



**AGENDA
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
JUNE 9, 2022
10:00 AM
CITY HALL COMMISSION CHAMBERS
204 ASH STREET
FERNANDINA BEACH, FL 32034**

- 1. CALL TO ORDER**
- 2. ROLL CALL**
- 3. PLEDGE OF ALLEGIANCE**
- 4. APPROVAL OF MINUTES**
 - 4.1 Approval of Minutes for the March 10, 2022, Meeting.
 - 4.2 Approval of Minutes for the March 24, 2022, Meeting.
- 5. DISCUSSION ITEMS**
- 6. NEW BUSINESS**
 - 6.1 TRC 2022-0018 - Asa Gillette with Gillette & Associates - 2754 & 2766 Bailey Road - To discuss the development of a 12-unit single-family subdivision.
 - 6.2 TRC 2022-0019 - Asa Gillette with Gillette & Associates - Amelia Island Parkway & Amelia Road - To discuss the development of 2 medical office buildings.
- 7. COMMITTEE BUSINESS**
- 8. ADJOURNMENT**

NEXT REGULAR TRC MEETING IS SCHEDULED FOR JUNE 23, 2022

Persons with disabilities requiring accommodations in order to participate in this program or activity should contact 310-3115, TTY 711, or through the Florida Relay Services at 1-800-955-8771 (TTY number for all City offices) at least 24 hours in advance to request such accommodations.



**MINUTES
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
MARCH 10, 2022**

1. CALL TO ORDER 10:00

2. ROLL CALL

MEMBERS PRESENT:

Don Kukla, Building

Andre Desilet, Stormwater/Utilities

Michelle Forstrom,

David Neville, Arborist

Jason Higginbotham, Fire

OTHERS PRESENT:

Taylor Hartmann, Recording Secretary

3. PLEDGE OF ALLEGIANCE

4. APPROVAL OF MINUTES

4.1 Approval of Minutes for the February 24, 2022, Regular Meeting.

ACTION TAKEN: A motion was made by Mr. Kukla, seconded by Mr. Higginbotham to approve the minutes as presented.

Vote upon passage of the motion was taken by ayes and nays and being all ayes, carried.

5. OLD BUSINESS

5.1 SPR2021-0016 - Gillette & Associates - S. 13th Street - Construct a group residential project with parking lot and dry retention.

Members agreed that this project can be issued a Local Development Order.

6. ADJOURNMENT 10:04

Andre Desilet, Director/Stormwater/Utilities



**MINUTES
TECHNICAL REVIEW COMMITTEE
REGULAR MEETING
MARCH 24, 2022**

1. CALL TO ORDER 10:00

2. ROLL CALL

MEMBERS PRESENT:

Kelly Gibson, Planning

Michelle Forstrom, Code Enforcement

Jason Higginbotham, Fire

Don Kukla, Building

Andre Desilet, Stormwater/Utilities

David Neville, Arborist

OTHERS PRESENT:

Taylor Hartmann, Recording Secretary

3. PLEDGE OF ALLEGIANCE

4. OLD BUSINESS

- 4.1 TRC 2022-0018: Asa Gillette with Gillette & Associates - 2754 & 2766 Bailey Rd. - Development of a 12-lot single family residential subdivision.

Ms. Gibson introduced the case and provided the applicant an overview of the first round of comments received by TRC members.

Mr. Gillette confirmed that the school concurrency process has been initiated and is currently in the School Board's hands.

Mr. Higginbotham indicated that he and Mr. Gillette have already spoken about the necessity of an auto turn assessment to ensure emergency vehicles have adequate space to maneuver.

Mr. Desilet stated that the proposed retention pond needs to ensure that it does not adversely affect the other stormwater systems in the immediate area.

Mr. Gillette confirmed that the proposed structures will be built on stem wall foundations.

Ms. Gibson and Mr. Desilet expressed significant concern that the potential buyers of these lots are not being told that accessory structures, such as pools and/or sheds, will likely not be possible due to tree preservation requirements and impervious surface limitations. Mr. Gillette will discuss these issues with the developer.

Bob Weintraub, President of Isle de Mai, spoke about being concerned with stormwater drainage and the proximity of the proposed system to Isle de Mai's existing system.

5. ADJOURNMENT 10:31

Technical Review Committee: Application - Submission #8807

Date Submitted: 3/2/2022

Technical Review Committee: Application

USE THIS FORM TO To apply for review of site plans, preliminary subdivision plats, final subdivision plats, minor subdivisions, and amendments to previously issued local development orders, change of use, and nonresidential building expansions.

Fees

☐Initial submittal + two submittals of full site plans. \$400 + 2% of the project construction cost (whichever is greater) for up to a maximum of \$5,500.

☐3rd and subsequent resubmittals. 1/2 of original application fees

☐Change of Use Determination Letter \$50

☐Engineering Review (Consultant Hourly Rate) \$125



November 24 Meeting is cancelled in observance of Thanksgiving Day.

IMPORTANT NOTES

This application will be routed to the various departments for review of the applicable codes and design standards.

****Please note that the TRC now meets at 10:00 AM on the second and fourth Thursdays of each month (unless otherwise indicated) in City Hall Chambers.****

Please see the Land Development Code (LDC) for detailed information:



Requirements for All Site Plans – see LDC section 11.01.04



Requirements for Subdivision Plats – see LDC section 11.01.05



Determination for Completeness & sufficiency – see LDC section 11.03.01



Review and Compliance Report by the Technical Review Committee – see LDC Section 11.03.02



Construction and Improvements – see LDC section 11.05.00



Amendments to Local Development Orders – see LDC section 11.06.00

The LDC is available for review at

www.fbfl.us/LDC

Application Requirements



See Development Plan Submittal Checklist



All applications are subject to a determination of completeness as required by Land Development Code section 11.03.01.



A determination of completeness is a determination that all required documents and plans have been submitted, and whether all fees have been paid.



A determination of completeness is not a determination of compliance with substantive standards and criteria.

Key Contacts

TRC will guide your application from start to finish, engaging other City departments as needed.



PROPERTY INFORMATION

Property information can be found at the Nassau County Property Appraiser’s Website’s Map Search

Site Address*

2754 Bailey Road and 2756 Bailey Road

City*

Fernandina Beach

State*

FL

Zip*

32034

Parcel ID #(s)

00-00-31-044B-0076-0010; 00-00-30-044B-0076-0020

Lot

76

Block

Subdivision

Zoning District

R-1

Future Land Use Designation*

Low Density Residential

Overlay District*

None

Size of Property (acres)*

4.85

Present Property Use*

vacant

Wetlands*

No

Flood Zone*

X, 0.2% chance flood h

Coastal Construction Control Line (CCCL)*

No

OWNER OF RECORD

As recorded with the Nassau County Property Appraiser

First Name*

Tim

Last Name*

Ritch

Company (if applicable)

Scott Road Partners,LLC

Mailing Address*

9995 Gate Pkwy N, Ste. 400

City*

Jacksonville

State*

FL

Zip*

32246

Telephone Number*

904-928-3912

Email Address*

tim@fcginc.net

OWNER'S AGENT

If other than owner. If an agent will be representing the owner, an Owner's Authorization For Agent Representation form must be included

First Name

Asa

Last Name

Gillette

Company (if applicable)

Gillette & Associates, Inc.

Mailing Address

20 South 4th St

City

Fernandina Beach

State

FL

Zip

32034

Telephone Number

9042618819

Email Address

asa@gilletteassociates.com

PROJECT INFORMATION

Project Type*

Site Plan Review

Project Name

Island Circle

Proposed Access Road*

Bailey Road

Rd Maintenance Jurisdiction*

County Maintained

Project Estimated Cost*

400,000

Description of Request*

12 lot single family residential subdivision

Site Plan Review Checklist



The names, address, telephone number, fax number, and email address of the person preparing the plan.



The date of preparation and date(s) of any modifications, a north arrow, and a written and graphic scale.



The legal description of the property, consistent with the required survey.



A vicinity map showing the location of the property.



The location of streams, bodies of water, natural features, roads, rights-of-way, street intersections, and paved areas within the boundaries of the property.



The location of streams, bodies of water, dunes and dune systems, and other natural features within 250 feet of the boundaries of the property.



The location of the mean high-water line, if such line is within the boundaries of the property.



A topographic survey, soils report, grading plan, and an erosion control plan.



A general floodplain map indicating areas subject to inundation and high groundwater levels up to a 100-year flood classification.



A statement indicating the distances to schools and public safety facilities intended to serve the proposed development.



The name, plat book, and page number of any recorded subdivision comprising all or part of the site.



The location and use of any existing and proposed principal or accessory buildings and structures, showing proposed setbacks, building heights, and other dimensional requirements of the zoning district.



Elevations of all proposed structures.



The access points, driveway design, on-site parking, including required parking lot landscaping, internal circulation, sidewalks, and bicycle facilities.



The location of existing and proposed utilities, utility services, and easements.



A tree survey showing protected trees, proposed replacement trees, if required, and landscaping and buffering. (See Section 4.05.00.)



A soil erosion and sediment control plan compliant with Section 3.01.04.



For a PUD site plan, a detailed, written list and explanation of how the proposed PUD differs from any provision of this LDC applicable to the underlying zoning district.



For site plans and PUD site plans where development is proposed in phases, the plans shall include phase lines and the following supporting information:



- o Timeline for the development; and



- o Benchmarks for monitoring the progress of construction of each phase regarding land clearing, soil stabilization and erosion control, installation of infrastructure, and installation of landscaping.



A summary block containing:



- o Land use category from the Future Land Use Map in the comprehensive plan;



- o Zoning district;



o Total acreage;



o Total square footage for non-residential uses;



o Total density and number of units, proposed and permissible for residential uses;



o Impervious surface ratio calculation, proposed and permissible;



o Floor area ratio calculation, proposed and permissible;



o Total number of parking spaces, required and provided; and



o Number of trees required to be protected, number of trees remaining on the site, and number of trees to be planted.



Additional plans, documents, or reports that are necessary to support the application shall be submitted. Such plans, documents, or reports may include, but are not limited to:



o Subdivision Plats (subdivision plats shall contain the information set forth in Section 11.01.05.)



o Concurrency analysis (concurrency analysis reports shall contain the information set forth in Section 7.04.00.)



o Traffic analysis reports (requirements for traffic analysis reports are set forth in Section 11.01.06.)



o Parking studies (requirements for parking studies are set forth in Section 7.01.04.)



o Stormwater management plans (requirements for stormwater management plans are set forth in Section 7.03.00.)



o Environmental Impact Study (requirements for environmental impact studies are set forth by regional, State, and federal agencies with jurisdiction and in Chapter 3 of the LDC.)



o Tree Protection Plan (Section 4.05.09.)



o Landscape Plan (Section 4.05.00.)



o Lighting Plan (Section 3.05.00.), Section 7.01.04.B)



o Signage Plan (Section 5.03.00.)



o Proof of Ownership “provide a legal description, Property Identification Number (PIN) and a copy of the warranty deed.



Concurrency analysis reports shall contain the information set forth in Section 7.04.00. Requirements for traffic analysis reports are set forth in Section 11.01.06. Requirements for parking studies



“Any development application for development proposed within the Airport Overlay Zone established for the Fernandina Beach Municipal Airport shall provide confirmation of notification to the FAA as r

School Concurrency

New residential development is required to demonstrate compliance with school concurrency as regulated in the City of Fernandina Beach through the City of Fernandina Beach’s Comprehensive Plan Intergovernmental Coordination Element and the Interlocal Agreement Amendment for Public School Facility Planning adopted by the City on May 21, 2019. No new residential rezoning, preliminary plat, site plan or functional equivalent may be approved by the City unless the residential development is exempt from requirements outlined in Section 9.13 of the Amended Interlocal Agreement OR a School Concurrency Reservation Letter has been issued by the School Board indicating that adequate school facilities exist.

Certification*

☒

By signing below, I certify that the information contained in this application is true and correct to the best of my knowledge at the time of the application.

☒

I acknowledge that I understand and have complied with all of the submittal requirements and procedures.

Applicant First Name*

Tricia

Applicant Last Name*

Bowen

Today's Date*

3/2/2022

Upload Supporting Documentation*

IslandCircle-Site_03-01-22.pdf

Upload 2

IslandCircle-DrainageReport.pdf

Upload 3

AGENT FORM.pdf

Upload 4

Bailey Road.pdf

Upload 5

J19950 Artisan Bailey Subdivision with signature.pdf

Upload 6

51884 November 23, 2021.pdf

Upload 7

Choose File No file selected



DEPARTMENT OF PLANNING & CONSERVATION

204 Ash Street | Fernandina Beach, FL 32034 | 904 310-3480 | www.fbfl.us/planning



Civil & Environmental Engineering • Mechanical & Structural Engineering • Construction Management

AGENT AUTHORIZATION

I, Timothy Ritch of Scott Road Partners, LLC the owner of the following property(s) do hereby authorize, Gillette & Associates, Inc. to act as my Agent in conjunction with all site plan applications and any other Local, State and/or Federal development permits necessary for development of the following property, without any further notice.

Property ID Number(s): 00-00-31-044B-00076-0010; 00-00-30-044B-0076-0020

Location: 2754 Bailey Road and 2766 Bailey Road, Fernandina Beach

[Signature] February 3, 2022 904-928-3912
Signature Date Telephone Number

STATE OF FLORIDA
COUNTY OF DUVAL

Subscribed and sworn to before me this 3RD day of February, 2022.

Who is personally known to me Tim Ritch or has produced identification

[Signature] Kimberly R. Simmons
Notary Public: Signature Printed Name

My Commission Expires: 02/25/2024



20 SOUTH 4TH STREET • AMELIA ISLAND, FLORIDA 32034 PHONE: (904) 261-8819

PREPARED BY AND RETURN TO:

Name: Wood & Smith, PA
Address: 303 Centre Street
Suite 100
Fernandina Beach, FL 32034

File No: 21-203023

Parcel ID #00-00-30-044B-0076-0010
#00-00-30-044B-0076-0020

(Space Above This Line For Recording Data)

Personal Representative's Deed

THIS INDENTURE made on this **14th** day of **December, 2021**, by **Debra Jones Portier, Personal Representative of the Estate of Robert P. Jones, deceased**, whose address is **2669 Bailey Road, Fernandina Beach, FL 32034 ("Grantor")** to **Scott Road Partners, LLC, a Florida limited liability company**, whose address is **9995 Gate Parkway North, Suite 400, Jacksonville, FL 32246 ("Grantee")**.

WITNESSETH: That Grantor, for good and valuable consideration in hand paid, grants, bargains, sells aliens, remises, releases, conveys and confirms to the Grantee, its heirs and assigns forever, the real property lying and being in Nassau County, Florida, and more particularly described as follows:

**All of Lot 76 of OCEAN BREEZE FARMS a subdivision of a portion of Sections 3 and 5, Township 2 North, Range 28 East; Nassau County, Florida, according to plat of said Subdivision recorded in Plat Book 2, page 19, public records of said Nassau County, Florida,
Less the Westerly 10 feet of said Lot 76 which has been taken to widen Bailey Road;
and also Except that portion thereof conveyed to Nassau County in Book 77, Page 520; Book 78, Page 525; & Book 79, Page 307, public records of Nassau County, Florida.**

TOGETHER with all and singular the tenements, hereditaments and appurtenances belonging or in any way appertaining to said Property.

TO HAVE AND TO HOLD the same to the Grantee, its heirs and assigns, in fee simple forever.

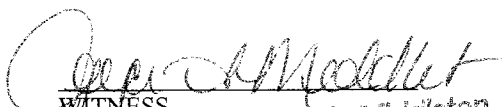
AND the Grantor does covenant to and with the Grantee, its heirs and assigns, that in all things preliminary to and in and about this conveyance that laws of the State of Florida have been followed and complied with in all respects.

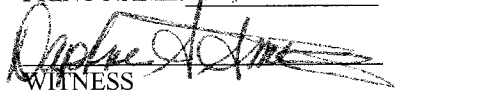
****SIGNATURE PAGE TO FOLLOW****

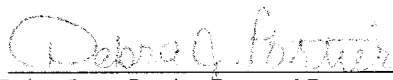
PERSONAL REPRESENTATIVE'S DEED
PAGE 2 of 2

IN WITNESS WHEREOF, the Grantor, Debra James Portier, Personal Representative of the Estate of Robert P. Jones, has set his/her hand and seal on the day and year first above written.

Signed, sealed and delivered in the presence of 2 witnesses:

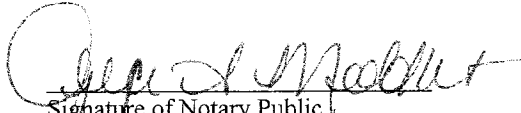

WITNESS
PRINT NAME: Joyce A. Middleton

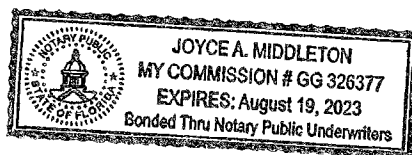

WITNESS
PRINT NAME: Daphne S. Smith

By: 
Debra Jones Portier, Personal Representative
of the Estate of Robert P. Jones

STATE OF FLORIDA
COUNTY OF NASSAU

The foregoing instrument was acknowledged before me by means of (☒) physical presence or () online notarization this 14 day of December, 2021, by Debra Jones Portier, Personal Representative of the Estate of Robert P. Jones.


Signature of Notary Public
Print, Type/Stamp Name of Notary



Personally Known: _____ OR Produced Identification: _____
Type of Identification:
Produced: Id Number



**REPORT OF PRELIMINARY GEOTECHNICAL EXPLORATION
& ENGINEERING EVALUATION
PROPOSED 12-LOT ARTISAN BAILEY RESIDENTIAL SUBDIVISION - LOTS, ROAD AND POND
SE QUADRANT OF BAILEY AND PINE ROADS
FERNANDINA BEACH, FLORIDA
AGES JOB NO. J19950, REPORT NO. 001**

**FOR
GILLETTE & ASSOCIATES
20 SOUTH 4TH STREET
FERNANDINA BEACH, FLORIDA 32034
ATTN: MR. ALEX BOLTON**

SEPTEMBER 3, 2021

**AGES of JAX, Inc.
P O Box 24008, Jacksonville FL 32241-4008
9556 Historic Kings Road South, Suite 201 ● Jacksonville, Florida 32257-2010
(904) 886-0766 ● (904) 880-5190 (FAX)**



OF JAX, INC.

September 3, 2021

GILLETTE & ASSOCIATES

20 South 4th Street
Fernandina Beach, Florida 32034
Attn: Alex Bolton

Re: Report of Preliminary Geotechnical Exploration & Engineering Evaluation
Proposed 12-lot Artisan Bailey Residential Subdivision - Lots, Road and Pond
SE Quadrant of Bailey and Pine Roads
Fernandina Beach, Florida
AGES Job No. J19950, Report 001

Ladies & Gentlemen:

AGES OF JAX (AGES), Inc. has completed a preliminary geotechnical exploration of the subsurface conditions beneath the construction area for the referenced project. Our services were performed in accordance with **AGES'** Service Authorization which was dated July 19, 2021. Formal authorization of our services was provided by Mr. Asa Gillette, P.E. on July 20, 2021. This report includes: (1) a brief description of the planned construction, (2) a summary of the field exploration performed and results that were obtained, (3) our recommendations for foundation design and construction of the proposed residential structures, (4) guideline pavement design and construction recommendations, (5) evaluation of the suitability of barrow soil located at the site, and (6) parameters for use in stormwater pond design and permitting.

The site preparation recommendations and foundation design recommendations presented herein are specific to the tested lots. Soil conditions on untested lots could be markedly different and the recommendation presented herein may or may not be applicable. If a higher confidence level is desired concerning the soil conditions on the untested lots, then we recommend that **AGES** be retained to perform supplemental penetration test borings prior to initiation of building construction.

We appreciate the opportunity to be of service as the geotechnical consultant during this phase of your project. Should you have any questions concerning this report or if we may be of any further service to you, please feel free to contact us.

Very truly yours,

Digitally signed
by Billy C
McMahan Jr P.E

AGES of JAX, INC.

Date:
2021.09.03
12:13:02 -04'00'

Bill C. McMahan, Jr., P.E.
Principal Engineer/Vice-President
Registered, Florida No. 42677

BCM/bcm/J19950 Artisan Bailey Subdivision.wpd
Attachments
Distribution: Addressee (1) Via Email

AGES of JAX, Inc.

P O Box 24008, Jacksonville FL 32241-4008
9556 Historic Kings Road South, Suite 201 ● Jacksonville, Florida 32257-2010
(904) 886-0766 ● (904) 880-5190 (FAX)

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Generalized Subsurface Profile	
Key to Soil Classification	
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Field Exploration Procedures	

REPORT ATTACHMENTS

1.0 PROJECT BACKGROUND INFORMATION

General information has been provided by Gillette & Associate Alex Bolton during recent telephone and email conversations. We have been provided with: (1) a Site Plan which shows the property boundaries, the planned site development and the requested preliminary boring locations (prepared by Gillette & Associates, undated; and (2) a Boundary and Topo Survey (prepared by Bowman Surveyors, undated).

We understand that a new 12-lot single family residential subdivision is being planned on a +/- 5-acre parcel of land located near the southeast corner of the Bailey Road and Pine Road in Fernandina Beach, Florida. The site is occupied by several mobile homes and pond. The western half of the site is grass covered. The eastern half of the site is wooded. The proposed development will include 12 - lots, asphalt paved roadways and the existing pond will be expanded to store/treat stormwater runoff. Detailed structural information has not been provided however we have assumed that the buildings will be wood or block framed and that column and wall loads will be on the order of 75 kips and 2 klf, respectively. Existing and proposed grading information has also not been provided. However we have assumed that 2 feet or less of elevating fill will be added to the site.

2.0 PRELIMINARY GEOTECHNICAL EXPLORATION

2.1 Purpose of Exploration

The objective of this geotechnical exploration was to obtain site and subsurface data for use in: (1) the preliminary evaluation of the site with respect to the proposed development, (2) the development of preliminary site preparation and foundation design recommendations for the planed residential structures, (3) the development of pavement design and construction recommendations, (4) the stormwater pond design and permitting processes, and (5) the evaluation of the pond areas soils for use as barrow fill and backfill. The geotechnical exploration should therefore consist of field exploration borings and a geotechnical engineering analysis of the collected test data.

2.2 Subsurface Testing

The field testing program, which was conducted on August 17 and 18, included: seven(7) 15-feet deep penetration test borings (B1 - B7) in the lot areas (one boring every other lot), three(3) 6-feet deep auger borings in the roadway areas (A1 - A3), and two(2) 15-feet deep auger borings in the planned pond expansion areas

The soil borings were located in the field by our personnel using GPS based locators with a 10 to 15 foot horizontal accuracy. The borings were drilled according to applicable standards. The boring locations are shown on the attached Field Exploration Plan should be considered approximate. The ground surface elevations at the boring locations were not determined.

The attached Test Boring Records and Auger Boring Records present the descriptions of the subsurface soils encountered, the groundwater levels encountered, and the penetration resistance values recorded when drilling and sampling the test borings. The subsurface conditions are graphically illustrated on the attached Generalized Subsurface Profile sheet. The stratification lines and depth designations on the boring records and profile represent the approximate boundary between the various soils encountered, as determined in the field by our drillers, and the transition from one strata to the next should be considered approximate. A brief discussion of the drilling, sampling, and field testing techniques used during the geotechnical exploration is provided in the attached Field Exploration Procedures section.

3.0 GEOTECHNICAL FINDINGS

3.1 Site Conditions

The subject site was visited by **AGES** personnel to identify site conditions which could impact our site preparation and foundation design recommendations. The site which is located in the southeast corner of the Bailey Road and Pine Road in Fernandina Beach, Florida, is occupied by several mobile homes and pond. The western half of the site is grass covered. The eastern half of the site is wooded. No standing water was observed on the site at the time of our visit.

3.2 Subsurface Conditions

The subsurface conditions outlined below highlight the major subsurface stratifications encountered during our geotechnical exploration of the site. When reviewing the boring records, the attached Generalized Subsurface Profile and the subsurface conditions outlined below, it should be understood that the subsurface conditions will vary across the proposed construction area and between the boring locations.

