locations where signal phasing plays an important role in determining the need for right turn lanes.

1. The lower threshold of 80 right turn vehicles per hour would be most used for higher volume (greater than 600 vehicles per hour, per lane in one direction on the major roadway) or two-lane roads where lateral movement is restricted. The 125 right turn vehicles per hour upper threshold would be most appropriate on lower volume roadways, multilane highways, or driveways with a large entry radius (50 feet or greater).
2. The lower threshold of 35 right turn vehicles per hour would be most appropriately used on higher volume two-lane roadways where lateral movement is restricted. The 55 right turn vehicles per hour upper threshold would be most appropriate on lower volume roadways, multilane highways, or driveways with large entry radius (50 feet or greater).

Note: A posted speed limit of 45 mph may be used with these thresholds if the operating speeds are known to be over 45 mph during the time of peak right turn demand.

Note on Traffic projections: Projecting turning volumes is, at best, a knowledgeable estimate. Keep this in mind especially if the projections of right turns are close to meeting the guidelines. In that case, consider requiring the turn lane.

Attachment F

Year 2021 Build-Out
Conditions HCM
Worksheets

HCM 2010 TWSC
3: Atlantic Avenue & Fort Clinch Road

Year 2021 Build-Out Conditions
Timing Plan: AM Peak

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↕	↕			↕			↕	
Traffic Vol, veh/h	6	218	15	16	303	3	13	0	13	2	0	4
Future Vol, veh/h	6	218	15	16	303	3	13	0	13	2	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	7	237	16	17	329	3	14	0	14	2	0	4

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	332	0	0	253	0	0	626	625	245	631	632	331
Stage 1	-	-	-	-	-	-	259	259	-	365	365	-
Stage 2	-	-	-	-	-	-	367	366	-	266	267	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1227	-	-	1312	-	-	397	401	794	394	398	711
Stage 1	-	-	-	-	-	-	746	694	-	654	623	-
Stage 2	-	-	-	-	-	-	653	623	-	739	688	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1227	-	-	1312	-	-	389	393	794	381	390	711
Mov Cap-2 Maneuver	-	-	-	-	-	-	389	393	-	381	390	-
Stage 1	-	-	-	-	-	-	741	689	-	649	615	-
Stage 2	-	-	-	-	-	-	641	615	-	721	683	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.2	0.4	12.3	11.6
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	522	1227	-	-	1312	-	-	552
HCM Lane V/C Ratio	0.054	0.005	-	-	0.013	-	-	0.012
HCM Control Delay (s)	12.3	8	0	-	7.8	-	-	11.6
HCM Lane LOS	B	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	0

HCM 2010 TWSC
5: Atlantic Avenue & Tarpon Avenue

Year 2021 Build-Out Conditions
Timing Plan: AM Peak

Intersection												
Int Delay, s/veh	5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	72	146	15	14	173	41	13	1	12	43	2	138
Future Vol, veh/h	72	146	15	14	173	41	13	1	12	43	2	138
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	140	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	78	159	16	15	188	45	14	1	13	47	2	150

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	233	0	0	175	0	0	640	586	167	571	572	211
Stage 1	-	-	-	-	-	-	323	323	-	241	241	-
Stage 2	-	-	-	-	-	-	317	263	-	330	331	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1335	-	-	1401	-	-	388	422	877	432	430	829
Stage 1	-	-	-	-	-	-	689	650	-	762	706	-
Stage 2	-	-	-	-	-	-	694	691	-	683	645	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1335	-	-	1401	-	-	300	393	877	403	401	829
Mov Cap-2 Maneuver	-	-	-	-	-	-	300	393	-	403	401	-
Stage 1	-	-	-	-	-	-	649	612	-	718	698	-
Stage 2	-	-	-	-	-	-	561	683	-	632	608	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	2.4	0.5	13.8	12.8
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	437	1335	-	-	1401	-	-	658
HCM Lane V/C Ratio	0.065	0.059	-	-	0.011	-	-	0.302
HCM Control Delay (s)	13.8	7.9	-	-	7.6	-	-	12.8
HCM Lane LOS	B	A	-	-	A	-	-	B
HCM 95th %tile Q(veh)	0.2	0.2	-	-	0	-	-	1.3

HCM 2010 TWSC
9: Atlantic Avenue & Tarpon Street

Year 2021 Build-Out Conditions
Timing Plan: AM Peak

Intersection

Int Delay, s/veh 1.3

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	26	207	196	4	13	25
Future Vol, veh/h	26	207	196	4	13	25
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	80	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	28	225	213	4	14	27