3.2.1 Soil Conditions - The lot penetration test borings (B1 - B7) typically encountered a 2 to 8 feet thick surface stratum of very loose to loose slightly silty fine sand (SP-SM). Penetration resistance values within this stratum ranged from 2 to 9 blows/foot. The surface stratum was underlain by alternating strata firm to very firm to dense slightly silty fine sand (SP-SM) and dense cemented slightly silty fine sand (SP-SM, Hardpan) which extended to the boring termination depth of 15 feet. The penetration resistance values typically ranged from 14 to 46 blows/foot.

Roadway auger borings A1 - A3 encountered slightly silty fine sand (SP-SM, A-3) which extended to the maximum boring termination depth of 6 feet.

Lake auger borings LA1 and LA2 encountered slightly silty fine sand (SP-SM, A-3) which extended to the maximum boring termination depth of 15 feet.

3.2.2 Groundwater Conditions - The groundwater levels encountered at the time of drilling ranged from approximately 1 to 2 feet below the ground surface. These water levels are estimated to be at the Seasonal High Groundwater Levels. The groundwater level should be expected to fluctuate due to seasonal climatic variations, changes in surface water runoff patterns across the site, construction activity and the subsequent development as planned, and other interrelated factors. Since groundwater variations are anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based on the assumption that variations will occur.

4.0 PRELIMINARY GEOTECHNICAL ENGINEERING EVALUATION

4.1 Basis of Evaluation & Recommendations

Our geotechnical engineering evaluation of the site and subsurface conditions with respect to the planned construction, and our recommendations for foundation design and site preparation/earthwork construction, are based upon: (1) our site observations; (2) the field test data obtained during this geotechnical exploration of the site; and (3) our understanding of the project information and structural loading conditions as presented in this report. If the structural information or the Site Plan are altered, please contact us so we may review our recommendations. If any site or subsurface conditions are encountered during construction which appear to deviate from data obtained during this geotechnical exploration (as documented herein), please contact us so that we may visit the site, observe the differing conditions, and evaluate the new information relative to our geotechnical evaluation and recommendations.

4.2 Foundation System Evaluation

Based upon our geotechnical findings and our understanding of the proposed construction, we consider the site and subsurface conditions adaptable for support of the proposed structures upon a properly designed and constructed shallow foundation. The shallow foundation system may be designed using an allowable bearing pressure of 3,000 psf. Provided that the design and construction recommendations outlined subsequently are implemented, we estimate that total structural settlement due to fill and structural loads should be on the order of one inch or less. In addition, post construction differential settlements should be within tolerable magnitudes ($< \frac{1}{2}$ inch). The settlements should occur concurrently with construction due to the generally sandy nature of the soils encountered. Site preparation necessary for proper foundation performance includes:

- * Clearing and grubbing the existing vegetation (and their root systems) and stripping of surficial topsoil from within the construction areas.
- * Compaction of the exposed natural soils using a 10 ton vibratory drum roller to improve the bearing capacity and reduce settlement potential of the upper 2 to 6 feet of very loose to loose fine sand;
- * Placement and compaction of any required elevating fill to densities equivalent to 95 percent of the Modified Proctor maximum dry density (ASTM D 1557) within the building areas.

4.3 Evaluation of Barrow Soil Suitability

The evaluation of barrow soil suitability depends upon: (1) the planned soil usage, (2) proposed construction methods, (3) the time of year during which site work construction will occur, (4) economic considerations, and (5) other factors. From a building pad perspective, we typically recommend that all fill and backfill soils consist of clean sands (fine sand to slightly silty fine sand) containing less than 12% fines (SP and SP-SM). Higher fines content sandy soils including: silty sands and clayey sands (SM and SC) can also be used in building pad development. However, these soils are more moisture sensitive and re-compaction in Florida during the wet summer months and/or use of the fill on wet sites will significantly increase re-compaction effort and could result in construction delays. Very silty sands, very clayey sands, and/or silts/clays (CH and CL), are not considered to be usable for building pad construction in Florida.

Based upon a review of borings LA1 and LA2, it is our opinion that the fine sands and slightly silty fine sands (SP to SP-SM, A-3) which were encountered in the upper 15 feet of profile will be usable as Class I structural fill/backfill.

5.0 PRELIMINARY FOUNDATION DESIGN & SITE PREPARATION RECOMMENDATIONS

5.1 Foundation Design Recommendations

5.1.1 Foundation Type - We consider a conventional shallow foundation system applicable for this project if the site preparation recommendations presented in section 5.2 of this report are performed.

5.1.2 Bearing Pressure & Depth - The maximum allowable soil bearing pressure for use in shallow foundation design should not exceed 3,000 psf. The foundations should be designed based upon the maximum load which could be imposed by all loading conditions. The exterior footings should bear at a depth of at least 12 inches below the exterior final grades and the interior footings should bear at least 12 inches below the interior floor slab. These minimum bearing depths should provide the necessary confinement for the soils at the foundation bearing levels.

5.1.3 Foundation Size - The minimum widths recommended for isolated spread-type footings and continuous wall footings are 24 and 18 inches, respectively. Even though the maximum allowable soil bearing pressure may not be achieved, these minimum width recommendations should still control the size of the foundations.

5.1.4 Bearing Material - The foundations may bear in either the compacted suitable natural granular soils or compacted structural backfill or fill. The bearing level soils, after compaction, should exhibit densities equivalent to 95 percent of the Modified Proctor maximum dry density (ASTM D 1557) to a depth of at least one-foot below the foundation bearing levels.

5.2 Site Preparation Recommendations

5.2.1 Temporary Groundwater Control - During our geotechnical exploration, the groundwater level was encountered at depths of 1 to 2 feet below the existing site grades. Dependent upon the groundwater levels encountered at the time of construction, temporary de-watering may be required in some areas of the site. The groundwater should be controlled at all times at a depth of at least two feet below the construction level. Groundwater drawdowns on the order of one to two feet, if required, can probably be best achieved using temporary perimeter drainage ditches, four to five feet deep, which are graded: (1) to a positive gravity outfall away from the site; or (2) to sumps where the collected groundwater and surface water runoff can be removed by pumping.

5.2.2 Site Clearing & Stripping - Initial site preparation should consist of clearing existing trees, vegetation, root systems and surficial topsoil from within the planned construction area. The clearing/stripping work should be performed within and to a distance of at least five feet beyond the perimeter of the planned building areas and three feet beyond the perimeter of the proposed pavement areas. It should be anticipated that surficial topsoil will be encountered in thicknesses of up to 6 inches. The perimeter areas may then need to be graded to help direct surface water runoff away from the construction areas.

5.2.3 Site & Fill Compaction - After completing the initial site preparation, the exposed sandy soils should be compacted to densities of at least 95 percent of the Modified Proctor maximum dry density to a depth of 24 inches using a vibratory drum roller which has a minimum at-drum weight on the order of 10 tons and a minimum drum diameter on the order of five feet, with the exception that a lightweight vibratory drum or sled compactor having a maximum static weight on the order of 1000 pounds should be used within 50 feet of existing structures. The roller will be required to improve the density of the upper approximate two feet of loose fine sands at this site. The structural fill required to raise the site to the planned finished grades may then be placed in loose lifts not exceeding 12 inches in thickness, and should be compacted to densities as recommended above. Loose lifts should not exceed eight inches in areas of the site in which lightweight vibratory equipment is used. Structural fill is defined as a non-plastic, inorganic, granular soil containing less than 12 percent material passing the No. 200 mesh sieve (i.e. a relatively clean sand).

5.2.4 Disturbed Soil Conditions - Should the near-surface soils and/or structural fill material experience "pumping" and subsequent soil strength loss during site work construction, work upon these areas should be immediately terminated and: (1) the disturbed soils removed and backfilled with "dry" structural fill soils, (i.e. percent water content on the order of five to 10 percent) which are then compacted; or (2) the excess moisture content within the disturbed soils allowed to dissipate before recompacting. Furthermore, the groundwater table should be checked and controlled as necessary to help insure proper draw-down of any high groundwater conditions that may be causing the "pumping" conditions during compaction or construction activity upon these soils.

5.2.5 Foundation Areas - After placement and compaction of all structural fill, the foundations may be excavated to their planned bearing levels using a "smoothed" bucket backhoe. All fine sandy soils present at the bearing level should be compacted to densities equal to 95% of the Modified Proctor maximum dry density. Compaction of the fine sandy bearing level soils in the small footing areas may be

best achieved using lightweight, walk-behind sleds, rollers, or tampers having a minimum total weight of 100 lbs. Loose lifts of backfilled fine sandy soil in the footing excavations, if needed, should be placed in thicknesses of up to 6 inches, prior to densification with the lightweight compaction equipment.

6.0 PRELIMINARY GUIDELINE PAVEMENT DESIGN AND CONSTRUCTION RECOMMENDATIONS

6.1 General

The subsurface conditions at this site appear favorable for use of flexible and/or rigid pavements in the parking lot and drive-through areas. Guideline pavement design and construction recommendations applicable to this project are presented below. The pavement thickness recommendations and construction recommendations presented in this report are based upon our past experience on similar paving projects. Detailed flexible pavement design calculations have not been performed to verify adequacy of the recommended pavement sections since detailed traffic loading information is unavailable at this time. We request that **AGES** be retained to review the final pavement design, paving and drainage plans, and specifications to evaluate whether our guideline recommendations have been properly interpreted and implemented into the project design.

6.2 Guideline Pavement Design Recommendations

The recommended minimum flexible pavement structure thicknesses for the subject project are provided in the following table.

<u>PAVEMENT COURSE</u>	<u>Automobiles & Light Truck Traffic</u>	<u>Moderate to Heavy Traffic</u>
Asphalt Wearing Surface(In.)*	1.5	2.0
Limerock Base Course (In.)**	6.0	8.0
Prepared Subgrade(In.)***	12.0	12.0

* Superpave 9.5 Asphaltic Concrete or Type SIII, as defined by the Florida DOT Standard Specifications for Road and Bridge Construction Manual.

** Florida Limerock, exhibiting an LBR Value of 100, and meeting the specifications outlined in the Florida DOT manual (Section 200).

*** Stabilized and compacted subgrade should exhibit a Maximum LBR value of at least 30. Subgrade stabilization should be performed in accordance with Section 160 of the standard FDOT specifications.

6.3 Guideline Pavement Construction Recommendations

6.3.1 Pavement Subgrade Preparation - Site preparation in the pavement areas should be performed in accordance with the guideline procedures outlined above, and should include: (1) clearing/stripping surface vegetation, (2) compaction of the exposed soils using a 10 ton vibratory drum roller, and (3) placement/compaction of backfill and any required structural fill (if any) to densities equivalent to 95 percent of the Modified Proctor maximum dry density. In addition, the upper one foot of subgrade in the pavement areas should be compacted to 98 percent of the Modified Proctor maximum dry density. Shell and or clay additives may be required to obtain the required design LBRs required. Also, the pavement structure should be designed to provide at least 30-inches of fine sands separating the limerock subgrade and any underlying clayey/silty soils. This will help to prevent "pumping" and reduce the chance of premature pavement failures which are caused by "perched" groundwater levels.

6.3.2 Limerock Base Course Preparation - The limerock base course should be placed and compacted to densities equivalent to 100 percent of the Modified Proctor maximum dry density (ASTM D 1557). The surface of the limerock base course material should be "primed" with any of the following cutback asphalt grades: RC-70, RC-250, MC-70, or MC-250. Approximately 0.15 gallons per square yard should be applied to the base course surface. Care should be exercised to prevent over-priming of the base course surface. Prior to asphalt placement, the prime coat should be fully set and cured. The specifications governing the placement of prime coats on pavement base course surfaces and sanding is outlined in Section 300 of the Florida DOT Standard Specifications for Road and Bridge Construction (1986).

The primed base course should be sanded if: (1) more than one day (24 hours) will elapse prior to laying the asphalt wearing surface and excess prime is not absorbed, or (2) construction equipment will operate over the unprotected primed base course. If sanding is required, approximately 10 pounds of sand per square yard of surface (i.e., about 1/4 inch of sand) should be spread over the primed base course. The sand surface coating should contain less than 10 percent fines (i.e., material passing the No. 200 sieve) and be composed of non-elastic, inorganic, granular soil. If krushcrete is substituted for the limerock base, then priming and sanding will not be required.

6.3.3 Asphalt Preparation - The placement and compaction of an asphaltic concrete wearing surface should be performed in general accordance with specifications outlined in the Florida DOT Road and Bridge Construction manual. Prior to asphalt placement, the base course should be broomed to remove any excess sand and then tacked with a bituminous binder.

6.4 Permanent Groundwater Control Measures

Groundwater control is of utmost importance in maintaining the integrity of flexible pavement structures. In areas where groundwater is anticipated to exist within 24 inches of the bottom of the pavement base course, a properly designed and constructed under drain system will be necessary to maintain this 24-inch separation between the pavement base course and the groundwater level. After finalizing the civil grading plan, we would be pleased to review this plan and evaluate the need for an under drain system. If required, we can design an under drain system for this site.

7.0 QUALITY CONTROL TESTING GUIDELINES

Prior to initiating compaction operations, we recommend that representative samples of the structural fill material and exposed in-place fine sandy soils be collected and tested to determine their compaction and classification characteristics. The maximum dry density, optimum moisture content gradation and plasticity characteristics should be determined. These tests are needed for compaction quality control of the structural fill and existing soils and to determine if the fill material is acceptable.

A representative number of field in-place density tests should be performed in the compacted existing soils and in each one foot lift of structural fill/backfill to confirm that the required degree of compaction has been obtained. At each test level, we recommend one density test be performed for every 5,000 square feet of building area (minimum of three locations). In-place density tests should also be performed at representative locations in the bearing level soils of the footings. We recommend that at least one density test be performed at a minimum of 25 percent of the isolated column footing bearing areas, and for every 100 linear feet of continuous footing bearing surface. In addition, in-place density tests should be performed at one location for every 10,000 square feet of pavement area (minimum of three locations).

8.0 CONSTRUCTION PLANS & SPECIFICATIONS REVIEW

We recommend that **AGES** be provided the opportunity to review the plans and the specifications prepared from the recommendations presented in this report. If necessary, we will suggest any modifications that may be required to verify that our recommendations have been properly interpreted and implemented. Our report has been written in a guideline recommendation format and is not appropriate for use as (or inclusion into) the specifications without being reworded in a specification type format. It is recommended that this report not be made part of the contract documents. However, it should be made available to prospective contractors for information purposes only.

DATA ATTACHMENTS

Site Location Map

Field Exploration Plan

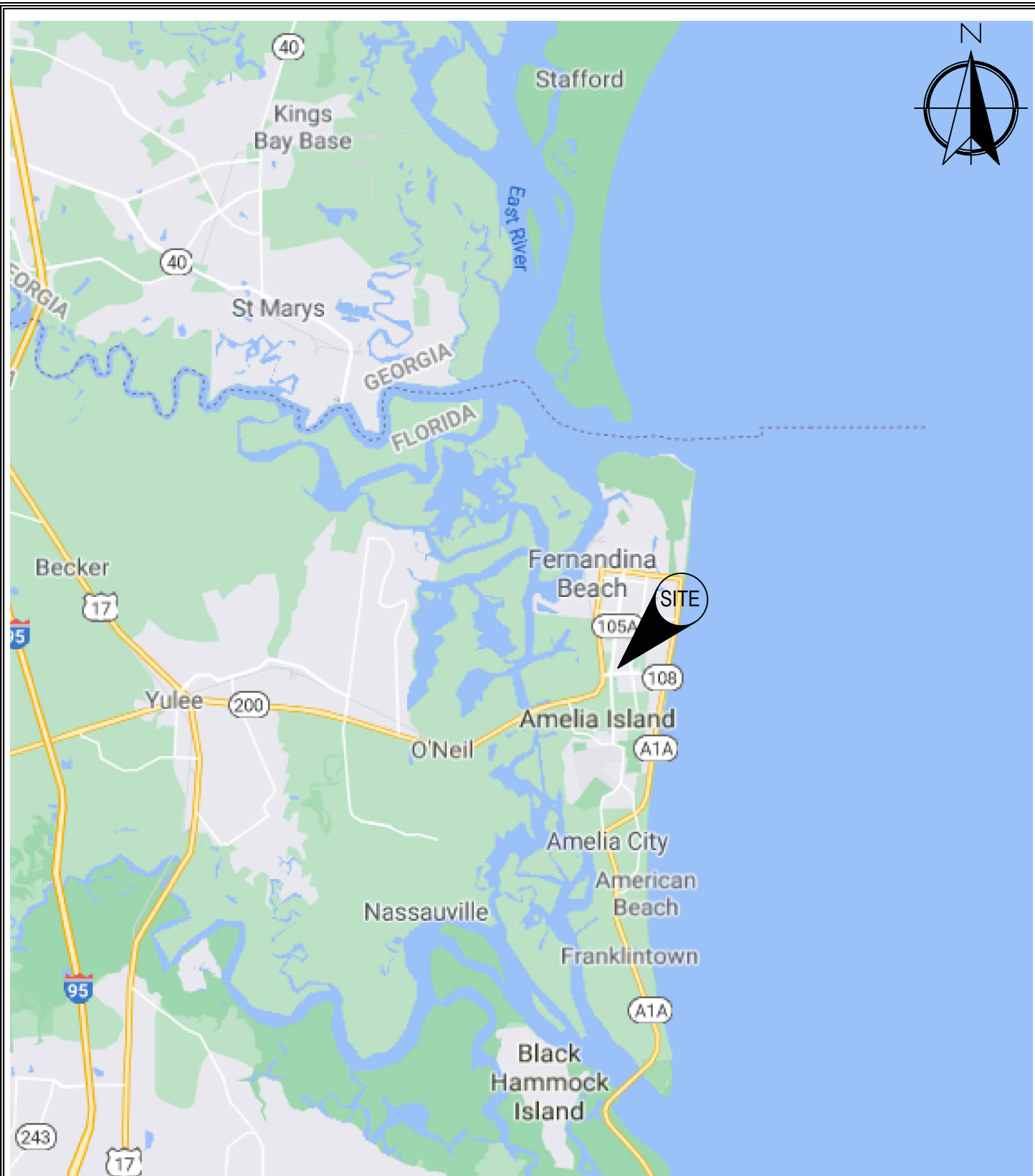
Generalized Subsurface Profile

Key to Soil Classification

Test Boring Records

Auger Boring Records

Field Exploration Procedures



REFERENCE:

Google.com



AGES of JAX Inc.
9556 Historic Kings Rd.
Jacksonville, Florida 32257

SITE LOCATION MAP

Artisan Bailey Subdivision
Fernandina Beach, Florida

DRAWN: DEP

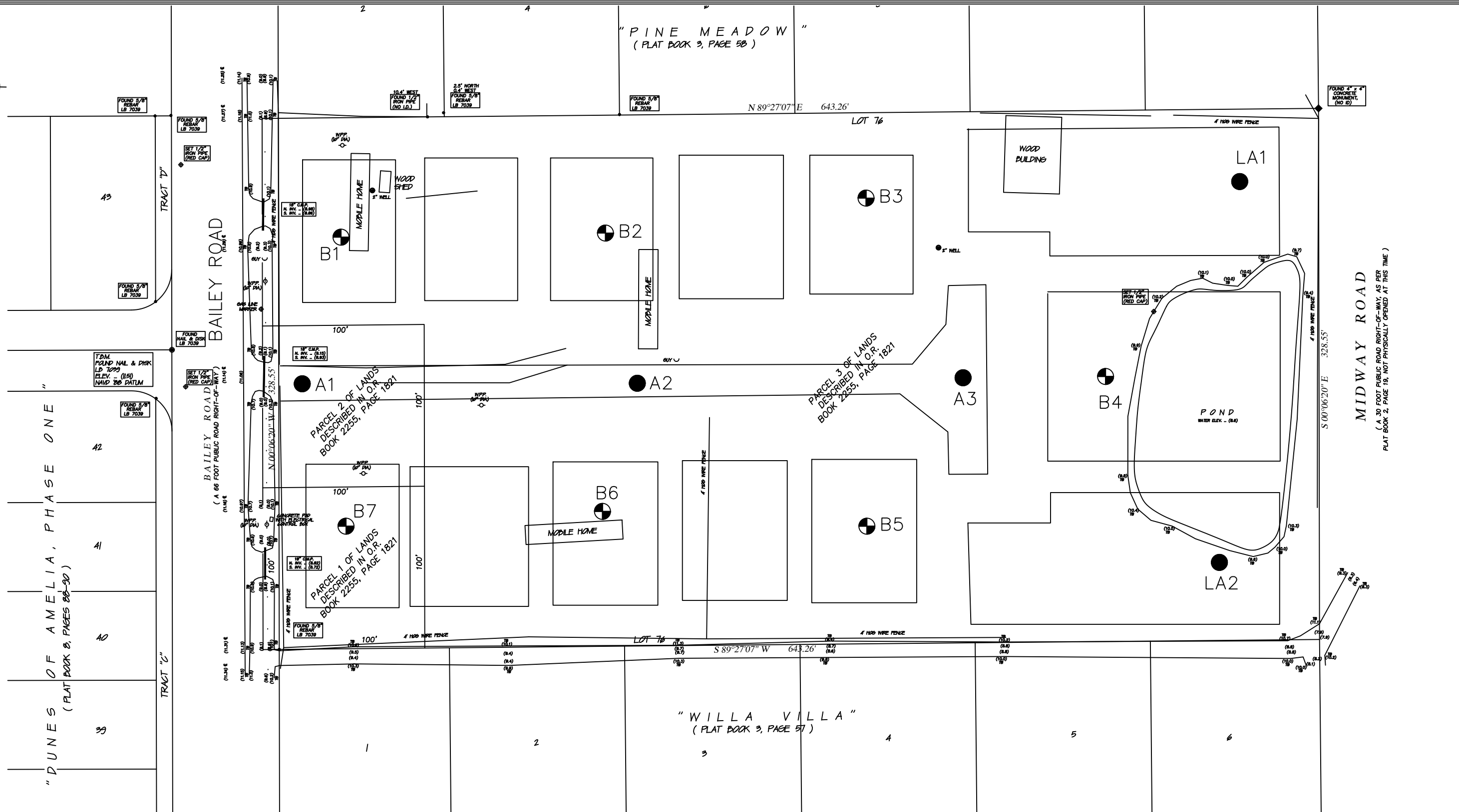
DATE: 09/01/21

SCALE:

CHECKED: BCM

JOB NO: J19950

NTS

**LEGEND:**

- ⊕ Penetration Test Boring Location (approximate)
- Auger Boring Location (approximate)

REFERENCE:

Site Plan provided by Gillette & Associates, undated.

NOTES:

The boring locations were established by AGES personnel using handheld GPS Locator Units. Boring locations as shown on this plan are approximate.