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	217	0	0 496 215
Stage 1	-	-	- 215 -
Stage 2	-	-	- 281 -
Critical Hdwy	4.12	-	- 6.42 6.22
Critical Hdwy Stg 1	-	-	- 5.42 -
Critical Hdwy Stg 2	-	-	- 5.42 -
Follow-up Hdwy	2.218	-	- 3.518 3.318
Pot Cap-1 Maneuver	1353	-	- 533 825
Stage 1	-	-	- 821 -
Stage 2	-	-	- 767 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1353	-	- 522 825
Mov Cap-2 Maneuver	-	-	- 522 -
Stage 1	-	-	- 804 -
Stage 2	-	-	- 767 -

Approach	EB	WB	SB
HCM Control Delay, s	0.9	0	10.6
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1353	-	-	-	688
HCM Lane V/C Ratio	0.021	-	-	-	0.06
HCM Control Delay (s)	7.7	-	-	-	10.6
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.2

HCM 2010 TWSC
3: Atlantic Avenue & Fort Clinch Road

Year 2021 Build-Out Conditions
Timing Plan: PM Peak

Intersection

Int Delay, s/veh 1.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↕	↕			↕			↕	
Traffic Vol, veh/h	11	322	16	12	284	6	15	0	12	9	0	14
Future Vol, veh/h	11	322	16	12	284	6	15	0	12	9	0	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	12	350	17	13	309	7	16	0	13	10	0	15

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	316	0	0	367	0	0	729	725	359	728	730	313
Stage 1	-	-	-	-	-	-	383	383	-	339	339	-
Stage 2	-	-	-	-	-	-	346	342	-	389	391	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1244	-	-	1192	-	-	338	352	685	339	349	727
Stage 1	-	-	-	-	-	-	640	612	-	676	640	-
Stage 2	-	-	-	-	-	-	670	638	-	635	607	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1244	-	-	1192	-	-	325	344	685	327	341	727
Mov Cap-2 Maneuver	-	-	-	-	-	-	325	344	-	327	341	-
Stage 1	-	-	-	-	-	-	632	605	-	668	633	-
Stage 2	-	-	-	-	-	-	649	631	-	615	600	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.2	0.3	14.1	12.7
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	424	1244	-	-	1192	-	-	492
HCM Lane V/C Ratio	0.069	0.01	-	-	0.011	-	-	0.051
HCM Control Delay (s)	14.1	7.9	0	-	8.1	-	-	12.7
HCM Lane LOS	B	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	0.2

HCM 2010 TWSC
5: Atlantic Avenue & Tarpon Avenue

Year 2021 Build-Out Conditions
Timing Plan: PM Peak

Intersection

Int Delay, s/veh 5.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	124	195	16	11	183	48	15	1	11	48	1	109
Future Vol, veh/h	124	195	16	11	183	48	15	1	11	48	1	109
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	140	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	135	212	17	12	199	52	16	1	12	52	1	118

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	251	0	0	229	0	0	800	766	221	746	748	225
Stage 1	-	-	-	-	-	-	491	491	-	249	249	-
Stage 2	-	-	-	-	-	-	309	275	-	497	499	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1314	-	-	1339	-	-	303	333	819	330	341	814
Stage 1	-	-	-	-	-	-	559	548	-	755	701	-
Stage 2	-	-	-	-	-	-	701	683	-	555	544	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1314	-	-	1339	-	-	236	296	819	297	303	814
Mov Cap-2 Maneuver	-	-	-	-	-	-	236	296	-	297	303	-
Stage 1	-	-	-	-	-	-	501	492	-	677	695	-
Stage 2	-	-	-	-	-	-	593	677	-	490	488	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	3	0.4	16.7	15
HCM LOS			C	C

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	336	1314	-	-	1339	-	-	529
HCM Lane V/C Ratio	0.087	0.103	-	-	0.009	-	-	0.325
HCM Control Delay (s)	16.7	8.1	-	-	7.7	-	-	15
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.3	0.3	-	-	0	-	-	1.4

HCM 2010 TWSC
9: Atlantic Avenue & Tarpon Street

Year 2021 Build-Out Conditions
Timing Plan: PM Peak

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	34	269	229	9	18	53
Future Vol, veh/h	34	269	229	9	18	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	80	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	37	292	249	10	20	58

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	259	0	0 620 254
Stage 1	-	-	- 254 -
Stage 2	-	-	- 366 -
Critical Hdwy	4.12	-	- 6.42 6.22
Critical Hdwy Stg 1	-	-	- 5.42 -
Critical Hdwy Stg 2	-	-	- 5.42 -
Follow-up Hdwy	2.218	-	- 3.518 3.318
Pot Cap-1 Maneuver	1306	-	- 452 785
Stage 1	-	-	- 788 -
Stage 2	-	-	- 702 -
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	1306	-	- 439 785
Mov Cap-2 Maneuver	-	-	- 439 -
Stage 1	-	-	- 766 -
Stage 2	-	-	- 702 -