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Jacksonville, Florida 32257

FIELD EXPLORATION PLAN

Artisan Bailey Subdivision
Fernandina Beach, Florida

DRAWN: DEP

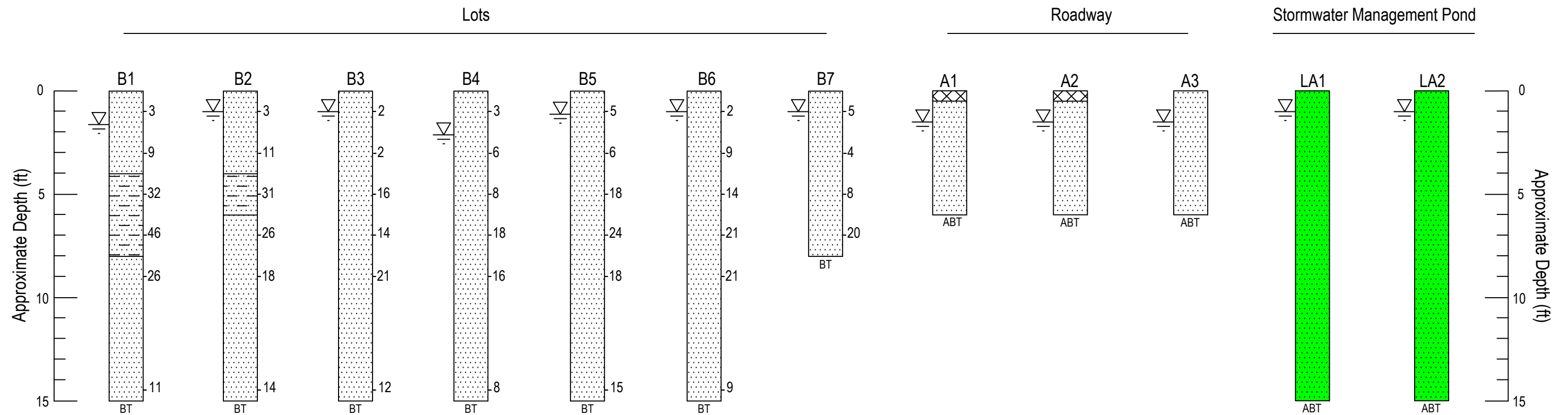
DATE: 09/01/21

SCALE:

CHECKED: BCM

JOB NO: J19950

1"=120'



FILL/BACKFILL CLASSIFICATION

- | | |
|---|---|
|  | CLASS I FILL/BACKFILL - Use as Trench Backfill, Elevating Fill in Roadway & Building Pads Areas |
|  | CLASS II FILL - Use as Elevating Fill in Building Pad Areas & Green Areas |
|  | UNSUITABLE - Not Suitable For Use as fill or backfill |

LEGENDS & SYMBOLS (USCS/AASHTO CLASSIFICATION)

- | | | | | | |
|---|--------------------------------------|---|--|---|--|
|  | Fine SAND (SP/A-3) |  | Silty CLAY (CH/A-7) |  | Standard Penetration Resistance (blows/ft) |
|  | Slightly Silty Fine SAND (SP-SM/A-3) |  | Sandy CLAY (CL/A-7) | | |
|  | Silty Fine SAND (SM/A-2-4) |  | Sandy SILT (MH/ML/A-4) | | |
|  | Very Silty Fine SAND (SM/A-4) |  | Variably Cemented
Fine SAND (HARDPAN) | | |
|  | Clayey Fine SAND (SC/A-2-6) |  | Fine SAND with Many Organics (A-8) | | |
|  | Silty Clayey Fine SAND (SM-SC/A-2-5) | | | | |
|  | Very Clayey Fine SAND (SC/A-6) | | | | |
| | | | |  | Boring Terminated |
| | | | |  | Auger Boring Terminated |
| | | | |  | Topsoil |
| | | | |  | Groundwater Level
Not Encountered |
| | | | |  | Groundwater Level
at Time of Drilling |



AGES of JAX Inc.
9556 Historic Kings Rd.
Jacksonville, Florida 32257

GENERALIZED SUBSURFACE PROFILE

Artisan Bailey Subdivision
Fernandina Beach, Florida

DRAWN: DEP

DATE: 09/01/21

SCALE:
1"=5'

CHECKED: BCM

JOB NO: J19950

KEY TO SOIL CLASSIFICATION

CORRELATION OF N-VALUE WITH RELATIVE DENSITY & CONSISTENCY

SANDS AND GRAVEL	
NO. OF BLOWS, N'	RELATIVE DENSITY
0 - 4	VERY LOOSE
5 - 10	LOOSE
11 - 20	FIRM
21 - 30	VERY FIRM
31 - 50	DENSE
OVER 50	VERY DENSE

SILTS AND CLAYS	
NO. OF BLOWS, N*	CONSISTENCY
0 - 2	VERY SOFT
3 - 4	SOFT
5 - 8	FIRM
9 - 15	STIFF
16 - 30	VERY STIFF
31 - 50	HARD
OVER 50	VERY HARD

PARTICLE SIZE IDENTIFICATION (UNIFIED SOIL CLASSIFICATION SYSTEM)

CATEGORY	DIMENSIONS
Boulders	Diameter exceeds 12 inches
Cobbles	3 to 12 inches
Gravel	Coarse - 0.75 to 3 inches in diameter
Sand	Coarse - 2.0 mm to 4.76 mm diameter Medium - 0.42 mm to 2.0 mm diameter
Silt and Clay	Less than 0.074 mm (Invisible to the naked eye)

MODIFIERS

These modifiers provide our estimate of the amount of minor constituents (sand silt or clay size particles) in the soil sample.

PERCENTAGE OF MINOR CONSTITUENT	MODIFIERS
5% to 12%	Slightly Silty, Slightly Clayey, Slightly Sandy Silty, Clayey, Sandy Very Silty, Very Clayey, Very Sandy
12% to 30%	
30% to 50%	

These modifiers provide our estimate of the amount of other components in the soil sample.

APPROXIMATE CONTENT OF OTHER COMPONENTS (SHELL, GRAVEL, ETC.)	MODIFIERS	APPROXIMATE CONTENT OF ORGANIC COMPONENTS
0% to 5%	TRACE FEW SOME MANY	1% to 2%
5% to 12%		2% to 4%
12% to 30%		4% to 8%
30% to 50%		>8%



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.5' Time: Drilling Date: 08/17/21
 Lat/Long N 30.67906° / W 81.46594°

JOB NO J19950
 BORING NO B1
 Sheet 1 of 1
 Boring Begun 08/17/21
 Boring Completed 08/17/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST					
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT	
	0								
		VERY LOOSE Dark Grey Brow Slightly Silty Fine SAND (SP-SM)	1	1	2	1	3	3	
		VERY LOOSE Grey Brown Slightly Silty Fine SAND (SP-SM)							
		LOOSE Brown Slightly Silty Fine SAND (SP-SM)	2	3	4	5	5	9	
	5		3	6	8	24	26	32	
		DENSE Brown Slightly Silty Fine SAND (SP-SM) (HARDPAN)							
			4	12	30	16	30	46	
			5	7	10	16		26	
	10	VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)							
		FIRM Brown Slightly Silty Fine SAND (SP-SM)							
	15		6	4	4	7		11	
		BORING TERMINATED							
	20								
25									
30									

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B1



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.0' Time: Drilling Date: 08/17/21
 Lat/Long N 30.62909° / W 81.46533°

JOB NO J19950
 BORING NO B2
 Sheet 1 of 1
 Boring Begun 08/17/21
 Boring Completed 08/17/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST					
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT	
	0								
		VERY LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)	1		1	1	2	2	3
		VERY LOOSE Grey Brown Slightly Silty Fine SAND (SP-SM)							
		FIRM Brown Slightly Silty Fine SAND (SP-SM)	2		9	5	6	6	11
	5	DENSE Dark Brown Slightly Silty Fine SAND (SP-SM) (HARDPAN)	3		7	19	12	16	31
		VERY FIRM Dark Brown Slightly Silty Fine SAND (SP-SM)	4		13	13	13	13	26
			5		8	8	10	12	18
	10								
		FIRM Brown Slightly Silty Fine SAND (SP-SM)							
	15		6		3	5	9		14
		BORING TERMINATED							
	20								
25									
30									

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B2



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.0' Time: Drilling Date: 08/16/21
 Lat/Long N 30.62902° / W 81.46495°

JOB NO J19950
 BORING NO B3
 Sheet 1 of 1
 Boring Begun 08/16/21
 Boring Completed 08/16/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST				
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT
	0							
		VERY LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)	1	1	1	1	1	2
		VERY LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)						
		VERY LOOSE Brown Slightly Silty Fine SAND (SP-SM)	2	1	1	1	4	2
	5		3	2	8	8	11	16
		FIRM to VERY FIRM Light Brown Slightly Silty Fine SAND (SP-SM)	4	5	6	8	8	14
			5	7	10	11		21
	10							
		FIRM Grey Brown Slightly Silty Fine SAND (SP-SM)						
	15		6	4	7	5		12
		BORING TERMINATED						
	20							
	25							
	30							

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B3



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 2.0' Time: Drilling Date: 08/16/21
 Lat/Long N 30.62887° / W 81.46439°

JOB NO J19950
 BORING NO B4
 Sheet 1 of 1
 Boring Begun 08/16/21
 Boring Completed 08/16/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST				
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT
	0							
		VERY LOOSE Brown Slightly Silty Fine SAND (SP-SM)	1	1	1	2	2	3
		VERY LOOSE Dark Grey Slightly Silty Fine SAND (SP-SM)						
			2	2	3	3	2	6
		LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)						
	5		3	2	4	4	10	8
			4	6	8	10	10	18
		FIRM Brown Slightly Silty Fine SAND (SP-SM)	5	5	8	8		16
	10							
		LOOSE Grey Brown Slightly Silty Fine SAND (SP-SM)						
	15		6	4	4	4		8
		BORING TERMINATED						
	20							
	25							
	30							

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B4



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.0' Time: Drilling Date: 08/16/21
 Lat/Long N 30.62852° / W 81.46460°

JOB NO J19950
 BORING NO B5
 Sheet 1 of 1
 Boring Begun 08/16/21
 Boring Completed 08/16/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST					
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT	
	0								
		LOOSE Dark Grey Brown Slightly Silty Fine SAND (SP-SM)	1	1	2	3	1	5	
		LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)	2	2	2	4	6	6	
	5		3	5	7	11	15	18	
		FIRM to VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)	4	8	12	12	12	24	
			5	9	7	11		18	
	10	FIRM Brown Slightly Silty Fine SAND (SP-SM)							
			FIRM Grey Brown Slightly Silty Fine SAND (SP-SM)						
	15		6	5	7	8		15	
			BORING TERMINATED						
	20								
	25								
30									

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B5



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.0' Time: Drilling Date: 08/16/21
 Lat/Long N 30.62861° / W 81.46526°

JOB NO J19950
 BORING NO B6
 Sheet 1 of 1
 Boring Begun 08/16/21
 Boring Completed 08/16/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST				
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT
	0							
		VERY LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)	1	1	1	1	4	2
		LOOSE Brown Slightly Silty Fine SAND (SP-SM)	2	4	5	4	5	9
	5	FIRM Dark Brown Slightly Silty Fine SAND (SP-SM)	3	4	5	9	13	14
			4	8	9	12	12	21
		VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)	5	15	11	10	10	21
	10							
		LOOSE Grey Slightly Silty Fine SAND (SP-SM)						
	15		6	3	3	6		9
		BORING TERMINATED						
	20							
	25							
	30							

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B5



TEST BORING RECORD

Project Artisan Bailey Subdivision
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.0' Time: Drilling Date: 08/17/21
 Lat/Long N 30.62864° / W 81.46571°

JOB NO J19950
 BORING NO B7
 Sheet 1 of 1
 Boring Begun 08/17/21
 Boring Completed 08/17/21
 Driller AW & CB
 Soil Inspected By B. McMahan

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST				
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT
	0							
		LOOSE Dark Grey Brown Slightly Silty Fine SAND w/ Brick Debris (SP-SM)	1	1	1	4	4	5
		VERY LOOSE Brown Slightly Silty Fine SAND (SP-SM)	2	4	2	2	2	4
	5	LOOSE Dark Brown Slightly Silty Fine SAND w/ Tr Small Rts (SP-SM)	3	2	3	5	10	8
		FIRM Brown Slightly Silty Fine SAND (SP-SM)	4	5	10	10	10	20
		BORING TERMINATED						
	10							
	15							
	20							
	25							
	30							

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J19950 B7



AUGER BORING RECORDS
Artisan Bailey Subdivision
Fernandina Beach, Florida
AGES Job No. J19950; Report No. 001

Date Performed: 08/17/21

Performed By: J. Mardini

Auger ID Lat/Long	Depth (Feet) ^a	SOIL DESCRIPTION (USCS/ AASHTO CLASSIFICATION)
----------------------	---------------------------	--

Roadway

A1 N 30.62884° W 81.46592°	0.0 - 0.5	Dark Brown Silty Fine SAND w/ Many Roots (PT/ A-8)
	0.5 - 1.5	Dark Brown Slightly Silty Fine SAND w/ Some Rocks (SP-SM/ A-3)
	1.5 - 2.5	Grey Slightly Silty Fine SAND (SP-SM/ A-3)
	2.5 - 6.0	Dark Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	A.B.T. ^b	GWL ^c = 1.5'
A2 N 30.62882° W 81.46495°	0.0 - 0.5	Dark Brown Silty Fine SAND w/ Many Roots (PT/ A-8)
	0.5 - 1.0	Dark Grey Slightly Silty Fine SAND w/ Trace Small Roots (SP-SM/ A-3)
	1.0 - 1.5	Grey Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	1.5 - 2.0	Dark Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	2.0 - 6.0	Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	A.B.T. ^b	GWL ^c = 1.5'
A3 N 30.62909° W 81.46439°	0.0 - 1.0	Dark Grey Silty Fine SAND w/ Some Roots (SM/ A-2-4)
	1.0 - 1.5	Grey Slightly Silty Fine SAND w/ Trace Roots (SP-SM/ A-2-4)
	1.5 - 6.0	Dark Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	A.B.T. ^b	GWL ^c = 1.5'

Stormwater Management Pond

LA1 N 30.62918° W 81.46408°	0.0 - 1.0	Dark Grey Brown Silty Fine SAND w/ Some Roots (SM/ A-2-4)
	1.0 - 3.5	Dark Grey Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	3.5 - 7.0	Dark Brown Slightly Silty Fine SAND w/ Trace Organics (SP-SM/ A-3)
	7.0 - 13.0	Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	13.0 - 15.0	Grey Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	A.B.T. ^b	GWL ^c = 1.0'
		Estimated Seasonal High GWL ^c = 1.0'
LA2 N 30.62856° W 81.46420°	0.0 - 0.5	Brown Slightly Silty Fine SAND (SP-SM/ A-3/ A-3)
	0.5 - 1.0	Dark Grey Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	1.0 - 3.0	Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	3.0 - 5.0	Dark Brown Slightly Silty Fine SAND w/ Trace Small Roots (SP-SM/ A-3)
	5.0 - 14.0	Brown Slightly Silty Fine SAND w/ Trace Wood (SP-SM/ A-3)
	14.0 - 15.0	Grey Brown Slightly Silty Fine SAND (SP-SM/ A-3)
	A.B.T. ^b	GWL ^c = 1.0'
		Estimated Seasonal High GWL ^c = 1.0'

a - Depth measured below ground surface existing at boring location at time of drilling

b - Auger Boring Terminated (A.B.T.)

c - Groundwater Level (GWL) depth below existing ground surface, recorded at time of drilling, unless otherwise noted
J19950 aug 1.wpd

FIELD EXPLORATION PROCEDURES

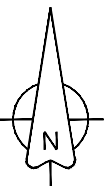
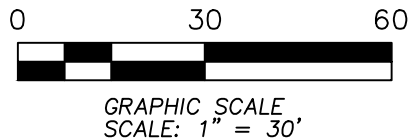
Penetration Borings

The penetration borings were made in general accordance with ASTM D 1586-67, "Penetration Test and Split-Barrel Sampling of Soils". The borings were advanced to the water table by augering and, after encountering the groundwater table, further advanced by rotary drilling techniques using a circulating bentonite fluid for borehole flushing and stability. At two-foot intervals within the upper 10 feet and at five-foot intervals thereafter, the drilling tools were removed from the borehole and a split-barrel sampler inserted to the borehole bottom and driven 18 inches into the material using a 140-pound SPT hammer falling, on the average, 30 inches per hammer blow. The number of hammer blows for the final 12 inches of penetration is termed the "penetration resistance, blow count, or N-value". This value is an index to several in-place geotechnical properties of the material tested, such as relative density and Young's Modulus.

After driving the sampler 18 inches (or less, if in hard rock or rock-like material) at each test interval, the sampler was retrieved from the borehole and a representative sample of the material within the split-barrel was placed in a glass jar and sealed. After completing the drilling operations, the samples for the borings were transported to our laboratory where they were examined by our engineer in order to verify the driller's field classifications. The samples will be kept in our laboratory for a period of two months after submittal of formal written report, unless otherwise directed by the client.

Auger Borings

The auger borings were performed manually using a post-hole auger. The auger borings were drilled in general accordance with ASTM D 1452-80 ("Soil Investigation and Sampling by Auger Borings"). Representative samples of the soils brought to the ground surface by the augering process were placed in glass jars, sealed, and transported to our laboratory where they were examined by a geotechnical engineer to verify the driller's field classification.



SURVEYOR'S NOTE:
THE "LESS AND EXCEPT" PARCELS DEEDED TO
NASSAU COUNTY IN OFFICIAL RECORDS BOOK 77,
PAGE 520, (AS TO PARCEL 2); OFFICIAL RECORDS
BOOK 78, PAGE 525, (AS TO PARCEL 3); AND
OFFICIAL RECORDS BOOK 79, PAGE 307, (AS TO
PARCEL 1), ARE NOT AVAILABLE ONLINE AT THE
NASSAU COUNTY CLERK OF THE CIRCUIT COURTS
OFFICE. THEREFORE WITHOUT A TITLE COMMITMENT,
AND A COPY OF THESE INSTRUMENTS, THIS FIRM CAN
NOT PLOT THESE "LESS AND EXCEPTS".

MAP SHOWING SURVEY OF BOUNDARY, TOPOGRAPHIC AND TREE SURVEY OF

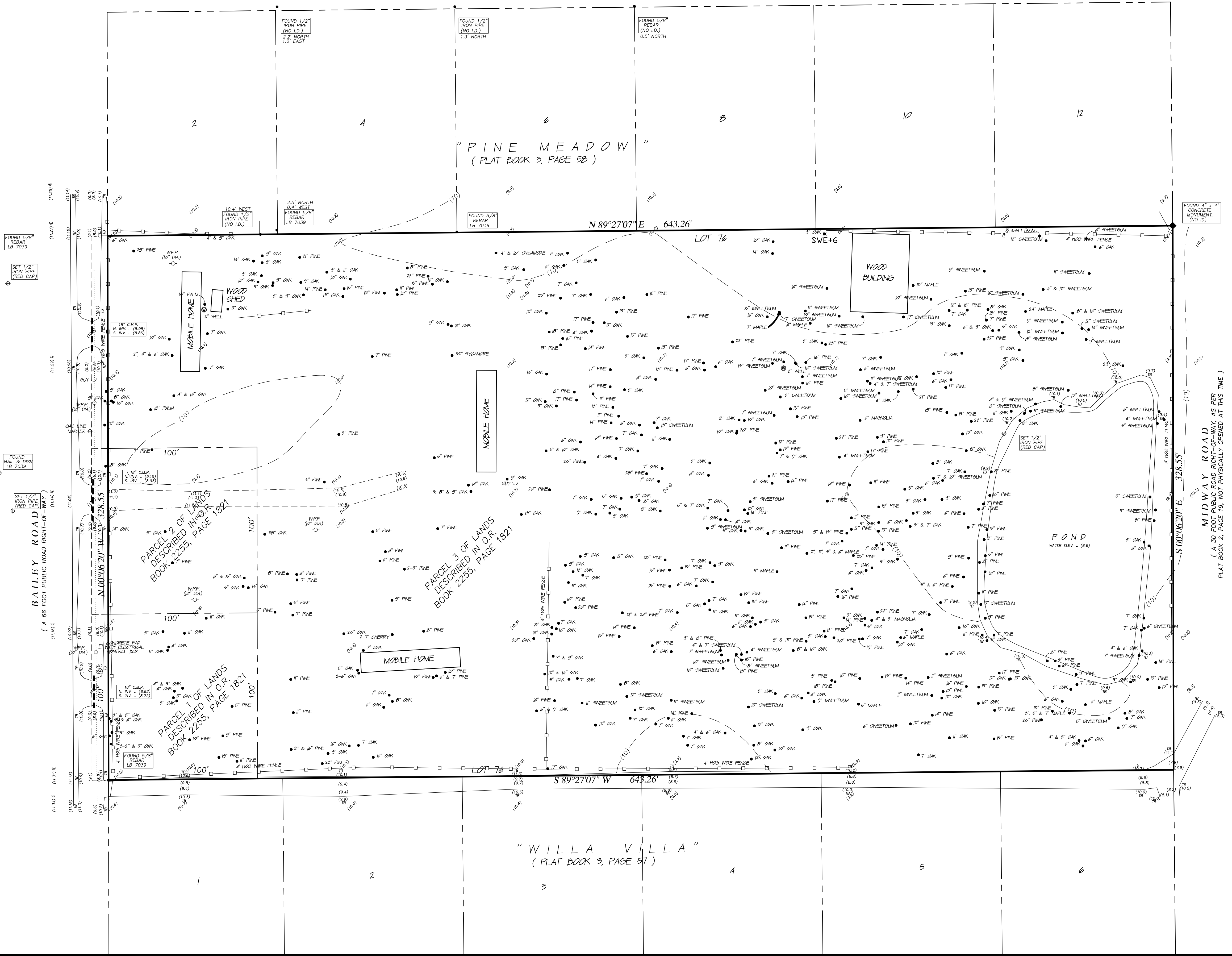
Legal description obtained from Quit-Claim Deed recorded in Official Records Book 2255, page 1821

All of Lot 76 of OCEAN BREEZE FARMS, a subdivision of a portion of Sections 3 and 5, Township 2 North, Range 28 East, Nassau County, Florida, according to plat of said Subdivision recorded in Plat Book 2, page 19, public records of said Nassau County, Florida, less the Westerly 100 feet of said Lot 76 which was taken to widen Bailey Road, and also except that portion thereof conveyed to Nassau County under O.R. Book 78, page 525, public records of Nassau County, Florida.

Surveyor's Note regarding the Legal Description contained in Official Records Book 2255, page 1821

The Legal Descriptions contained within this deed as Parcel 1 ad Parcel 2 are the same lands as described as a LESS AND EXCEPT in Parcel 3, ("and also less the Easterly 100 feet of the Westerly 110 feet of the Southerly 200 of said Lot 76).