Approach	EB	WB	SB
HCM Control Delay, s	0.9	0	11.2
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1306	-	-	-	654
HCM Lane V/C Ratio	0.028	-	-	-	0.118
HCM Control Delay (s)	7.8	-	-	-	11.2
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.4



Gopher Tortoise 10 or Fewer Burrows
FLORIDA FISH AND WILDLIFE CONSERVATION COMMISSION
Division of Habitat and Species Conservation
Species Conservation Planning Section
620 South Meridian Street, Mail Station 2A
Tallahassee, Florida 32399-1600
(850) 921-1031

Permittee Name: Main Beach Sojourn, LLLP
Permittee Address: 15 Piedmont Center Ste 930
ATLANTA, GEORGIA
30305 UNITED STATES
Permit Number: GTT-18-00881
Effective Date: November 1, 2018
Expiration Date: November 1, 2019
Agent Name: Michael Glover
Agent Address: 791 West River Oak Drive
ORMOND BEACH, FLORIDA 32174
UNITED STATES

IS AUTHORIZED TO:

Capture by using bucket traps, live traps, hand shovel and backhoe excavation of tortoise burrows, transport and relocate all gopher tortoise (*Gopherus polyphemus*) hatchlings less than or equal to 60 mm carapace length (CL), and up to 7 gopher tortoises greater than 60 mm CL by non-harmful means, and to molest, damage or destroy gopher tortoise burrows while conducting these activities, subject to the following conditions and provisions, in association with development activities within the following site that contains 10 or fewer gopher tortoise burrows.

AUTHORIZED LOCATION(S): 4.73-acre Hotel site located at 2700 Atlantic Ave (T3N,R29E,S14; with Latitude 30° 40' 3.3" N and Longitude 81° 26' 1.51" W), in Nassau Co., relocated to the on-site recipient area.

Permittee Signature: _____ Date: _____

Not valid unless signed. By signature, confirms that all information provided to issue the permit is accurate and complete, and indicates acceptance and understanding of the provisions and conditions listed below. **Any false statements or misrepresentations when applying for this permit may result in felony charges and will result in revocation of this permit.**

Authorized By: Eric Seckinger Authorized for: Eric Sutton, Executive Director

Authorizing Signature: _____ Date: 11/01/2018
Species Conservation Planning Section

PERMIT CONDITIONS AND PROVISIONS:

- 1 Authorization to conduct the specified activities in association with the relocation of gopher tortoises in Florida is subject to Rules 68A-9.002 and 68A-27 Florida Administrative Code (F.A.C.), and the Florida Fish and Wildlife Conservation Commission's (hereafter, "FWC") Gopher Tortoise Permitting Guidelines (April 2008 - revised January 2017) [hereafter, "Permitting Guidelines"], and the following provisions/conditions.
- 2 Authorized activities are also predicated and conditioned on the information and assurances provided in the Permittee's 10/23/2018 application, the assurances of which are herein incorporated by reference.
- 3 Gopher tortoises may be live-captured by non-harmful methods authorized by this permit, and relocated and released on-site within the legal boundaries of the authorized location to preclude their harm due to imminent

construction or land clearing activities. If bucket-trapping is used, traps must be checked at least once per day and remain in place for at least 28 consecutive days or until a tortoise is captured, whichever occurs first. Release of tortoises must be accomplished in such a way as to preclude tortoises returning to their burrows. Temporary fencing shall be used to exclude relocated tortoises from construction sites. If temporary pens are used to confine tortoises, shade, water, and forage (i.e. food) must be available therein, and confinement must not exceed 10 days and comply with all other requirements of the Permitting Guidelines.