- GENERAL NOTES:
- 1) BEARINGS SHOWN HEREON ARE BASED ON THE EASTERLY RIGHT-OF-WAY LINE OF "BAILEY ROAD", AS N 00°06'20" W, AS MONUMENTED AND ARE BASED ON THE U.S. DEPARTMENT OF COMMERCE, NATIONAL OCEANIC & ATMOSPHERIC ADMINISTRATION (NOAA), NATIONAL GEODETIC SURVEY (NGS) DATUM, NORTH AMERICA DATUM OF 1983 (2011), OR NAD83 (2011), FOR THE STATE OF FLORIDA, STATE PLANE COORDINATE SYSTEM, FOR ZONE 901(FL. EST.).
 - 2) THIS SURVEY IS PROTECTED BY COPYRIGHT AND IS CERTIFIED ONLY TO THE ENTITIES LISTED ON THIS SURVEY AND ONLY FOR THIS PARTICULAR TRANSACTION AND SCOPE OF WORK. ANY USE OF THIS SURVEY WITHOUT THE EXPRESS WRITTEN PERMISSION OF THIS SURVEYOR AND/OR FIRM IS STRICTLY PROHIBITED. USE OF THIS SURVEY IN ANY SUBSEQUENT TRANSACTIONS IS EXPRESSLY PROHIBITED AND IS NOT AUTHORIZED BY THIS SURVEYOR AND/OR FIRM. THIS SURVEYOR AND/OR FIRM EXPRESSLY DISCLAIMS ANY CERTIFICATION TO ANY PARTIES IN FUTURE TRANSACTIONS. NO ENTITY OTHER THAN THOSE LISTED ON THIS SURVEY SHOULD RELY UPON THIS SURVEY FOR ANY PURPOSE.
 - 3) UNLESS A TITLE COMMITMENT IS REFERENCED GRAPHICALLY ON THE FACE OF THIS SURVEY/SKETCH, THERE MAY BE ADDITIONAL COVENANTS AND RESTRICTIONS, EASEMENTS OF RECORD, BUILDING RESTRICTION/SETBACK LINES, RESTRICTIONS, AND OTHER MATTERS, EVIDENCED BY TITLE EXAMINATION BY A TITLE COMPANY, THAT HAVE NOT BEEN SHOWN HEREON. THESE ITEMS ARE NOT REQUIRED OR A PART OF A STATE OF FLORIDA MINIMUM TECHNICAL STANDARDS SURVEY, AS OUTLINED IN THE STATE OF FLORIDA, ADMINISTRATIVE CODE, 65-17.051.
 - 4) NOTE: NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS AND/OR DELETIONS TO SURVEY MAPS OR REPORTS BY OTHER THAN THE SIGNING PARTY OF PARTIES IS PROHIBITED WITHOUT THE WRITTEN CONSENT OF THE SIGNING PARTY OR PARTIES.
 - 5) NOTICE OF LIABILITY: THIS SURVEY IS CERTIFIED TO THOSE INDIVIDUALS, ENTITIES AND/OR FIRMS AS SHOWN ON THE FACE OF THIS SURVEY. ANY OTHER USE, BENEFIT OR RELIANCE BY ANY OTHER PARTY IS STRICTLY PROHIBITED AND RESTRICTED. THIS SURVEYING FIRM AND THE SIGNING SURVEYOR IS RESPONSIBLE ONLY TO THOSE THAT APPEAR IN THE CERTIFICATION AND HEREBY DISCLAIMS ANY OTHER LIABILITY AND HEREBY RESTRICTS THE RIGHTS OF OTHERS, (INDIVIDUAL OR ENTITIES) TO USE THIS SURVEY WITHOUT THE EXPRESS WRITTEN CONSENT OF THIS FIRM AND/OR SURVEYOR.
 - 6) ELEVATIONS SHOWN HEREON ARE BASED ON THE U.S. DEPARTMENT OF COMMERCE, NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION (NOAA), NATIONAL GEODETIC SURVEY (NGS) DATUM, NORTH AMERICAN VERTICAL DATUM OF 1988 (NAV88). THE NORTH AMERICAN VERTICAL DATUM OF 1988 IS THE CURRENT VERTICAL CONTROL DATUM OF ORTHOMETRIC HEIGHT ESTABLISHED FOR VERTICAL CONTROL SURVEYING IN THE UNITED STATES BASED UPON THE GENERAL ADJUSTMENT OF THE NORTH AMERICAN DATUM OF 1988, AND USES THE HELMERT ORTHOMETRIC HEIGHT, WHICH CALCULATES THE LOCATION OF THE GEOD (WHICH APPROXIMATES SEA LEVEL) FROM MODELED LOCAL GRAVITY.
 - 7) THIS DRAWING MAY HAVE BEEN ENLARGED OR REDUCED FROM THE ORIGINAL DRAWING. THEREFORE THE GRAPHIC SCALE SHOULD BE UTILIZED TO DETERMINE IF THIS MAP IS TO THE ORIGINAL SIZE AND SCALE.
 - 8) UNLESS OTHERWISE NOTED, ANY PORTION OF THE SUBJECT PROPERTY GRAPHICALLY SHOWN HEREON, MAY BE DEEMED WETLANDS, BY LOCAL, STATE OR FEDERAL GOVERNMENTAL AGENCIES. UNLESS AND ENVIRONMENTAL WETLAND JURISDICTIONAL DETERMINATION SURVEY WAS PERFORMED BY THIS FIRM, AT THIS TIME, THE DETERMINATION OF THE ENVIRONMENTAL WETLAND JURISDICTIONAL WETLAND LINE(S) AND APPROVAL BY THE APPOINTMENTED GOVERNMENTAL AGENCIES, IS THE RESPONSIBILITY OF THE OWNER(S), HIS OR HER AGENTS, AND ANY PARTY/PARTIES THAT UTILIZE THIS SURVEY WITHOUT OBTAINING AN ENVIRONMENTAL WETLAND JURISDICTIONAL LINE DETERMINATION AND APPROVAL BY THE APPROPRIATE GOVERNMENTAL AGENCIES DO SO AT THEIR OWN RISK.
 - 9) EVERY ATTEMPT WAS MADE BY THIS FIRM TO CORRECTLY IDENTIFY THE TREES SHOWN HEREON BY SIZE AND SPECIES TYPE, (I.E. HICKORY, LIVE OAK, ETC.). HOWEVER, THIS FIRM DOES NOT HAVE ON ITS STAFF A BOTANIST OR DENDROLOGIST, NOR DOES IT PRACTICE IN THE FIELDS OF BOTANY OR DENDROLOGY, OR REPRESENTS ITSELF AS AN EXPERT IN TREE IDENTIFICATION, WHICH IS NOT A SURVEYING FUNCTION. THEREFORE, THERE MAY BE MIS-IDENTIFIED TREES SHOWN ON THIS SURVEY.



JONATHAN B. BOWAN
STATE OF FLORIDA
REGISTERED LAND SURVEYOR
CERTIFICATE No. 4600

JOB No. 51884
FIELD BOOK 588, PAGE 39
CAD FILE NAME: ARTISAN\OCEAN BREEZE FARMS\51884.Dwg

FIELD DATE: MARCH 12, 2021
MAP DATE: APRIL 6, 2021
REVISED JUNE 23, 2021 (MAP), JUNE 21 (FIELD)
ADDED TREES 5" AND LARGER
JOB No. 53838- ADDED PINES

Prepared by
A & J Land Surveyors, Inc.
3847 Luella Street
Jacksonville, Florida 32207
T 904.346.1733
F 904.346.1736



Civil & Environmental Engineering • Mechanical & Structural Engineering • Construction Management

SITE DRAINAGE EVALUATION

**Island Circle Subdivision
Bailey Road
Fernandina Beach, Florida**

BY

**Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL**

Issue Date: March 1, 2022



Digitally signed by ASA R
GILLETTE
DN: cn=ASA R GILLETTE, c=US,
o=Florida,
email=asa@gilletteassociates.com
Date: 2022.03.02 14:01:38 -05'00'

EXISTING CONDITIONS

The site consists of 4.85 acres of land with frontage along Bailey Road in Fernandina Beach, Florida. The property currently drains to a wetland area just east of the property. The property is currently partially developed with mobile homes and a pond. A copy of the geotechnical report is included with this submittal. A curve number of 74 was chosen for the pre-development basin based on the soil types. In the pre-developed state, a time of concentration of 20 minutes was calculated based upon a length of overland flow of 550 feet at 1.0 fps.

PROPOSED CONDITIONS

The proposed development for this property includes the construction of 12 3,500 s.f. footprint homes accessed from a new 600-foot long road. The road and homes will be served by a s.f. wet pond that provides stormwater retention for the entire site. Stormwater runoff from the proposed improvements is routed to the pond through a series of storm water pipes and structures.

The pond has been designed with a top of bank elevation of 12.00' and a normal water level elevation of 9.00' based on geotechnical data provided in the geotechnical report (see attached). The western side of the pond is bulkheaded down to the bottom of the pond, and the rest of the pond banks slope at 3:1 from the top of bank to 1' below the normal water level and then at 2:1 from 1' below the normal water level to the bottom of the pond at elevation 4.00'.

Stormwater runoff from the proposed improvements is routed to the pond through a series storm water pipes and structures. The proposed outfall for the pond is a drop structure on the southeast side of the pond. The weir elevation is 10.1' in an effort to minimize the discharge to the boundary and meet the pre-vs-post condition for the project.

A composite curve number of 84.3 was used in the ICPR analysis based on a weighted average of the curve numbers for the impervious area, pervious area, and pond area. This calculation is shown in the pond spreadsheet within this report.

A Time of Concentration of 17 minutes was used in the ICPR analysis. The factors considered for this calculation are a Minimum Time of Concentration of 10 minutes, a maximum of 203 feet of overland flow at 1 foot per second, and 440 feet of pipe flow at 2.5 feet per second. The calculation is shown below. After rounding up to the nearest whole number, 17 minutes was determined as the ToC value for the analysis.

$$\text{ToC} = \text{Min. ToC} + [(\text{Length of Overland Flow}/1 \text{ fps}) + (\text{Length of Pipe flow}/2.5 \text{ fps})]/60 \text{ s} = 17 \text{ Min.}$$

The site was modeled with ICPR and the results are summarized on the following page. The pre-development discharge for the project is represented in the Hydrology Report by the "Flow Max cfs" value for the basin "pre-1", for the respective storm event. The post-development discharge is represented by the "Max Outflow cfs" value of the "pond-1" basin in the Node Report. A node reach diagram has been included in this report to illustrate how ICPR was modeled.

Summary of Design – Mean Annual Storm Event

Predevelopment Discharge (Total):	7.95 cfs
Post Development Discharge (Total):	7.94 cfs
Max Stage Elevation Pond:	10.76 ft
Top of Bank Elevation Pond:	12.00 ft

Summary of Design – 10 year Storm Event

Predevelopment Discharge (Total):	12.59 cfs
Post Development Discharge (Total):	9.99 cfs
Max Stage Elevation Pond:	11.26 ft
Top of Bank Elevation Pond:	12.00 ft

POND CALCULATIONS

Island Circle Post Developed Wet Pond

Basin Calculations

Basin 1

Composite Cn

Total Drainage Area =	4.85 Ac.	
Impervious Area =	1.69 Ac.	38% (not including pond area)
Pond Area =	0.36 Ac.	
Pervious Area =	2.80 Ac.	
Impervious Cn =	98.0	
Pond Cn =	100.0	
Pervious Cn =	74.0	
Composite Cn =	84.3	

Time of Concentration, Tc

Minimum Time of Concentration =	10 min.
Length of Overland Flow @ 1ft/sec. =	230 ft.
Length of Pipe Flow @ 2.5 ft/sec. =	440 ft.
Tc =	17 min.

Equivalent Runoff Coefficient, Ct

Soil Storage, S = 1000/Cn -10 =	1.86 in.
Precipitation Depth, Pt =	7.68 (Zone 5)

Ct = 1 - S/Pt * (1.2 - S/(Pt + .8S)) = 0.76

Pond Information

El. (ft.)		Area (Ac.)	Volume (Ac.-ft.)
12.00	TOB	0.50	2.67
11.00		0.45	2.19
10.10	Weir	0.41	1.80
10.00		0.41	1.76
9.00	NWL	0.36	1.38
8.00		0.32	1.04
7.00		0.29	0.74
6.00		0.26	0.47
5.00		0.23	0.22
4.00		0.21	0.00

Treatment Volume Required, TVR

TVR = 1" * (1/12")*Total Drainage Area =	0.40 Ac.-ft.
or	
TVR = 2.5" * (1/12")*Total Impervious Drainage Area =	0.35 Ac.-ft.
TVR =	0.40 Ac.-ft.

Treatment Volume Provided, TVP

TVP = Vol @ Weir - Vol @ NWL = 0.42 Ac.-ft. OK

Permanent Pool Volume Required, PPV

Drainage Area, Da =	4.85 Ac.	
Ct =	0.76	
Wet Season Rainfall Depth, R =	28 in.	
Residence Time, RT =	21 days	OK
Length of Wet Season, WS =	153 days	

PPV Required = Da*Ct*R*RT/(WS*12) = 1.18 Ac.-ft.

Permanent Pool Volume Provided, PPV

PPV Provided = Vol @ NWL = 1.38 Ac.-ft. OK

Pond Geometry Checks

Depth =	5.00 ft.	OK	
Mean Depth =	3.84 ft.	OK	OK
Average length =	300 ft.		
Average width =	80 ft.		
Ave Length to Width Ratio	3.8 :1	OK	

Sizing Bleed-Down Orifice for Residency Time

Formulas:

$$Q = .6 \cdot A (2 \cdot 32.2 \cdot h)^{1/2}$$

$$A = D^2 \cdot \pi / 4$$

$$t = TV / (2 \cdot Q \cdot 3600)$$

$$h = (h_1 + h_2) / 2$$

Where

Q = Rate of Discharge (cfs)

A = Orifice Area (ft²)

D = Orifice Diameter (ft)

π = 3.14159

h = Depth of Water Above the Flowline of the Orifice (ft)

t = Recovery Time (hrs)

TV = Treatment Volume Required (ft³)

h₁ = elevation @ TV - NWL Elevation

h₂ = elevation when 1/2 TV has been released - NWL Elevation

Treatment Volume (Ac.-ft.)	0.42	
Weir Elevation (ft.)	10.00	(minimum diameter for 6 in ² area is 2.8 inches)
NWL Elevation (ft.)	9.00	is value < 2.8 " in diameter? YES
Orifice Diameter (in.)	2	Orifice Diameter 2.8

Calculated Values:

h (ft)	0.79
A (ft ²)	0.02
Q (cfs)	0.09

t	27.46
---	-------

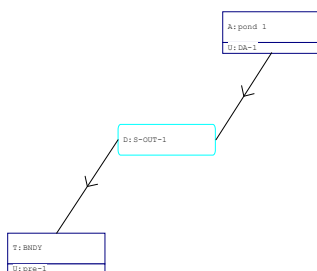
t Should be between 24 and 30 hrs.	TRUE	(minimum diameter for 6 in ² area is 2.8 inches)
------------------------------------	------	---

NODE DIAGRAM

Nodes
 A Stage/Area
 V Stage/Volume
 T Time/Stage
 M Manhole

 Basins
 O Overland Flow
 U SCS Unit CN
 S SBUH CN
 Y SCS Unit GA
 Z SBUH GA

 Links
 P Pipe
 W Weir
 C Channel
 D Drop Structure
 B Bridge
 R Rating Curve
 H Breach
 E Percolation
 F Filter
 X Exfil Trench



ICPR ANALYSIS
(MEAN ANNUAL AND 10-YEAR STORM EVENTS)

```
=====
==== Basins =====
=====
```

```

Name: DA-1                      Node: pond 1                      Status: Onsite
Group: BASE                     Type: SCS Unit Hydrograph CN

Unit Hydrograph: Uh484          Peaking Factor: 484.0
Rainfall File: Flmod           Storm Duration(hrs): 24.00
Rainfall Amount(in): 9.000     Time of Conc(min): 17.00
Area(ac): 4.850               Time Shift(hrs): 0.00
Curve Number: 84.30           Max Allowable Q(cfs): 999999.000
DCIA(%): 38.00
```

```
-----
Name: pre-1                      Node: BNDY                      Status: Onsite
Group: BASE                     Type: SCS Unit Hydrograph CN

Unit Hydrograph: Uh323          Peaking Factor: 323.0
Rainfall File: Flmod           Storm Duration(hrs): 24.00
Rainfall Amount(in): 9.000     Time of Conc(min): 20.00
Area(ac): 4.850               Time Shift(hrs): 0.00
Curve Number: 74.40           Max Allowable Q(cfs): 999999.000
DCIA(%): 1.90
```

```
=====
==== Nodes =====
=====
```

```

Name: BNDY                      Base Flow(cfs): 0.000          Init Stage(ft): 9.000
Group: BASE                     Warn Stage(ft): 10.000
Type: Time/Stage
```

Time(hrs)	Stage(ft)
0.00	9.000
12.00	9.500
24.00	10.000

```
-----
Name: pond 1                    Base Flow(cfs): 0.000          Init Stage(ft): 9.000
Group: BASE                     Warn Stage(ft): 12.000
Type: Stage/Area
```

Stage(ft)	Area(ac)
2.000	0.1100
3.000	0.1400
4.000	0.1700
5.000	0.2000
6.000	0.2400
7.000	0.2800
8.000	0.3200
9.000	0.3600
10.000	0.4100
11.000	0.4500
12.000	0.5000

```
=====
==== Drop Structures =====
=====
```

```

      Name: S-OUT-1          From Node: pond 1          Length(ft): 20.00
      Group: BASE           To Node: BNDY              Count: 1

      UPSTREAM      DOWNSTREAM      Friction Equation: Automatic
      Geometry: Circular      Circular      Solution Algorithm: Most Restrictive
      Span(in): 18.00      18.00      Flow: Both
      Rise(in): 18.00      18.00      Entrance Loss Coef: 0.000
      Invert(ft): 8.900      8.700      Exit Loss Coef: 1.000
      Manning's N: 0.013000      0.013000      Outlet Ctrl Spec: Use dc or tw
      Top Clip(in): 0.000      0.000      Inlet Ctrl Spec: Use dc
      Bot Clip(in): 0.000      0.000      Solution Incs: 10

```

Upstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

Downstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

*** Weir 1 of 2 for Drop Structure S-OUT-1 ***

TABLE

```

      Count: 1          Bottom Clip(in): 0.000
      Type: Horizontal      Top Clip(in): 0.000
      Flow: Both          Weir Disc Coef: 3.200
      Geometry: Rectangular      Orifice Disc Coef: 0.600

      Span(in): 120.00      Invert(ft): 10.100
      Rise(in): 12.00      Control Elev(ft): 10.100

```

*** Weir 2 of 2 for Drop Structure S-OUT-1 ***

TABLE

```

      Count: 1          Bottom Clip(in): 0.000
      Type: Vertical: Mavis      Top Clip(in): 0.000
      Flow: Both          Weir Disc Coef: 3.200
      Geometry: Circular      Orifice Disc Coef: 0.600

      Span(in): 2.80      Invert(ft): 9.000
      Rise(in): 2.80      Control Elev(ft): 9.000

```

```
=====
==== Hydrology Simulations =====
=====
```

Name: 10-year
Filename: X:\CIVIL\artisan\Bailey Road\Drainage\ICPR\10year.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Flmod
Rainfall Amount(in): 7.25

```

Time(hrs)      Print Inc(min)
-----
30.000      5.00

```

Name: mean
Filename: X:\CIVIL\artisan\Bailey Road\Drainage\ICPR\mean.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: FLMOD
Rainfall Amount(in): 5.40

```

Time(hrs)      Print Inc(min)
-----
30.000      5.00

```



```

=====
==== Routing Simulations =====
=====

```

Name: 10-year Hydrology Sim: 10-year
 Filename: X:\CIVIL\artisan\Bailey Road\Drainage\ICPR\10year.I32

Execute: Yes Restart: No Patch: No
 Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
 Time Step Optimizer: 0.000
 Start Time(hrs): 0.000 End Time(hrs): 30.00
 Min Calc Time(sec): 0.2500 Max Calc Time(sec): 300.0000
 Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
11.000	30.000
16.000	5.000
24.000	15.000
30.000	30.000

Group	Run
BASE	Yes

```

-----
Name: mean                      Hydrology Sim: mean
Filename: X:\CIVIL\artisan\Bailey Road\Drainage\ICPR\mean.I32

```

Execute: Yes Restart: No Patch: No
 Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
 Time Step Optimizer: 0.000
 Start Time(hrs): 0.000 End Time(hrs): 30.00
 Min Calc Time(sec): 0.2500 Max Calc Time(sec): 300.0000
 Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
11.000	30.000
16.000	5.000
24.000	15.000
30.000	30.000

Group	Run
BASE	Yes

Simulation	Basin	Group	Time Max hrs	Flow Max cfs	Volume in	Volume ft3
10-year	DR-1	BASE	12.05	21.495	6.064106751	619
10-year	pre-1	BASE	12.13	12.592	4.361	76768.976
mean	DR-1	BASE	12.05	15.288	4.282	75390.213
mean	pre-1	BASE	12.13	7.945	2.773	48825.725

Name	Group	Simulation	Max Time Stage hrs	Max Stage ft	Warning Stage ft	Max Delta Stage ft	Max Surf Area ft2	Max Time Inflow hrs	Max Inflow cfs	Max Time Outflow hrs	Max Outflow cfs
BNDY	BASE	10-year	24.06	10.000	10.000	0.0035	0	12.17	21.734	0.00	0.000
pond 1	BASE	10-year	12.39	11.260	12.000	0.0050	20167	12.08	21.348	12.39	9.999
BNDY	BASE	mean	24.01	10.000	10.000	0.0035	0	12.17	15.311	0.00	0.000
pond 1	BASE	mean	12.36	10.762	12.000	0.0050	19187	12.08	15.199	12.36	7.940

Name	Group	Simulation	Max Time Flow hrs	Max Flow cfs	Max Delta Q cfs	Max Time US Stage hrs	Max US Stage ft	Max Time DS Stage hrs	Max DS Stage ft
S-OUT-1	BASE	10-year	12.39	9.999	0.182	12.39	11.260	24.06	10.000
S-OUT-1	BASE	mean	12.36	7.940	0.144	12.36	10.762	24.01	10.000

STORMWATER PIPE CALCULATIONS

Gillette & Associates, Inc.

STORM SEWER TABULATION FORM

PROJ #:

PROJECT: Bailey Road Island Circle

COUNTY: Nassau

DATE: 03/01/22

BY: AG

SHEET 1 OF 1

MINOR LOSSES HAVE BEEN CONSIDERED

STRUCT. NO.	STRUCT. TYPE	LINE TYPE	LINE LENGTH (FT)	DRAINAGE AREA (ACRES)		SUBTOTAL CA	TIME OF CONC. (MIN)	TIME OF FLOW IN SECTION (MIN)	i (IN/HR)	TOTAL CA	TOTAL RUNOFF (CFS)	INLET ELEV. (FT)	H.G. ELEV.			DIA. (IN.)	SLOPE (%)	V (FPS)	CAPACITY (CFS)	NOTES & REMARKS
				CROWN ELEV.																
				INVERT ELEV.																
				UPPER END	LOWER END								FALL (FT.)							
				INCRMNT	SUBTOTAL															
S-1	C.B.	HDPE	23	0.18	0.18	0.16	10.00	0.13	7.01	0.16	1.14	11.80	11.79	11.78	0.01	18	0.01	0.6	1.1	
S-2													9.55	9.49	0.06					
													8.05	7.99	0.06		0.25	3.0	5.3	
S-2	C.B.	HDPE	231	0.24	0.42	0.38	10.13	1.28	6.98	0.38	2.64	11.80	11.78	11.59	0.19	18	0.06	1.5	2.6	
S-3													9.49	8.90	0.59					
													7.99	7.40	0.59		0.25	3.0	5.3	
S-3	C.B.	HDPE	262	1.21	1.90	2.13	11.41	1.46	6.69	2.13	14.24	11.70	11.59	11.38	0.21	36	0.05	2.0	14.2	
S-4													8.88	8.62	0.26					
													5.88	5.62	0.26		0.10	3.0	21.2	
S-4	C.B.	HDPE	21	0.74	2.64	2.80	12.87	0.12	6.39	2.80	17.88	11.40	11.38	11.22	0.16	36	0.07	2.5	17.9	
S-5													8.62	8.59	0.02					
													5.62	5.59	0.02		0.10	3.0	21.2	Tailwater 11.22
S-7	C.B.	HDPE	23	0.98	0.98	0.78	10.00	0.13	7.01	0.78	5.50	11.70	11.67	11.59	0.08	24	0.06	1.7	5.5	
S-3													8.92	8.88	0.04					
													6.92	6.88	0.04		0.17	3.0	9.4	

ENGINEERING PLANS

FOR

ISLAND CIRCLE SUBDIVISION

FOR

ARTISAN HOMES LLC

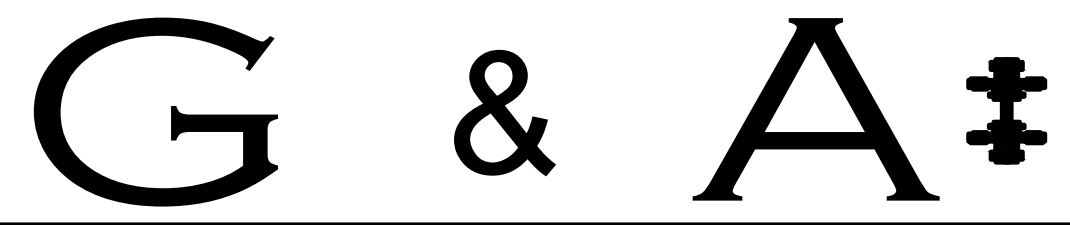
GILLETTE & ASSOCIATES, INC.

CIVIL & ENVIRONMENTAL ENGINEERING
MECHANICAL & STRUCTURAL ENGINEERING
PERMITTING & CONSTRUCTION MANAGEMENT

20 SOUTH 4TH STREET AMELIA ISLAND, FLORIDA 32034
PHONE: 904/261-8819

PERMITTING AGENCIES

CITY OF FERNANDINA BEACH
ST. JOHN'S RIVER WATER MANAGEMENT DISTRICT
FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION



ISSUE DATE: 03/01/22



ISLAND CIRCLE SUBDIVISION

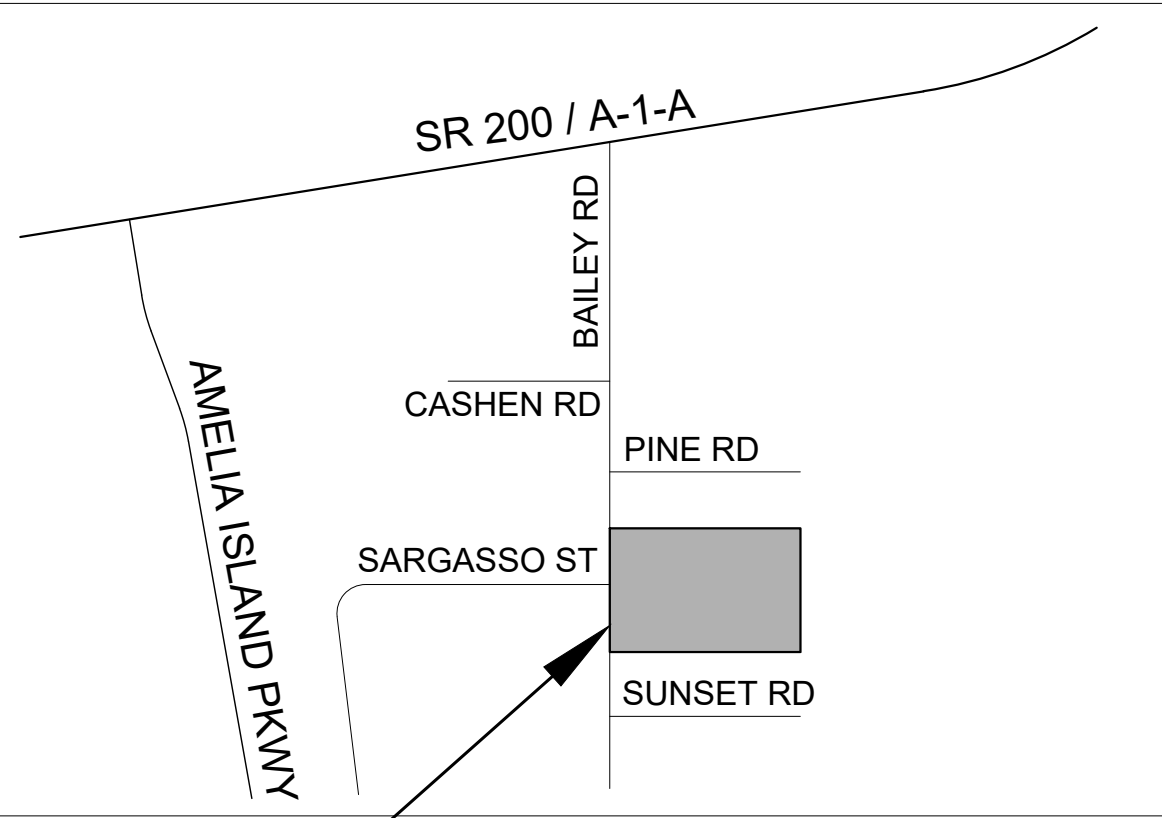
FOR

ARTISAN HOMES LLC

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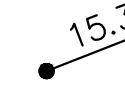
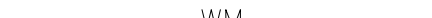


SITE LOCATION MAP

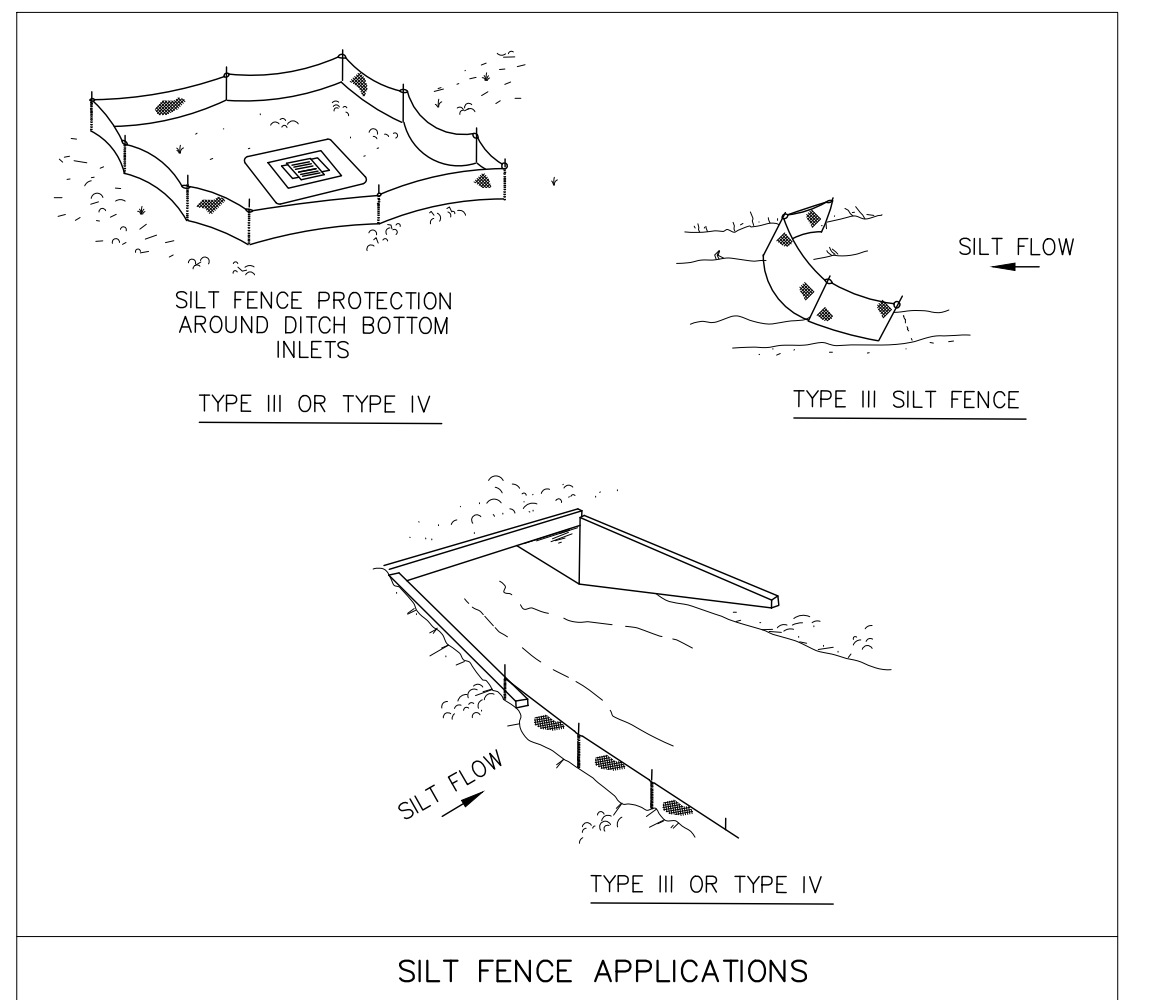
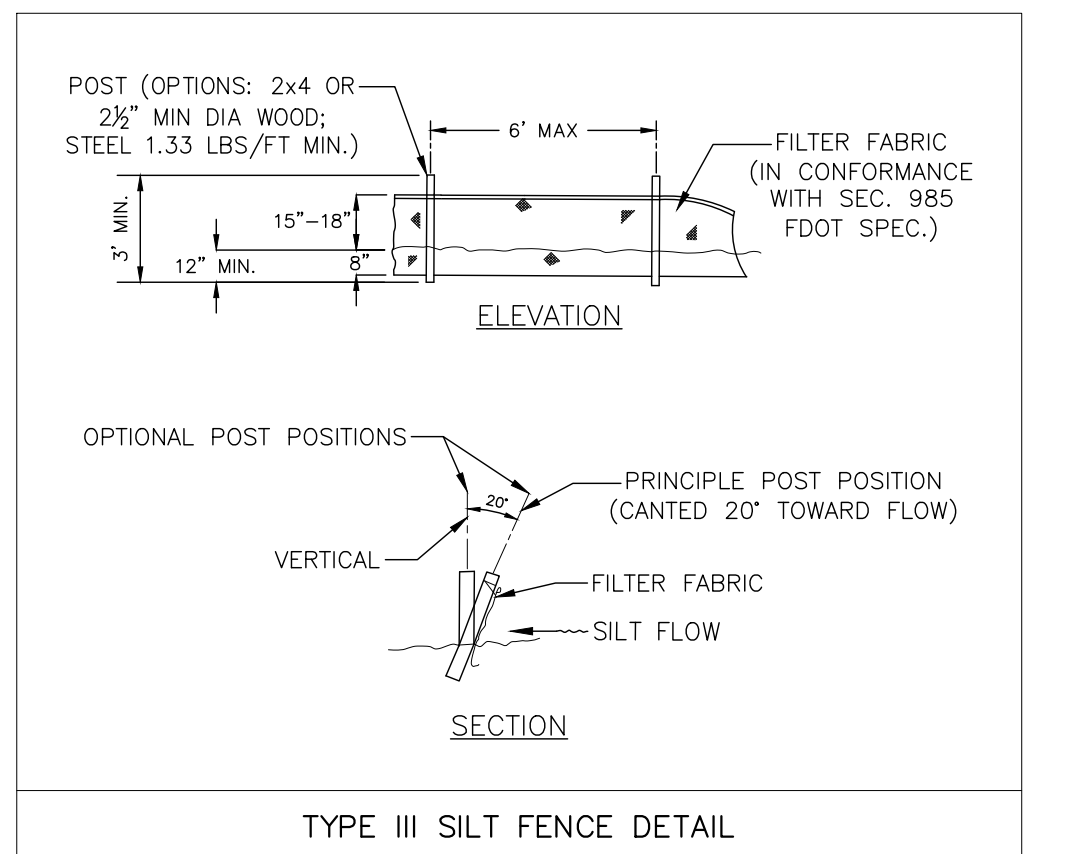


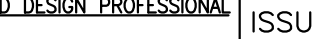
SITE

G&A Project
No.: 21-10-36

SITE / GEOMETRY NOTES	SITE / GEOMETRY NOTES CONT.	INDEX			
		ALL PERMITTING ENTITIES	PAGE #	SHEET	DESCRIPTION
			1	CVR	COVER SHEET
			2	IN-1	INDEX SHEET
			3	PRE-1	PREDEVELOPMENT PLAN
			4	GE-1	GEOMETRY PLAN
			5	UT-1	UTILITY PLAN
			6	DR-1	DRAINAGE PLAN
			7	PP-1	ROAD PLAN AND PROFILE
			8	LS-1	LANDSCAPE PLAN
			9	TM-1	TREE MITIGATION PLAN
			10	TM-2	TREE MITIGATION DATA
			11	TM-3	TREE MITIGATION DATA
12	DT-1		MISCELLANEOUS DETAILS		
13	DT-2	DRAINAGE DETAILS			
WATER NOTES	FIRE NOTES	1. ALL WATER & SEWER CONSTRUCTION SHALL BE ACCOMPLISHED BY AN UNDERGROUND UTILITY CONTRACTOR LICENSED UNDER THE PROVISIONS OF CHAPTER 489 FLORIDA STATUTES.			
		2. WHEN DEWATERING CAPACITY REQUIRES A PERMIT (CUP), IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN THE PERMIT THROUGH THE ST. JOHNS RIVER WATER MANAGEMENT DISTRICT.			
		3. WATER MAINS SHALL BE LAID AT LEAST 10 FEET HORIZONTALLY FROM ANY EXISTING OR PROPOSED SANITARY HAZARD. THE DISTANCE SHALL BE MEASURED EDGE TO EDGE. WHERE PARALLEL WATER AND SEWER MAINS HAVE LESS THAN 10 FOOT HORIZONTAL SEPARATION, 20 FOOT LENGTHS OF C900 (DR18) WATER QUALITY PIPE SHALL BE USED WITH THE JOINTS STAGGERED AT 10 FOOT INTERVALS. THE PIPES SHALL BE PLACED IN SEPARATE TRENCHES WITH THE WATER MAIN AT LEAST 18 INCHES ABOVE THE SEWER.			
		4. WATER MAINS CROSSING SANITARY HAZARDS SHALL BE LAID TO PROVIDE A MINIMUM VERTICAL DISTANCE OF 18 INCHES BETWEEN THE OUTSIDE OF THE WATER MAIN AND THE OUTSIDE OF THE SEWER MAIN. WHERE WATER AND SEWER MAINS CROSS WITH BETWEEN 18 INCHES AND 6 INCHES OF VERTICAL CLEARANCE, A 20 FOOT SECTION OF C900 (DR18) WATER QUALITY PIPE SHALL BE CENTERED ON THE POINT OF THE CROSSING. THE CONTRACTOR SHALL FIELD VERIFY THE VERTICAL SEPARATION. THE MINIMUM SEPARATION BETWEEN WATER AND SEWER PIPES IS TO BE 6 INCHES OUTSIDE DIAMETER TO OUTSIDE DIAMETER.			
		5. NO WATER MAIN PIPE SHALL PASS THROUGH OR COME IN CONTACT WITH ANY PART OF A SEWER MANHOLE.			
		6. TRACER WIRE SHALL BE PROVIDED ON ALL WATER MAINS AND FORCE MAINS. TRACER WIRE SHALL BE #10 UF SOLID COPPER WIRE, COATED, AND SUITABLE FOR BURIAL. COATING SHALL BE BLUE FOR WATER AND GREEN FOR SEWER MAIN CONNECTIONS. WIRE SHALL BE ATTACHED DIRECTLY TO TOP OF PIPE AND BROUGHT INTO EACH VALVE BOX AND COILED TO ALLOW FOR EASY ACCESS AND EXTENSION ABOVE FINISHED GRADE FOR ATTACHMENT OF LOCATING EQUIPMENT.			
		7. ALL PIPE LENGTHS ARE APPROXIMATE DIMENSIONS. ALL DRAINAGE STRUCTURES SHALL BE CONSTRUCTED TO CONFORM WITH TYPICAL SECTIONS AND DETAILS AS SHOWN ON THE MISCELLANEOUS DETAILS SHEET AND IN ACCORDANCE WITH THE SPECIFICATIONS.			
		8. ALL UNDERGROUND UTILITIES MUST BE INSTALLED PRIOR TO PREPARATION OF SUBGRADE FOR PAVEMENT.			
		LEGEND			
			EXISTING VALVE		PROPOSED SPOT ELEVATION
			EXISTING WATER LINE		PROPOSED WATER LINE
			EXISTING FIRE HYDRANT		PROPOSED FIRE HYDRANT
			EXISTING PROPERTY LINE		PROPOSED LOT LINE
	EXISTING EDGE OF PAVEMENT		PROPOSED EDGE OF PAVEMENT		
	PIPE		DRAINAGE STRUCTURE		

K				Scale: NONE	Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034	 GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819	ARTISAN HOMES LLC 2754 BAILEY ROAD FERNANDINA BEACH, FLORIDA	ISLAND CIRCLE SUBDIVISION	INDEX SHEET	 REGISTERED DESIGN PROFESSIONAL ASA R. GILLETTE, P.E. FLORIDA P.E. NO. 96177	ISSUE DATE: 03/01/22
J				Project Mgr: AG							
H				Designed by: AG/EC	Certificate of Authorization No. 9332						Page 2 of 13
F				Drawn by: EC/AG	DO NOT SCALE THIS DRAWING -- DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"						Project No. 21-10-36
E				QA/QC: NG							
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B											
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No.	By	Date		Revision							



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J				Project Mgr: AG						
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No.	By	Date		Revision						

SITE NOTES

PROPERTY PARCELS:
00-00-30-044B-0076-0010
00-00-30-044B-0076-0020
TOTAL LOT AREA: 4.85 AC
ZONING: R-1
FLOOD ZONE: X

LOTS
MINIMUM WIDTH: 75 FT
MINIMUM AREA: 0.25 AC
FRONT YARD SETBACK: 25 FT
CORNER LOT SECONDARY FRONT SETBACK: 15 FT
SIDE YARD SETBACK: 10% LOT WIDTH
REAR YARD SETBACK: 25 FT

LANDSCAPE BUFFERS
NORTH & SOUTH: NC RS-2
REQUIRED BUFFER: NONE

EAST: RS-1
REQUIRED BUFFER: NONE

WEST: RLM
REQUIRED BUFFER: NONE

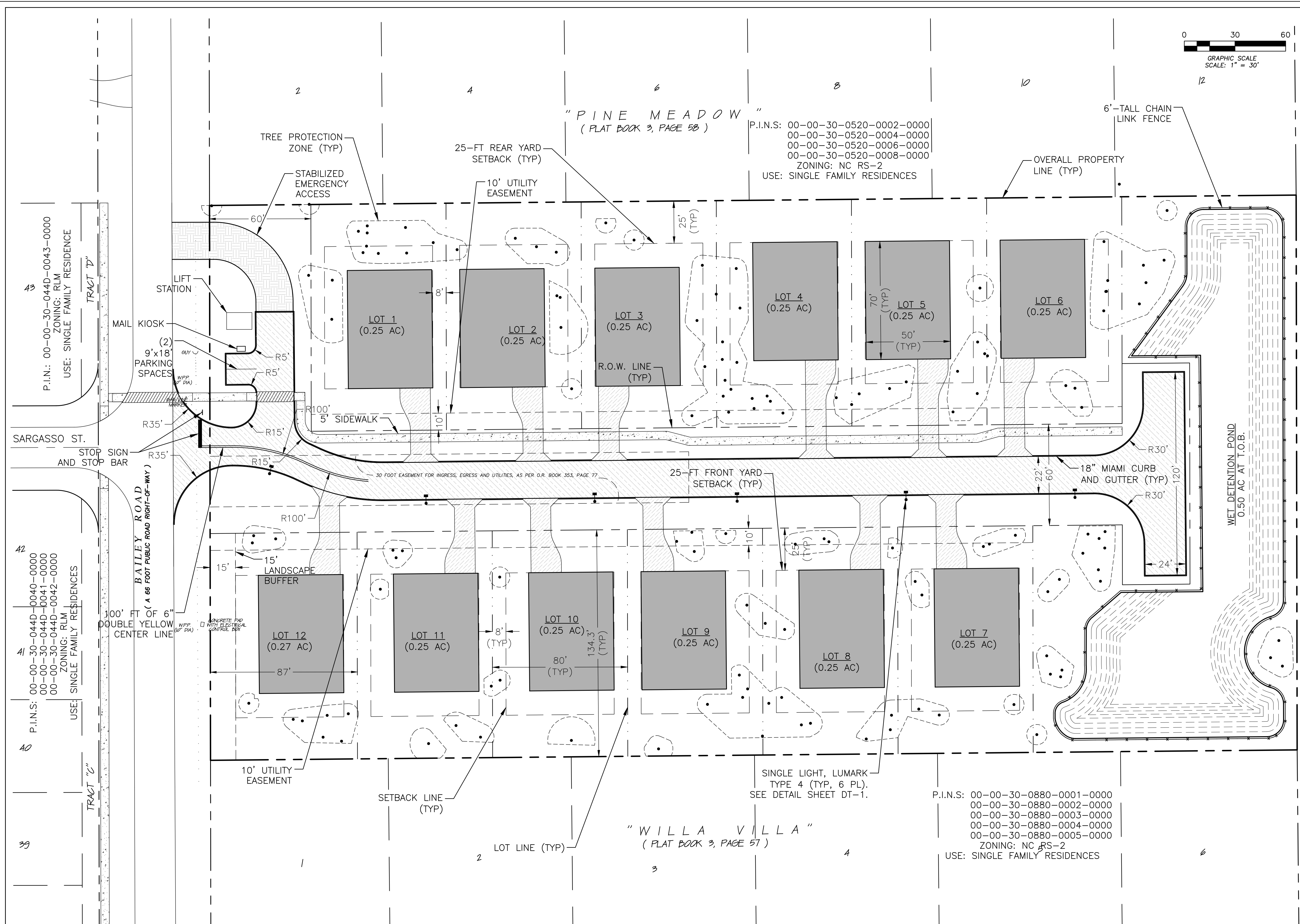
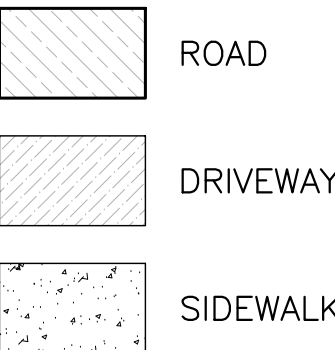
BUILDINGS
PROPOSED HOUSES: 3,500 SF
MAXIMUM FLOOR AREA RATIO: 50%
MAXIMUM PROPOSED FLOOR AREA RATIO: 33%
MAXIMUM BUILDING HEIGHT: 35 FT
PROPOSED BUILDING HEIGHT:

IMPERVIOUS AREA
MAXIMUM ISR FOR LOTS: 60%
MAXIMUM PROPOSED LOT ISR: 37%
BUILDING IMPERVIOUS AREA: 42,000 SF
PAVEMENT IMPERVIOUS AREA: 30,497 SF
POND AT N.W.L.: 15,665 SF
TOTAL IMPERVIOUS AREA: 88,162 SF
PROPOSED TOTAL ISR: 42%

UTILITIES
WATER: COFB
SEWER: COFB
ELECTRICITY: FLORIDA PUBLIC UTILITY
NO WELLS WITHIN 200' OF SITE

LOT TABLE					
LOT #	WIDTH (FT)	GROSS AREA (AC)	GROSS AREA (SF)	NET AREA (AC)	NET AREA (SF)
1	80	0.25	10,741	0.23	9,942
2	80	0.25	10,741	0.23	9,942
3	80	0.25	10,741	0.23	9,942
4	80	0.25	10,741	0.23	9,942
5	80	0.25	10,741	0.23	9,942
6	80	0.25	10,741	0.23	9,942
7	80	0.25	10,741	0.23	9,942
8	80	0.25	10,741	0.23	9,942
9	80	0.25	10,741	0.23	9,942
10	80	0.25	10,741	0.23	9,942
11	80	0.25	10,741	0.23	9,942
12	87	0.27	11,681	0.21	8,947

HATCH LEGEND



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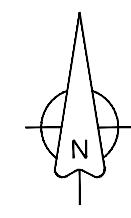
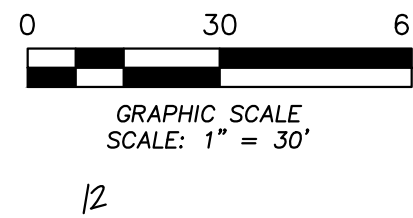
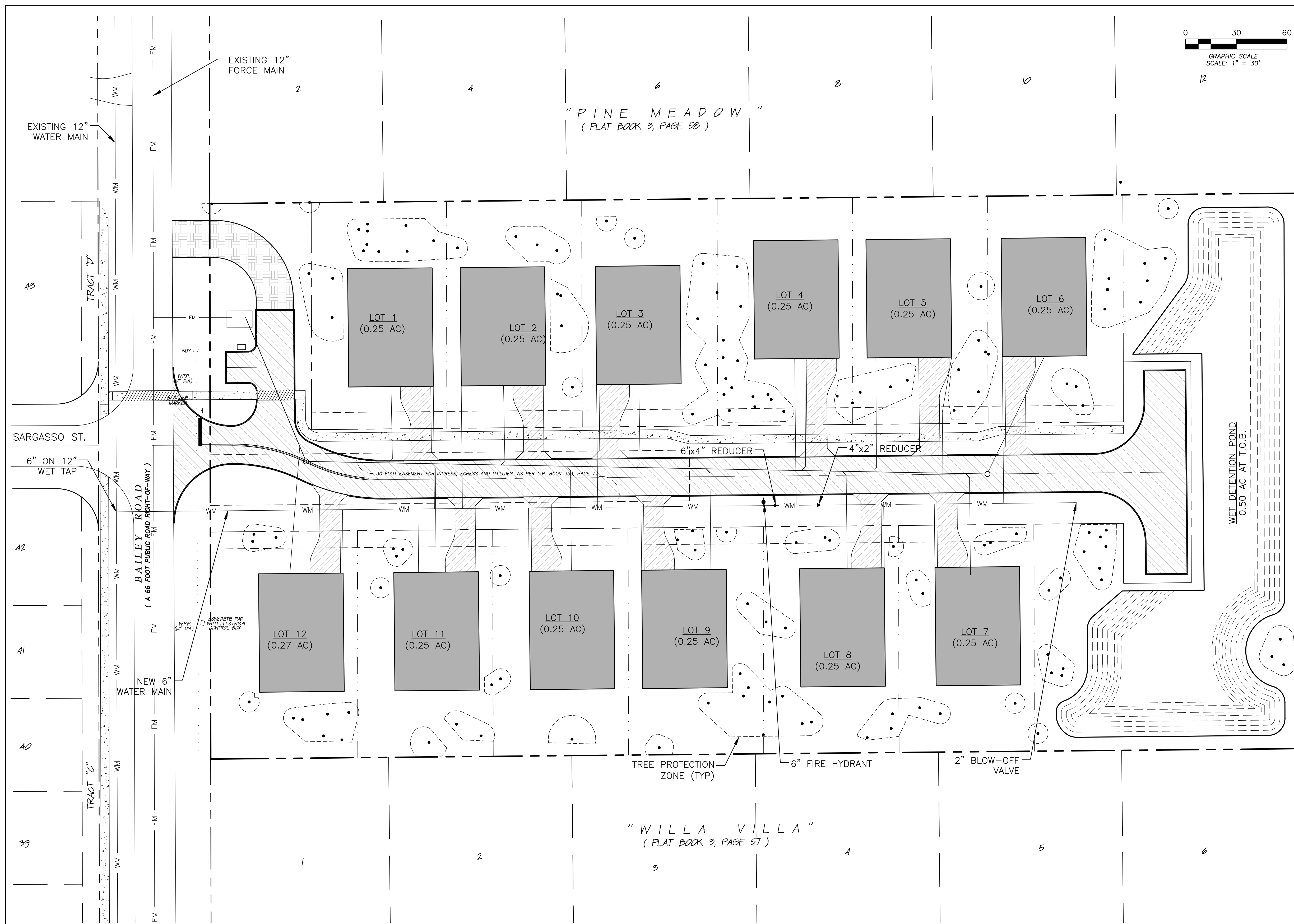
ARTISAN HOMES LLC
2754 BAILEY ROAD
FERNANDINA BEACH, FLORIDA