- 4 During colder months, tortoises shall only be captured and relocated when the low temperature at the recipient site is forecasted by the National Weather Service (www.weather.gov) to be above 50° F for three consecutive days [72 hours] after release (including the day of relocation). This three-day window of milder temperatures is required to allow the relocated tortoises to settle into the recipient site and reduce the chance of cold-related stress or mortality. If capturing tortoises using bucket traps, the Permittee shall cover all traps on days forecasted to be below 50° F. The 28 consecutive day trapping period must restart at day 1 when a trap is closed for any reason.
- 5 Either this original permit, or a complete copy, must be clearly posted at the affected site at all times while engaged in the permitted tortoises relocation activities, and should remain posted until construction activities are completed. This permit shall be effective only after issuance of written local government approval for land clearing or grading or construction activities. Originals or copies of all such legally required permits and approvals shall be available for inspection on the property during permitted activities.
- 6 This permit does not authorize access to any public or private properties. Permission to access the property must be secured from the appropriate landowners prior to undertaking any work on such properties.
- 7 The Permittee, by signing this permit, specifically agrees to allow authorized FWC personnel, upon presentation of credentials as may be required by law, access to the donor and recipient sites, at reasonable times, for the purpose of inspecting the capture/relocation activities authorized under this permit.
- 8 This permit can be suspended, revoked or not renewed for just cause pursuant to 68-1.010, F.A.C. and Chapter 120, Florida Statutes. It is non-transferable.
- 9 Any gopher tortoise mortality or injury that occurs while conducting activities authorized under this permit shall be reported to the Gopher Tortoise Program Coordinator (by phone at 850-921-1031 or by email to GTPermits@MyFWC.com) within 48 hours of the occurrence. An injured gopher tortoise shall be promptly taken to either a licensed wildlife rehabilitation facility or a licensed veterinarian for evaluation and treatment. Contact information for the facility or veterinarian shall be included with the information reported.
- 10 The Permittee is encouraged to submit an after action report detailing the capture/relocation to the Gopher Tortoise Program Coordinator using the FWC's [online permit system](#) within 30 days of release of the captured/relocated tortoises.
- 11 Nonnative wildlife that are members of the families Pythonidae, Boidae, Varanidae, Iguanidae and Teiidae captured during gopher tortoise relocation activities shall be euthanized in accordance with the requirements of the Permitting Guidelines. Authorized Agents are authorized to transport conditional species for purposes of euthanasia in accordance with the transport requirements in the Permitting Guidelines. All other non-native wildlife captured should be humanely euthanized or allowed to escape on-site. Sightings of Burmese python, Argentine tegu, or other nonnative wildlife species should be reported to the FWC within 48 hours by either calling the toll-free number 1-888-IVEGOT1 (1-888-483-4681) or online at www.IveGot1.org.
- 12 Gopher tortoise commensals listed in 68A-27 F.A.C. as either State-designated threatened species or species of special concern (this does not include the eastern indigo snake [*Drymarchon couperi*] and other Federally-designated Endangered and Threatened species) and encountered in the gopher tortoise capture operation should either be released on-site or allowed to escape unharmed, or be donated to an educational or research facility that possesses the appropriate FWC scientific collecting/educational use permit and is authorized to receive additional specimens of the captured species. Non-listed native commensals should either be allowed to escape unharmed or released on-site. Refer to Appendix 9 of the Permitting Guidelines for additional information on gopher tortoise commensals. If you have questions regarding handling/releasing gopher tortoise commensals encountered during capture operations, contact the Gopher Tortoise Program Coordinator's office by calling 850-921-1031.
- 13 This permit does not authorize the take of Federally-designated Endangered and Threatened species. Only individuals who are in possession of a valid permit or authorization issued by the United States Fish and Wildlife Service (USFWS) to capture or possess an eastern indigo snake or other Federally-designated Endangered and Threatened species may physically handle those species. If individuals without a USFWS permit or authorization encounter an eastern indigo snake during attempts to capture gopher tortoises or during subsequent land alteration or development activities within the property, all movement of heavy equipment and land alteration or development activities within the vicinity of the snake shall cease to allow the snake to vacate the area. No

movement of heavy equipment, or land alteration or development activities within the vicinity of the snake shall resume until the snake has vacated the work area.

A person whose substantial interests are affected by FWC's action may petition for an administrative proceeding (hearing) under sections 120.569 and 120.57 of the Florida Statutes. A person seeking a hearing on FWC's action shall file a petition for hearing with the agency within 21 days of receipt of written notice of the decision. The petition must contain the information and otherwise comply with section 120.569, Florida Statutes, and the uniform rules of the Florida Division of Administration, chapter 28-106, Florida Administrative Code. If the FWC receives a petition, FWC will notify the Permittee. Upon such notification, the Permittee shall cease all work authorized by this permit until the petition is resolved. The enclosed Explanation of Rights statement provides additional information as to the rights of parties whose substantial interests are or may be affected by this action.



**CARTER ENVIRONMENTAL
SERVICES, INC.**



7 Waldo Street
St. Augustine, FL 32084
904-540-1786

www.carterenv.com

Gopher Tortoise Burrow Location Map

2700 Atlantic Ave

Project: 5.18302

Nassau County, Florida

Date: Sep 19 2018

Figure:

4