ISLAND CIRCLE SUBDIVISION

GEOMETRY PLAN

REGISTERED DESIGN PROFESSIONAL
ASA R. GILLETTE, P.E.
FLORIDA P.E. NO. 56177

ISSUE DATE: 03/01/22
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MIDWAY ROAD
(A 30 FOOT PUBLIC ROAD RIGHT-OF-WAY, AS PER THIS TIME)
PLAT BOOK 2, PAGE 19, NOT PHYSICALLY OPENED AT THIS TIME)

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ARTISAN HOMES LLC
2754 BAILEY ROAD
FERNANDINA BEACH, FLORIDA

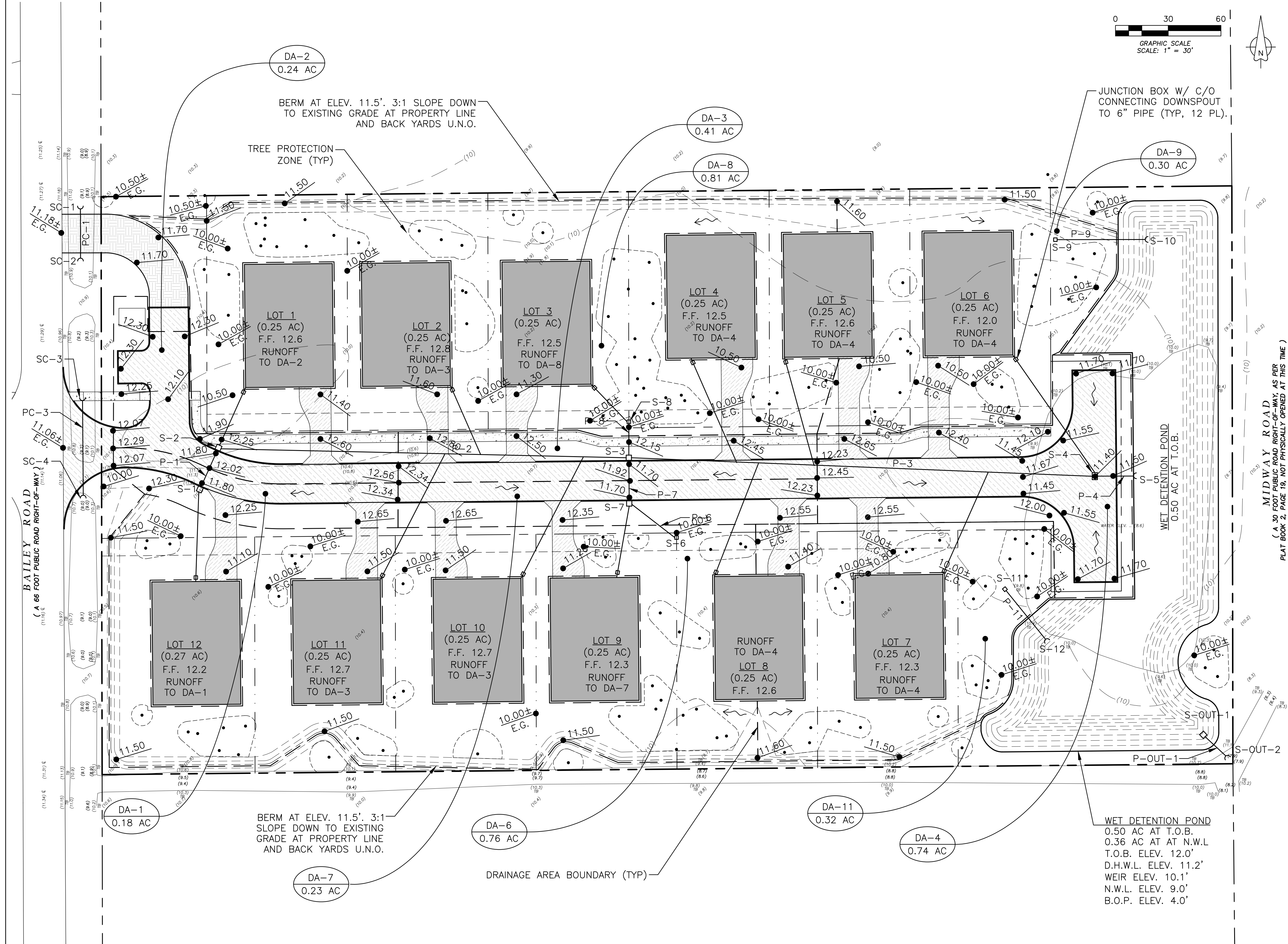
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UTILITY PLAN

REGISTERED DESIGN PROFESSIONAL

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FLORIDA P.E. NO. 56177

ISSUE DATE: 03/01/22
Sheet UT-1
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Project No. 21-10-36



NOTE: DRIVEWAYS TO BE CONCRETE AND CROWNED ON EACH LOT OUTSIDE OF RIGHT OF WAY. SEE DETAIL SHEET DT-2.

STRUCTURE TABLE						
NAME	TYPE	TOP/INLET ELEV.	INVERT ELEVATION			
			N	S	E	W
S-1	CURB INLET	11.80	8.05	7.94	—	—
S-2	CURB INLET	11.80	—	7.99	7.99	—
S-3	CURB INLET	11.70	7.45	6.88	5.88	7.40
S-4	CURB INLET	11.40	7.60	—	5.62	5.62
S-5	MES	—	—	—	—	5.59
S-6	YARD DRAIN	10.00	—	—	—	7.50
S-7	CURB INLET	11.70	6.92	8.05	7.42	—
S-8	YARD DRAIN	10.10	—	7.50	—	8.17
S-9	YARD DRAIN	10.10	—	—	7.50	—
S-10	MES	—	—	—	—	7.37
S-11	YARD DRAIN	10.10	—	7.50	—	—
S-12	MES	—	7.41	—	—	—
SC-1	MES	—	—	9.00	—	—
SC-2	MES	—	8.90	—	—	—
SC-3	MES	—	—	9.10	—	—
SC-4	MES	—	8.90	—	—	—
S-OUT-1	MOD. TYPE E INLET	—	—	—	—	—
S-OUT-2	MES	—	—	—	—	—

PIPE TABLE						
NAME	FROM	TO	SIZE	LENGTH	MATERIAL	
P-1	S-1	S-2	18"	23'	HDPE	
P-2	S-2	S-3	18"	231'	HDPE	
P-3	S-3	S-4	36"	262'	HDPE	
P-4	S-4	S-5	36"	21'	HDPE	
P-6	S-6	S-7	18"	31'	HDPE	
P-7	S-7	S-3	24"	23'	HDPE	
P-8	S-8	S-3	18"	19'	HDPE	
P-9	S-9	S-10	18"	50'	HDPE	
P-11	S-11	S-12	18"	35'	HDPE	
PC-1	SC-1	SC-2	18"	28'	RCP	
PC-3	SC-3	SC-4	18"	54'	RCP	
P-OUT-1	S-OUT-1	S-OUT-2	16"	—	HDPE	

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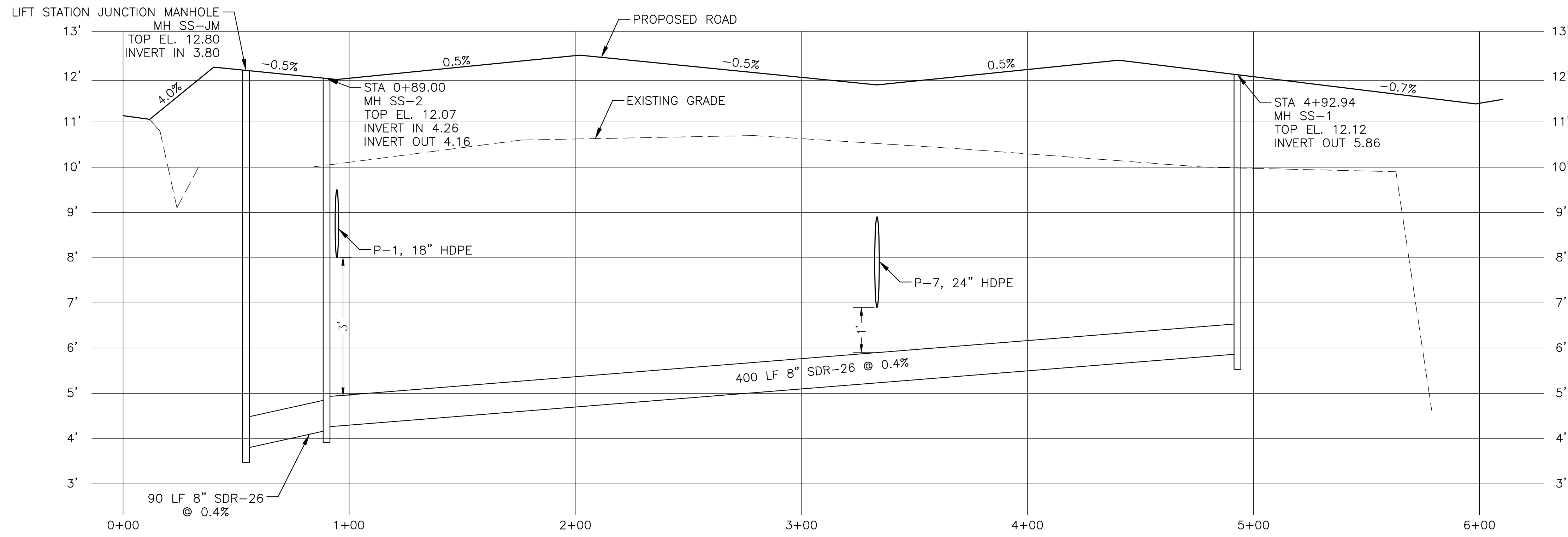
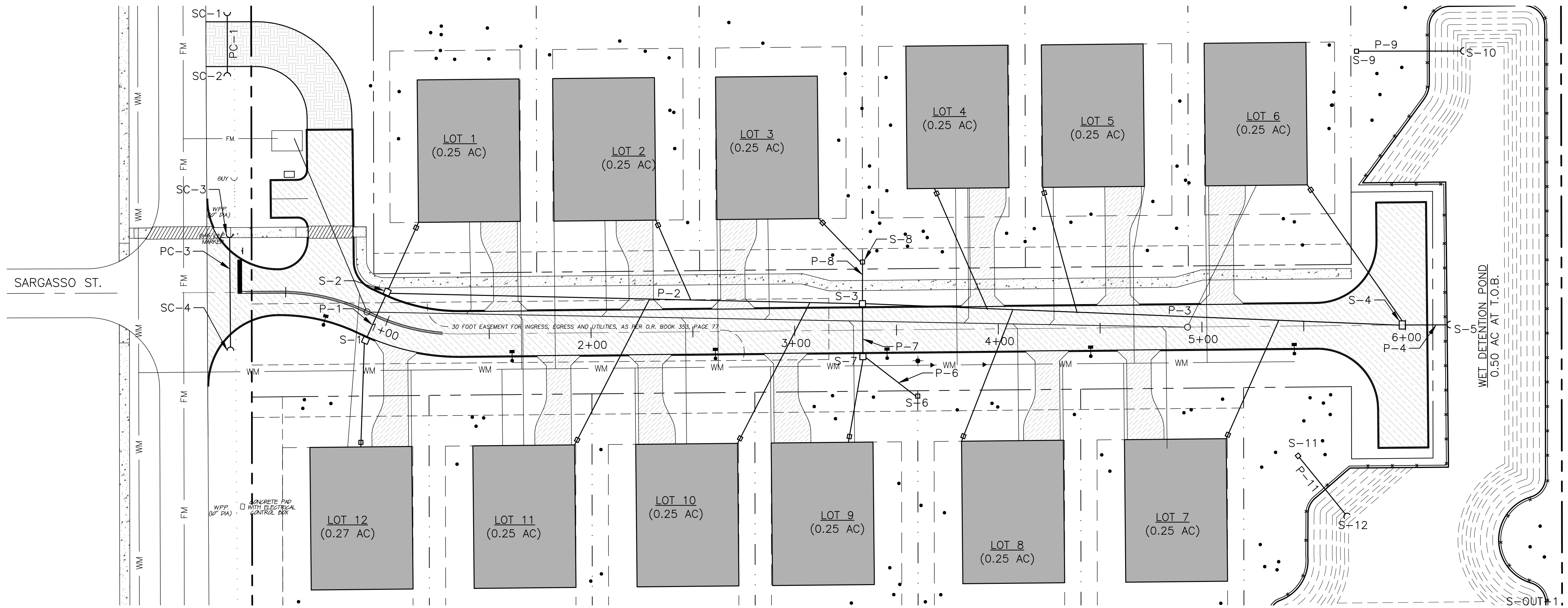
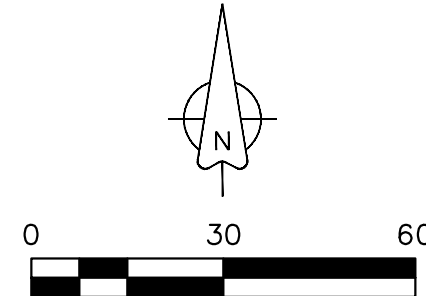
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FERNANDINA BEACH, FLORIDA

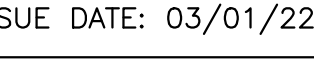
ISLAND CIRCLE SUBDIVISION

DRAINAGE PLAN

REGISTERED DESIGN PROFESSIONAL
ASA R. GILLETTE, P.E. FLORIDA P.E. NO. 56177

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J				Project Mgr: AG									Sheet
I				Designed by: AG/EC									PP-1
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A	EC	03/01/22		ISSUE FOR C.O.F.B. REVIEW									
No.	By	Date		Revision									

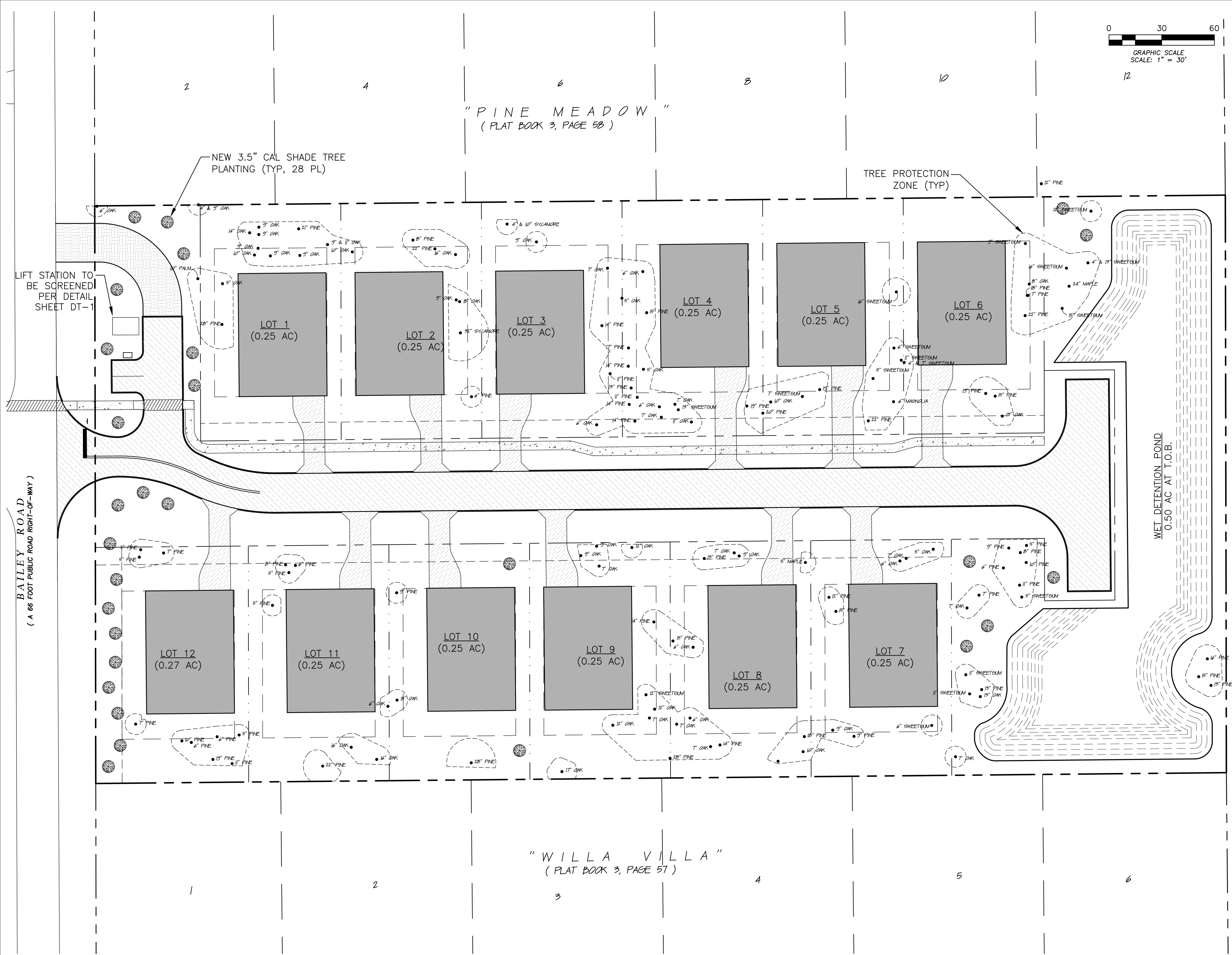
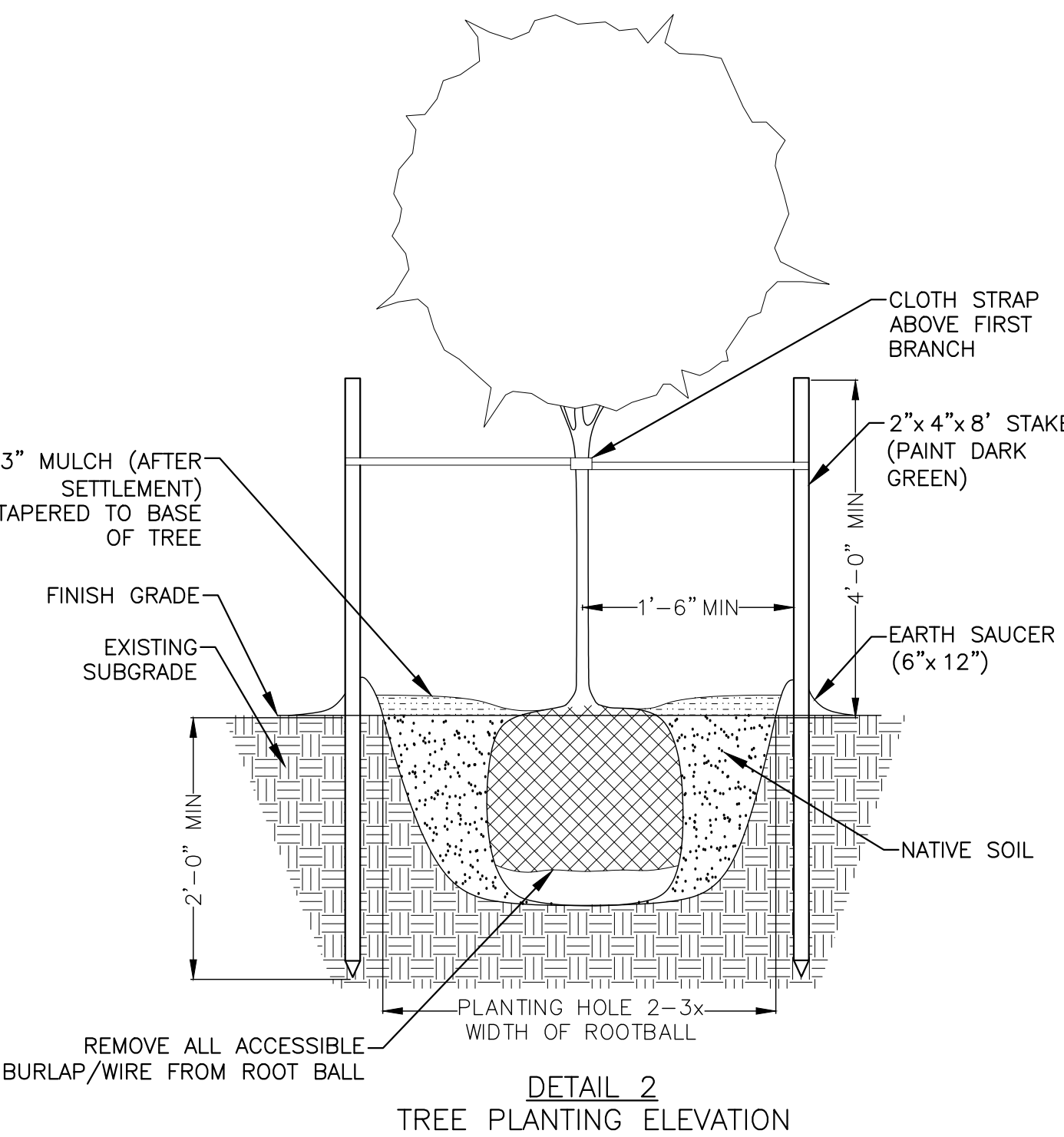
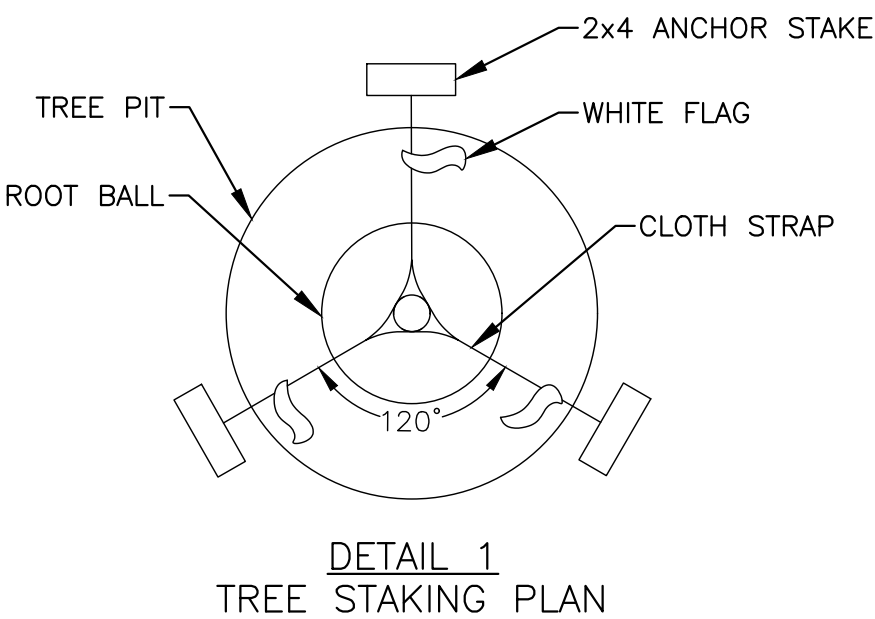
LANDSCAPE NOTES

BUFFERS
NORTH & SOUTH: NC RS-2
REQUIRED BUFFER: NONE

EAST: RS-1
REQUIRED BUFFER: NONE

WEST: RLM
REQUIRED BUFFER: NONE

SINGLE-FAMILY LOTS
REQUIRED TREES: 1 SHADE TREE/2,500 SF = 5 SHADE TREES
PROPOSED TREES: PRESERVE EXISTING TREES AND PLANT 2 NEW TREES



K J I H G F E D C B A		Scale: 1"=30'		Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034		G & A [®] GILLETTE & ASSOCIATES, INC. 20 SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819		ARTISAN HOMES LLC 2754 BAILEY ROAD FERNANDINA BEACH, FLORIDA		ISLAND CIRCLE SUBDIVISION		LANDSCAPE PLAN		REGISTERED DESIGN PROFESSIONAL AS R. GILLETTE, P.E. FLORIDA P.E. NO. 56177		ISSUE DATE: 03/01/22	
EC		03/01/22		ISSUE FOR C.O.F.B. REVIEW		Project Mgr: AG		Designed by: AG/EC		Drawn by: EC/AG		QA/QC: NG		Sheet LS-1		Page 8 of 13	
No.		By		Date		Revision		DO NOT SCALE THIS DRAWING - DIMENSIONS AND NOTES TAKE PRECEDENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"						Project No. 21-10-36			



EXISTING TREE LEGEND		
SYMBOL	DESCRIPTION	QTY.
X	TREE TO BE REMOVED	414
.	TREE TO REMAIN	151

PROPOSED MITIGATION	
PREDEVELOPED PROTECTED TOTAL (DBH):	3,503
REQUIRED PRESERVATION (50% DBH):	1,751.5
PRESERVED TOTAL (DBH):	1,654
DEFICIT (DBH):	97.5
PROPOSED PLANTINGS (3.5" SHADE TREES):	28
PROPOSED MITIGATION (DBH):	98

K			
J			
I			
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G			
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E			
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C			
B			
A	EC	03/01/22	ISSUE FOR C.O.F.B. REVIEW
No.	By	Date	Revision

Scale: 1"=30'
Project Mgr: AG
Designed by: AG/EC
Drawn by: EC/AG
QA/QC: NG

Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034
Certificate of Authorization No. 9332
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G & A⁺
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

ARTISAN HOMES LLC
2754 BAILEY ROAD
FERNANDINA BEACH, FLORIDA

ISLAND CIRCLE SUBDIVISION

TREE MITIGATION PLAN

REGISTERED DESIGN PROFESSIONAL

ASA R. GILLETTE, P.E.
FLORIDA P.E. NO. 56177

ISSUE DATE: 03/01/22
Sheet TM-1
Page 9 of 13
Project No. 21-10-36

TREE MITIGATION DATA						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
664	Pond P	29	4	0	N	0
665	Pond P	7	3	7	N	0
666	Pond P	7	2	7	Y	7
667	Pond P	5	4	0	N	0
668	Pond P	5	4	0	N	0
669	Pond P	7	3	7	Y	7
670	Pond P	14	1	14	N	0
671	Pond P	15	4	0	N	0
672	Pond P	21	1	21	Y	21
673	Pond P	18	4	0	N	0
674	Pond P	10	2	10	N	0
675	Pond P	11	2	11	N	0
676	Pond P	8	4	0	N	0
677	Pond P	22	1	22	Y	22
678	Pond P	8	1	8	Y	8
679	Pond P	7	2	7	N	0
680	Pond P	5	4	0	N	0
681	Pond P	8	1	8	Y	8
682	Pond P	6	3	6	N	0
683	Pond P	6	2	6	Y	6
684	Pond P	6	3	6	Y	6
685	Pond P	8	3	8	Y	8
686	Pond P	7	4	0	N	0
687	Pond P	9	1	9	Y	9
688	Pond P	5	4	0	N	0
688	Pond P	5	4	0	N	0
689	Pond P	5	1	5	N	0
690	Pond P	7	4	0	N	0
691	Pond P	5	1	5	Y	5
692	Pond P	8	1	8	N	0
693	Pond P	11	2	11	N	0
694	Pond P	11	2	11	N	0
695	Pond P	10	1	10	N	0
696	Pond P	6	4	0	N	0
696	Pond P	7	2	7	N	0
697	Pond P	10	3	10	N	0
698	Pond P	22	1	22	Y	22
699	Pond P	8	4	0	N	0
699	Pond P	16	4	0	N	0
700	Pond P	10	2	10	Y	10
701	Pond P	9	1	9	Y	9
702	Pond P	19	1	19	Y	19
703	Pond P	11	2	11	Y	11
704	Pond P	5	1	5	N	0
705	Pond P	28	4	0	N	0
706	Pond P	20	4	0	N	0
707	Pond P	20	1	20	N	0
708	Pond P	14	1	14	Y	14
709	Pond P	14	2	14	Y	14
710	Pond P	17	2	17	N	0
711	Pond P	12	2	12	N	0
712	Pond P	11	2	11	Y	11
713	Pond P	13	2	13	Y	13
714	Pond P	11	1	11	Y	11
715	Pond P	14	1	14	Y	14
716	Pond P	15	3	15	N	0
717	Pond P	15	2	15	N	0
718	Pond P	17	1	17	Y	17
719	Pond P	14	1	14	Y	14
720	Pond P	18	2	18	N	0
721	Pond P	17	4	0	N	0
722	Pond P	23	5	0	N	0
723	Pond P	13	4	0	N	0
724	Pond P	15	1	15	Y	15
725	Pond P	19	1	19	N	0
726	Pond P	15	2	15	N	0
727	Pond P	17	1	17	N	0
728	Pond P	17	4	0	N	0
729	Pond P	13	1	13	N	0
730	Pond P	22	4	0	N	0
731	Pond P	16	4	0	N	0

TREE MITIGATION DATA CONTINUED						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
732	Pond P	23	1	23	N	0
733	Pond P	16	2	16	N	0
734	Pond P	13	4	0	N	0
735	Pond P	20	1	20	Y	20
736	Pond P	19	2	19	Y	19
737	Pond P	12	4	0	N	0
738	Pond P	13	1	13	N	0
739	Pond P	22	1	22	Y	22
740	Pond P	21	4	0	N	0
741	Pond P	16	2	16	N	0
742	Pond P	11	4	0	N	0
743	Pond P	12	4	0	Y	0
744	Pond P	10	3	10	N	0
745	Pond P	15	4	0	N	0
746	Pond P	9	4	0	N	0
746	Pond P	12	4	0	N	0
747	Pond P	18	1	18	N	0
748	Pond P	13	1	13	Y	13
750	Pond P	13	1	13	N	0
751	Pond P	18	4	0	N	0
752	Pond P	15	1	15	Y	15
753	Pond P	14	4	0	N	0
754	Pond P	14	3	14	Y	14
755	Pond P	13	4	0	N	0
757	Pond P	21	4	0	N	0
757	Pond P	24	4	0	N	0
758	Pond P	20	1	20	N	0
759	Pond P	10	4	0	N	0
760	Pond P	16	1	16	N	0
762	Pond P	14	4	0	N	0
763	Pond P	11	2	11	N	0
764	Pond P	17	1	17	N	0
765	Pond P	19	4	0	N	0
766	Pond P	11	2	11	N	0
767	Pond P	12	1	12	Y	12
768	Pond P	13	4	0	N	0
768	Pond P	9	4	0	N	0
769	Pond P	15	2	15	Y	15
770	Pond P	23	4	0	N	0
771	Pond P	14	4	0	N	0
772	Pond P	16	4	0	N	0
773	Pond P	22	4	0	N	0
774	Pond P	14	1	14	N	0
775	Pond P	15	1	15	Y	15
776	Pond P	12	1	12	Y	12
777	Pond P	12	4	0	N	0
778	Pond P	20	1	20	N	0
779	Pond P	19	2	19	N	0
780	Pond P	9	4	0	N	0
780	Pond P	13	4	0	N	0
781	Pond P	15	1	15	N	0
782	Pond P	13	4	0	N	0
783	Pond P	9	4	0	N	0
784	Pond P	18	2	18	N	0
785	Pond P	14	4	0	N	0
786	Pond P	12	4	0	N	0
787	Pond P	14	4	0	N	0
788	Pond P	12	1	12	Y	12
789	Pond P	16	1	16	N	0
790	Pond P	17	4	0	N	0
791	Pond P	15	2	15	N	0
792	Pond P	15	1	15	N	0
793	Pond P	15	2	15	N	0
794	Pond P	13	4	0	N	0
795	Pond P	20	3	20	N	0
796	Pond P	7	3	7	Y	7
797	Pond P	9	1	9	N	0
798	Pond P	10	1	10	N	0
799	Pond P	5	1	5	N	0
800	Pond P	8	2	8	N	0
801	Pond P	7	2	7	N	0

TREE MITIGATION DATA CONTINUED						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
802	Pond P	11	1	11	N	0
803	Pond P	9	4	0	N	0
804	Pond P	11	1	11	Y	11
805	Pond P	5	4	0	N	0
805	Pond P	6	4	0	N	0
806	Pond P	6	1	6	Y	6
807	Pond P	10	1	10	Y	10
808	Pond P	8	3	8	Y	8
809	Pond P	8	1	8	Y	8
810	Pond P	5	1	5	Y	5
811	Pond P	9	1	9	Y	9
812	Pond P	7	1	7	Y	7
813	Pond P	8	1	8	N	0
814	Pond P	8	1	8	N	0
815	Pond P	7	4	0	N	0
816	Pond P	6	1	6	N	0
817	Pond P	6	4	0	N	0
818	Pond P	7	1	7	N	0
819	Pond P	10	1	10	N	0
820	Pond P	8	1	8	N	0
821	Pond P	9	4	0	N	0
822	Pond P	13	4	0	N	0
823	Pond P	17	1	17	N	0
824	Pond P	12	1	12	N	0
825	Pond P	21	4	0	N	0
826	Pond P	17	1	17	N	0
827	Pond P	24	1	24	Y	24
828	Pond P	15	3	15	Y	15
829	Pond P	15	1	15	Y	15
830	Pond P	13	3	13	Y	13
831	Pond P	16	2	16	Y	16
832	Pond P	22	4	0	N	0
833	Pond P	22	1	22	Y	22
834	Pond P	8	4	0	N	0
835	Pond P	19	2	19	N	0
836	Pond P	12	2	12	Y	12
837	Pond P	18	1	18	Y	18
838	Pond P	12	4	0	N	0
838	Pond P	15	4	0	N	0
839	Pond P	28	2	28	Y	28
840	Pond P	5	1	5	Y	5
841	Pond P	5	2	5	Y	5
842	Pond P	6	4	0	N	0
843	Pond P	7	1	7	Y	7
844	Pond P	6	1	6	Y	6
845	Pond P	6	1	6	Y	6
846	Pond P	5	1	5	Y	5
847	Pond P	6	4	0	N	0
848	Pond P	6	3	6	Y	6
849	Pond P	7	4	0	N	0
850	Pond P	8	1	8	N	0
851	Pond P	7	1	7	N	0
852	Pond P	6	4	0	N	0
853	Pond P	28	1	28	Y	28
854	Pond P	14	1	14	Y	14
855	Pond P	14	4	0	N	0
856	Pond P	10	4	0	N	0
857	Pond P	19	1	19	N	0
858	Pond P	9	5	0	N	0
859	Pond P	14	1	14	Y	14
860	Pond P	28	1	28	Y	28
861	Pond P	18	1	18	Y	18
862	Pond P	9	2	9	Y	9
863	Pond P	22	2	22	N	0
864	Pond P	24	4	0	N	0
865	Pond P	24	4	0	N	0
866	Pond P	16	4	0	N	0
867	Pond P	19	5	0	N	0
867	Pond P	16	5	0	N	0
868	Pond P	20	4	0	N	0
869	Pond P	15	4	0	N	0

TREE MITIGATION DATA CONTINUED						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
871	Pond P	22	2	22	N	0
872	Pond P	29	4	0	N	0
873	Pond P	19	2	19	Y	19
874	Pond P	22	3	22	N	0
876	CAM	48	5	0	N	0
877	MAG	6	4	0	N	0
877	MAG	5	4	0	N	0
877	MAG	4	4	0	N	0
6060	WO	8	4	0	N	0
6061	WO	10	4	0	N	0
6087	WO	14	4	0	N	0
6155	WO	4	4	0	N	0
6155	WO	9	2	9	Y	9
6164	SABAL	10	1	10	Y	10
6167	WO	4	5	0	N	0
6167	WO	14	2	14	N	0
6168	RED TIP	2	5	0	N	0
6168	RED TIP	4	5	0	N	0
6168	RED TIP	6	5	0	N	0
6169	WO	10	1	10	N	0
6170	SABAL	18	1	18	N	0
6190	CHY	11	4	0	N	0
6191	LAU	11	2	11	N	0
6192	CAM	14	4	0	N	0
6202	CAM	9	2	9	Y	9
6203	WO	14	2	14	Y	14
6204	CAM	9	4	0	N	0
6205	WO	9	4	0	N	0
6206	WO	10	4	0	N	0
6207	WO	9	4	0	N	0
6208	WO	5	4	0	N	0
6208	WO	9	4	0	N	0
6209	WO	9	3	9	Y	9
6209	WO	11	3	11	Y	11
6210	WO	10	2	10	Y	10
6214	WO	13	4	0	N	0
6226	WO	16	3	16	Y	16
6227	WO	16	2	16	Y	16
6228	WO	8	1	8	Y	8
6229	WO	9	4	0	N	0
6233	CAM	20	5	0	N	0
6234	CHY	7	4	0	N	0
6234	CHY	7	4	0	N	0
6236	LAU	38	4	0	N	0
6237	WO	14	4	0	N	0
6238	WO	3	4	0	N	0
6238	WO	8	4	0	N	0
6238	WO	9	4	0	N	0
6239	WO	9	2	9	N	0
6240	WO	13	3	13	N	0
6241	WO	5	5	0	N	0
6241	WO	10	3	10	N	0
6255	SYC	4	5	0	N	0
6255	SYC	10	1	10	Y	10
6256	WO	9	3	9	Y	9
6257	WO	16	3	16	Y	16
6258	WO	12	2	12	N	0
6259	SYC	32	1	32	Y	32
6260	WO	14	4	0	N	0
6261	WO	12	2	12	N	0
6273	WO	17	2	17	Y	17
6275	WO	9	4	0	N	0
6275	WO	16	3	16	N	0
6276	WO	12	5	0	N	0
6276	WO	14	5	0	N	0
6277	WO	20	4	0	N	0
6278	WO	8	3	8	N	0
6279	WO	8	1	8	N	0
6297	WO	11	1	11	Y	11
6298	SG	13	3	13	Y	13
6299	WO	9	1	9	N	0

TREE MITIGATION DATA CONTINUED						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
6386	SG	11	2	11	N	0
6387	SG	4	4	0	N	0
6387	SG	13	1	13	Y	13
6388	SG	16	1	16	Y	16
6389	MA	24	3	24	Y	24
6390	SG	15	1	15	Y	15
6391	WO	6	4	0	N	0
6391	WO	9	4	0	N	0
6392	WO	8	3	8	Y	8
6393	WO	9	5	0	N	0
6394	WO	9	4	0	N	0
6396	WO	11	1	11	N	0
6397	SG	12	2	12	N	0
6398	WO	8	4	0	N	0
6399	SG	4	4	0	N	0
6399	SG	9	3	9	N	0
6400	SG	13	2	13	N	0
6401	SG	8	1	8	N	0
6402	WO	29	2	29	N	0
6403	SG	9	1	9	N	0
6404	SG	14	2	14	N	0
6405	SG	12	1	12	N	0
6406	SG	12	1	12	N	0
6407	SG	9	2	9	N	0
6408	SG	8	4	0	N	0
6408	SG	10	3	10	N	0
6409	WO	13	3	13	Y	13
6410	WO	8	4	0	N	0
6481	WO	9	4	0	N	0
6482	WO	8	1	8	N	0
6491	WO	5	5	0	N	0
6492	LAU	5	5	0	N	0
6493	LAU	6	5	0	N	0
6493	LAU	8	5	0	N	0
6494	WO	5	2	5	N	0
6495	WO	3	2	3	N	0
6496	WO	5	1	5	N	0
6497	WAX M	4	5	0	N	0
6497	WAX M	5	5	0	N	0
6499	WO	5	3	5	N	0
6500	WO	5	1	5	N	0
6501	WO	7	2	7	N	0
6502	CAM	7	5	0	N	0
6503	WO	5	1	5	Y	5
6504	WO	7	4	0	N	0
6505	LAU	5	4	0	N	0
6506	LAU	6	4	0	N	0
6506	LAU	6	4	0	N	0
6507	WO	7	4	0	N	0
6508	WO	6	1	6	Y	6
6509	LAU	7	4	0	N	0
6510	WO	5	5	0	N	0
6511	WO	9	2	9	Y	9
6512	CHY	9	3	9	Y	9
6513	WO	8	2	8	Y	8
6514	WO	6	1	6	Y	6
6515	WO	3	4	0	N	0
6515	WO	6	4	0	N	0
6516	WO	5	4	0	N	0
6516	WO	6	4	0	N	0
6517	WO	5	4	0	N	0
6517	WO	5	4	0	N	0
6518	WO	6	4	0	N	0
6519	WO	2	4	0	N	0
6519	WO	2	4	0	N	0
6519	WO	5	4	0	N	0
6520	WO	7	2	7	N	0
6521	WO	7	3	7	N	0
6522	WO	8	4	0	N	0
6523	WO	6	3	6	Y	6
6524	WO	7	2	7	Y	7

TREE MITIGATION DATA CONTINUED						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
6525	WO	5	2	5	N	0
6526	WO	7	1	7	N	0
6527	WO	5	4	0	N	0
6527	WO	7	4	0	N	0
6529	WO	7	2	7	Y	7
6530	WO	7	1	7	N	0
6531	WO	7	1	7	N	0
6532	WO	6	4	0	N	0
6533	WO	7	2	7	Y	7
6534	WO	5	3	5	N	0
6535	WO	7	4	0	N	0
6536	WO	5	3	5	N	0
6539	CAM	7	5	0	N	0
6539	CAM	9	5	0	N	0
6541	WO	7	1	7	Y	7
6542	WO	7	2	7	Y	7
6543	WO	6	2	6	Y	6
6544	WO	7	4	0	N	0
6545	WO	7	1	7	Y	7
6546	SG	5	4	0	N	0
6547	WO	6	3	6	Y	6
6548	SG	7	5	0	N	0
6549	WO	6	2	6	N	0
6550	SG	4	4	0	N	0
6550	SG	7	2	7	N	0
6551	WO	6	4	0	N	0
6552	WO	6	4	0	N	0
6553	WO	5	4	0	N	0
6554	WO	5	4	0	N	0
6555	WO	5	1	5	N	0
6556	WO	5	1	5	N	0
6557	WO	7	2	7	N	0
6558	WO	6	3	6	N	0
6559	WO	6	4	0	N	0
6560	WO	5	4	0	N	0
6561	WO	5	3	5	N	0
6562	MA	5	3	5	Y	5
6563	SG	5	3	5	Y	5
6564	SG	5	3	5	N	0
6565	WO	6	4	0	N	0
6566	SG	7	3	7	N	0
6567	SG	7	1	7	Y	7
6568	MA	6	1	6	Y	6
6569	SG	7	2	7	N	0
6570	SG	5	1	5	N	0
6571	SG	5	1	5	N	0
6572	WO	4	4	0	N	0
6572	SG	7	3	7	Y	7
6573	SG	6	3	6	N	0
6574	WO	6	3	6	N	0
6577	WO	5	4	0	N	0
6578	WO	6	3	6	Y	6
6579	WO	7	1	7	Y	7
6580	WO	7	1	7	N	0
6581	WO	6	2	6	N	0
6582	WO	6	2	6	Y	6
6583	WO	5	3	5	N	0
6584	WO	6	4	0	N	0
6585	WO	5	3	5	Y	5
6586	WO	7	1	7	Y	7
6587	CHY	7	4	0	N	0
6588	WO	7	4	0	N	0
6589	WO	6	4	0	N	0
6590	WO	5	4	0	N	0
6591	WO	6	2	6	Y	6
6592	WO	5	2	5	N	0
6593	WO	6	2	6	N	0
6594	WO	5	3	5	Y	5
6595	WO	7	1	7	Y	7
6596	WO	6	4	0	N	0
6597	WO	6	4	0	N	0

TREE MITIGATION DATA CONTINUED						
TREE ID	SPECIES	DBH	HEALTH STATUS	PROTECTED DBH	RETAINED? (Y/N)	RETAINED DBH
6598	SG	5	3	5	N	0
6599	SG	7	1	7	Y	7
6601	WO	5	4	0	N	0
6602	SG	6	1	6	Y	6
6603	SG	6	4	0	N	0
6604	WO	7	3	7	N	0
6605	WO	6	2	6	Y	6
6606	WO	7	2	7	Y	7
6607	WO	7	1	7	N	0
6608	WO	5	3	5	N	0
6609	WO	5	4	0	N	0
6610	WO	7	4	0	N	0
6611	WO	5	4	0	N	0
6612	MA	5	1	5	Y	5
6613	WO	6	4	0	N	0
6614	WO	5	2	5	N	0
6615	WO	6	4	0	N	0
6616	SG	6	1	6	Y	6
6617	WO	7	2	7	Y	7
6618	SG	6	1	6	N	0
6619	WO	4	4	0	N	0
6619	WO	5	4	0	N	0
6620	WO	7	4	0	N	0
6621	WO	6	1	6	N	0
6622	WO	7	1	7	Y	7
6623	WO	5	2	5	Y	5
6624	WO	7	3	7	N	0
6625	WO	5	2	5	N	0
6626	WO	6	4	0	N	0
6627	WO	5	4	0	N	0
6628	WO	5	4	0	N	0
6629	WO	6	4	0	N	0
6630	WO	7	4	0	N	0
6631	WO	7	2	7	N	0
6632	WO	5	1	5	N	0
6633	SG	5	1	5	Y	5
6634	WO	5	1	5	N	0
6635	MA	6	1	6	N	0
6636	MA	6	1	6	N	0
6637	WO	5	4	0	N	0
6638	SG	5	1	5	N	0
6640	WO	6	2	6	N	0
6641	WO	5	4	0	N	0
6641	WO	5	4	0	N	0
6642	WO	6	1	6	N	0
6643	MA	5	4	0	N	0
6643	MA	6	4	0	N	0
6643	MA	7	4	0	N	0
6644	SG	7	1	7	N	0
6645	WO	4	4	0	N	0
6645	WO	6	4	0	N	0
6646	SG	6	1	6	N	0
6647	WO	7	4	0	N	0
6648	WO	7	4	0	N	0
6649	WAX M	6	5	0	N	0
6650	WO	5	4	0	N	0
6651	SG	5	1	5	N	0
6652	SG	5	1	5	N	0
6653	SG	6	1	6	N	0
6654	MA	5	4	0	N	0
6655	SG	6	1	6	N	0
6656	SG	5	4	0	N	0
6657	WO	5	4	0	N	0
6658	CHY	7	2	7	N	0
6659	WO	7	1	7	N	0
6661	SG	6	1	6	N	0
6662	MA	6	3	6	N	0
6663	WO	6	4	0	N	0
PREDEVELOPED PROTECTED TOTAL:				3503		
PRESERVED TOTAL:					1654	
REQUIRED PRESERVATION (50%):				1751.5		
DEFICIT:				97.5		

HEALTH STATUS CODES		
STATUS CODE	MITIGABLE? (Y/N)	HEALTH STATUS
1	Y	NO APPARENT DEFECT
2	Y	MINOR DEFECT
3	Y	COULD KEEP TREE BUT HAS DEFECTS
4	N	POOR HEALTH AND SUGGEST REMOVAL
5	N	SEVERE DEFECT AND SHOULD BE REMOVED

EXISTING TREE LEGEND		
SYMBOL	DESCRIPTION	QTY.
LAU	LAUREL OAK	9
WO	WATER OAK	221
MA	MAPLE	17
MAG	MAGNOLIA	3
SG	SWEETGUM	72
Pond P	POND PINE	217
CHY	CHERRY	7
SABAL	SABAL PALM	2
CAM	CAMPHOR	8
SYC	SYCAMOR	3
RED TIP	RED TIP SHRUB	3
WAX M	WAX MYRTLE	3
X	TREE TO BE REMOVED	415
•	TREE TO REMAIN	150

PROPOSED MITIGATION	
PREDEVELOPED PROTECTED TOTAL (DBH):	3,503
REQUIRED PRESERVATION (50% DBH):	1,751.5
PRESERVED TOTAL (DBH):	1,654
DEFICIT (DBH):	97.5
PROPOSED PLANTINGS (3.5" SHADE TREES):	28
PROPOSED MITIGATION (DBH):	98

K			
J			
I			
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E			
D			
C			
B			
A	EC	03/01/22	ISSUE FOR C.O.F.B. REVIEW
No.	By	Date	Revision

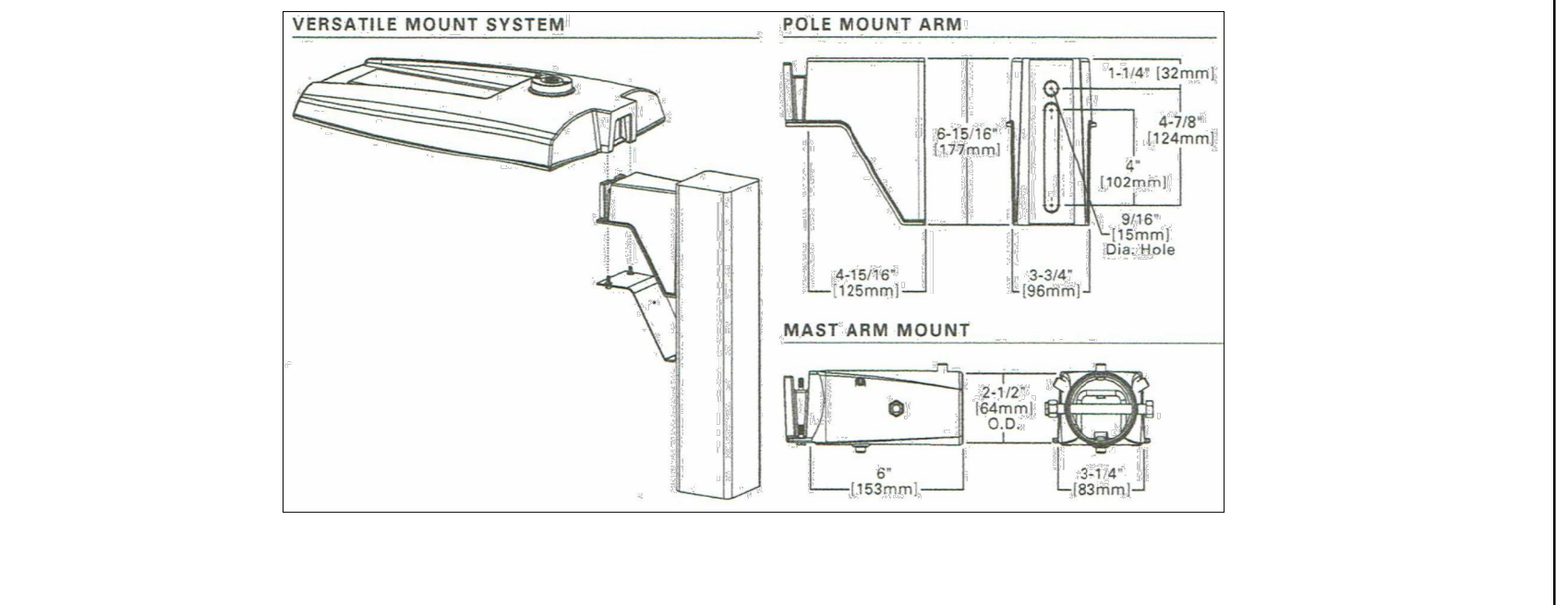
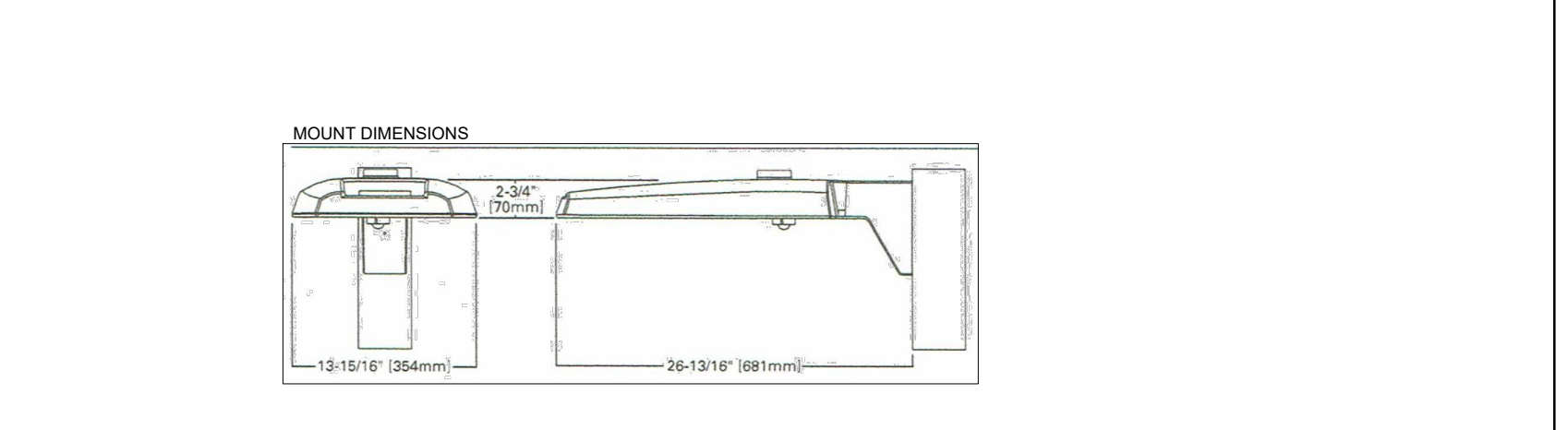
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Project Mgr: AG
Designed by: AG/EC
Drawn by: EC/AG
QA/QC: NG

Gillette & Associates, Inc. 20 South 4th Street Fernandina Beach, FL 32034
Certificate of Authorization No. 9332
DO NOT SCALE THIS DRAWING – DIMENSIONS AND NOTES TAKE PREFERENCE. DRAWING IS REDUCED IF LESS THAN 22" x 34"

NOT FOR CONSTRUCTION

The diagram is a site plan showing a rectangular area labeled "LIFT STATION PAD" with an arrow pointing to it. Above the pad is a row of circles representing shrubs, with an arrow pointing to them from the text "CHEESEWOOD SHRUB (TYP ~18 PL) SCREENING HEDGE SHRUBS TO BE AT LEAST 30" TALL AT INSTALLATION AND PLANTED AT 30" O.C.". Above the shrubs is a line labeled "BAILEY ROAD R.O.W." with an arrow pointing to it.

LIFT STATION SCREENING



The diagram shows a vertical light pole with a 'DIRECT BURY' label. To the right of the pole, there is a list of 'POLE NOTES' numbered 1 through 3.

POLE NOTES:

1. PROVIDE POLE/LT FIXTURES WITH ALL REQUIRED HARDWARE FOR MTG.
2. PROVIDE/INSTALL CONDUIT, AND PULLSTRING, JEA SHALL PROVIDE/INSTALL CONDUCTORS, POLE, MTG ARM, AND LIGHT FIXTURE. INSTALL ALL PER UTILITY CO REQUIREMENTS.
3. PROVIDE AND INSTALL CONCRETE LIGHT POLE PER MANUFACTURERS RECOMMENDATIONS TO WITHSTAND LOCAL EPA WIND SPEED RATINGS FOR AREA OF INSTALLATION (130MPH MINIMUM).

-

SITE LIGHTING DETAIL			

		REGISTERED DESIGN PROFESSIONAL	ISSUE DATE: 03/01/22
			Sheet DT 1

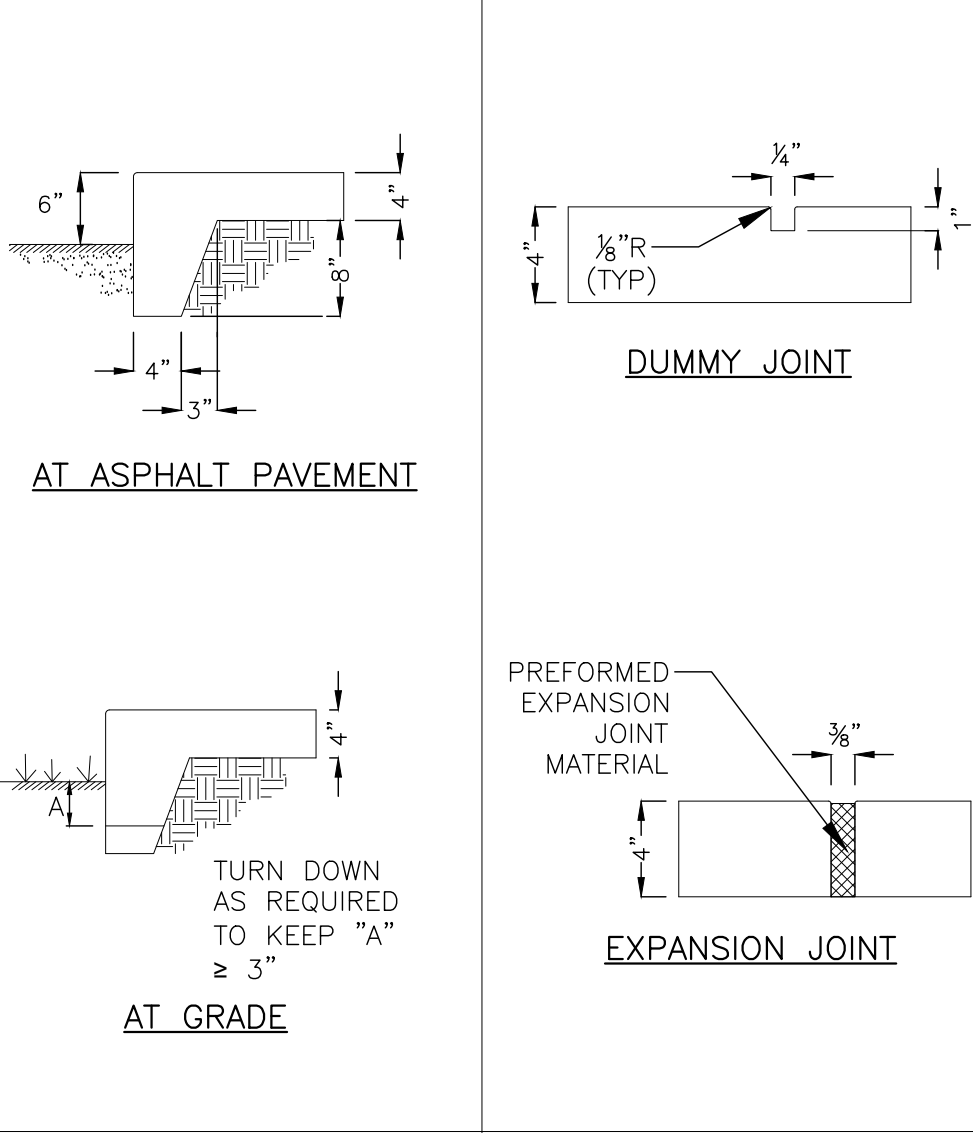
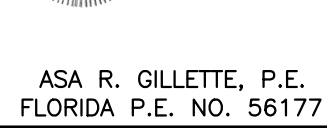
SECTION MISCELLANEOUS DETAILS


 David A. Gallette
 State of New Hampshire
 License No. 10000
 Exp. 12/31/2024

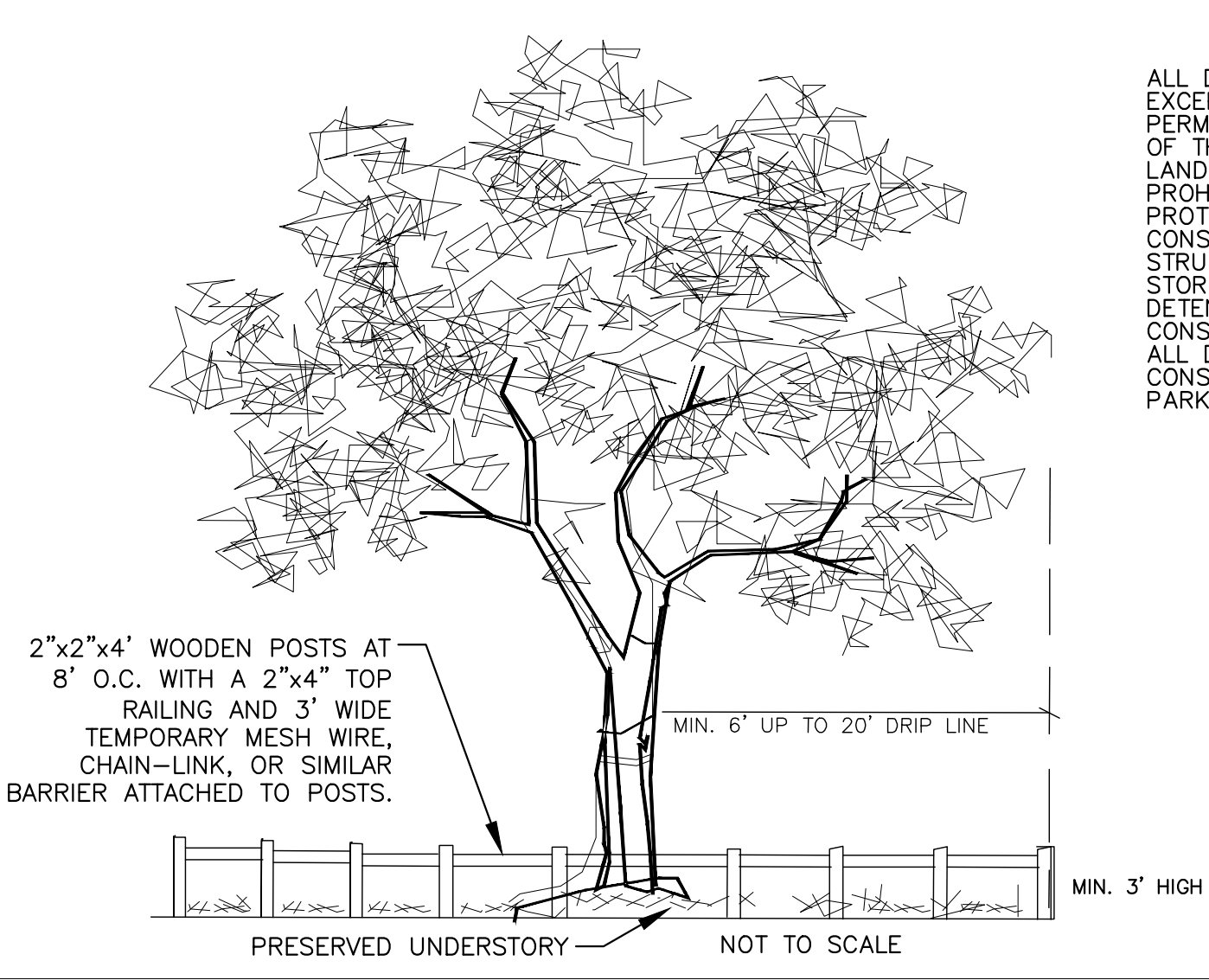
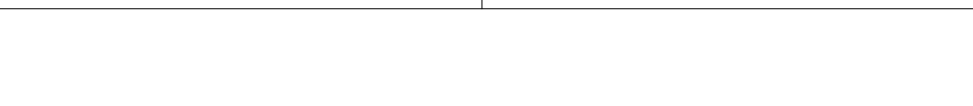
Drawn by: AGH
 Gallette
 01-04-24 Gallette, D.A.,
 01-04-24
 email: dave@galletteinc.com
 cell: 603.222.4432 fax: 603.222.4432

DT-1
 Page 12 of 13

	ASA R. GILLETTE, P.E. FLORIDA P.E. NO. 56177	Project No. 21-10-36
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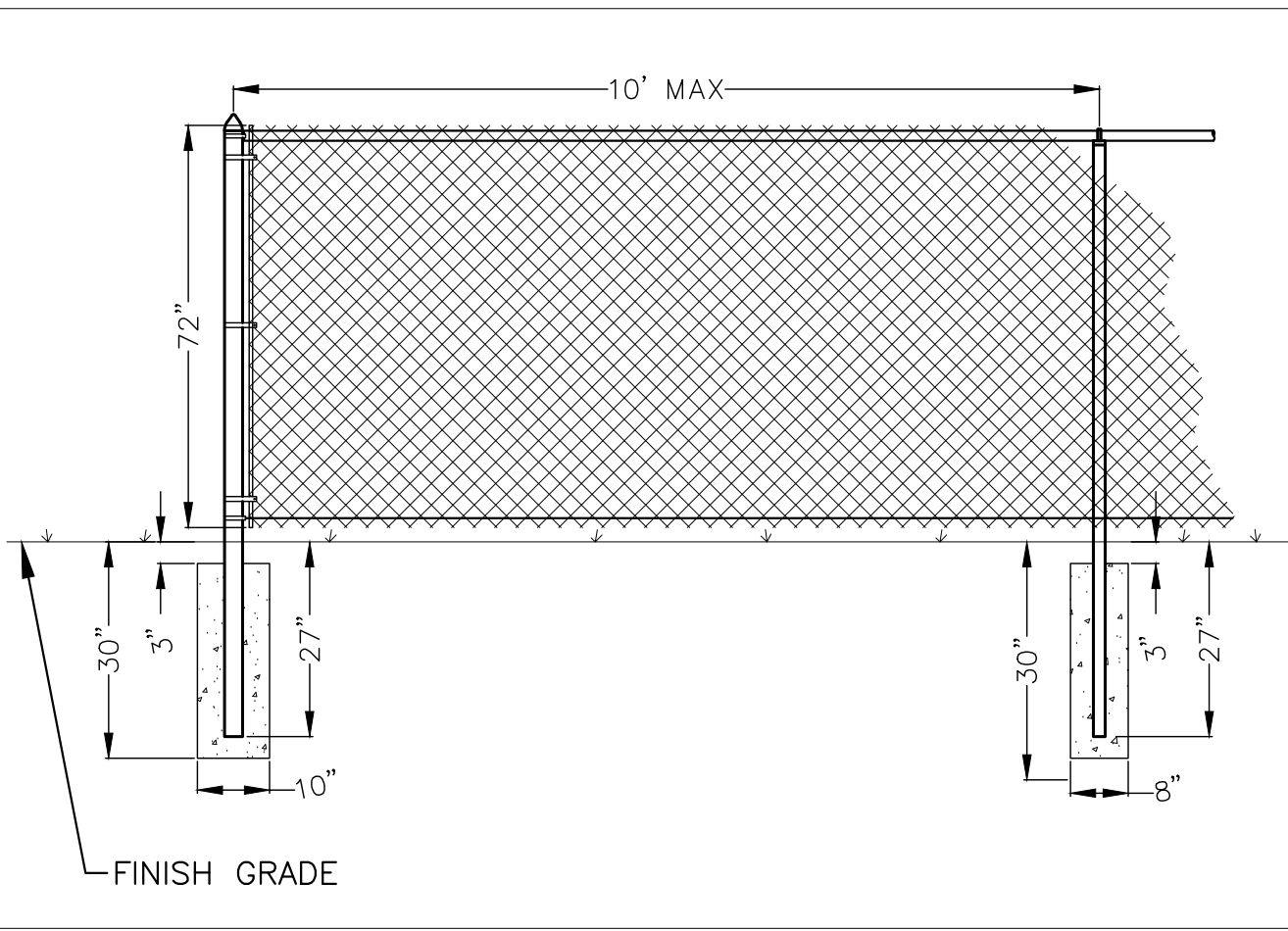


SIDEWALK TURNDOWN DETAILS | SIDEWALK JOINT DETAILS



TREE PROTECTION DETAIL

10' MAX



	FENCE DETAIL
--	--------------

8. ARTISAN HOMES, LLC

ARTISAN HOMES LLC

SOUTH 4TH STREET FERNANDINA BEACH, FL 32034 PHONE: (904) 261-8819	2754 BAILEY ROAD FERNANDINA BEACH, FLORIDA
---	---

TYPICAL PAVEMENT DETAIL				
K				

K				Scale:
J				
I				
H				Printed Name: _____

H			Project Mgr: AG
G			
F			Designed by: AG/EC

E				Drawn by: <u>EC/AG</u>
D				
C				

B				QA/QC: _____ NG
A	EC	03/01/22	ISSUE FOR C.O.F.B. REVIEW	
No.	Rv	Date	Revision	

Certificate of Authorization No. 9

20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034

ARTISAN HOMES LLC

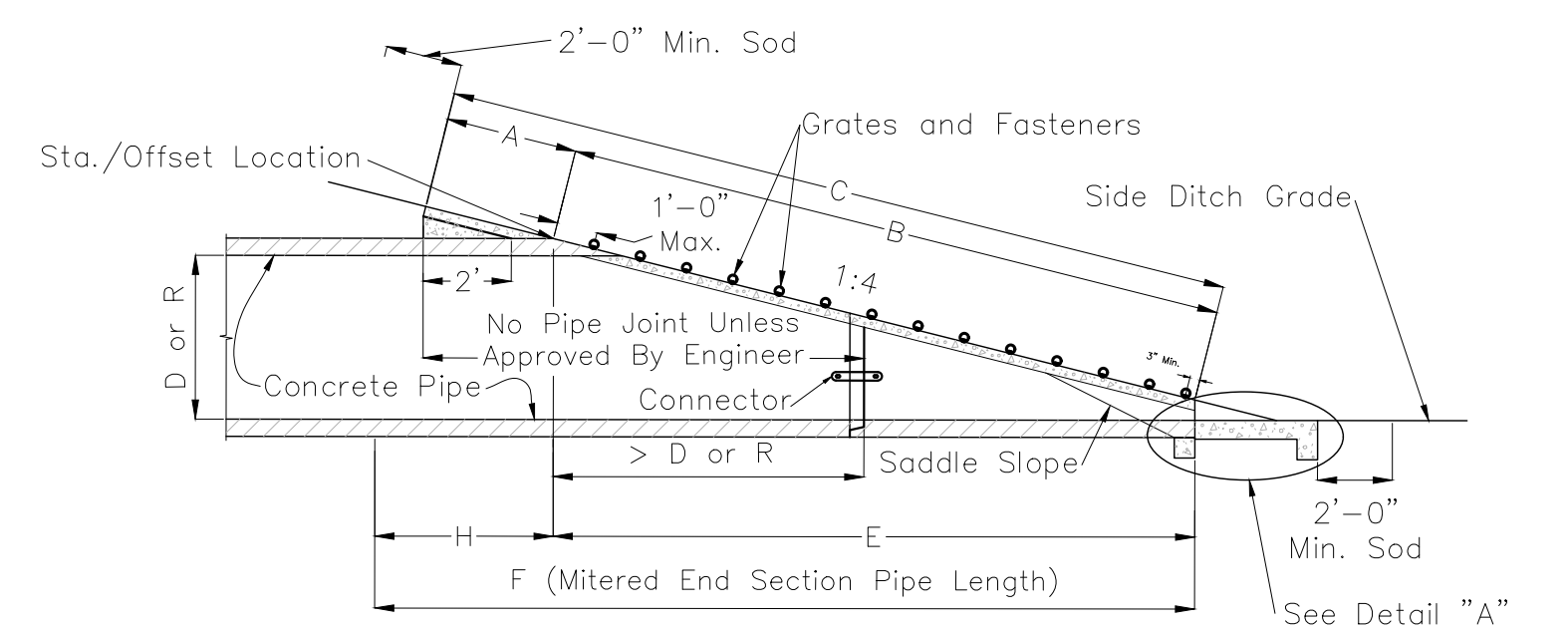
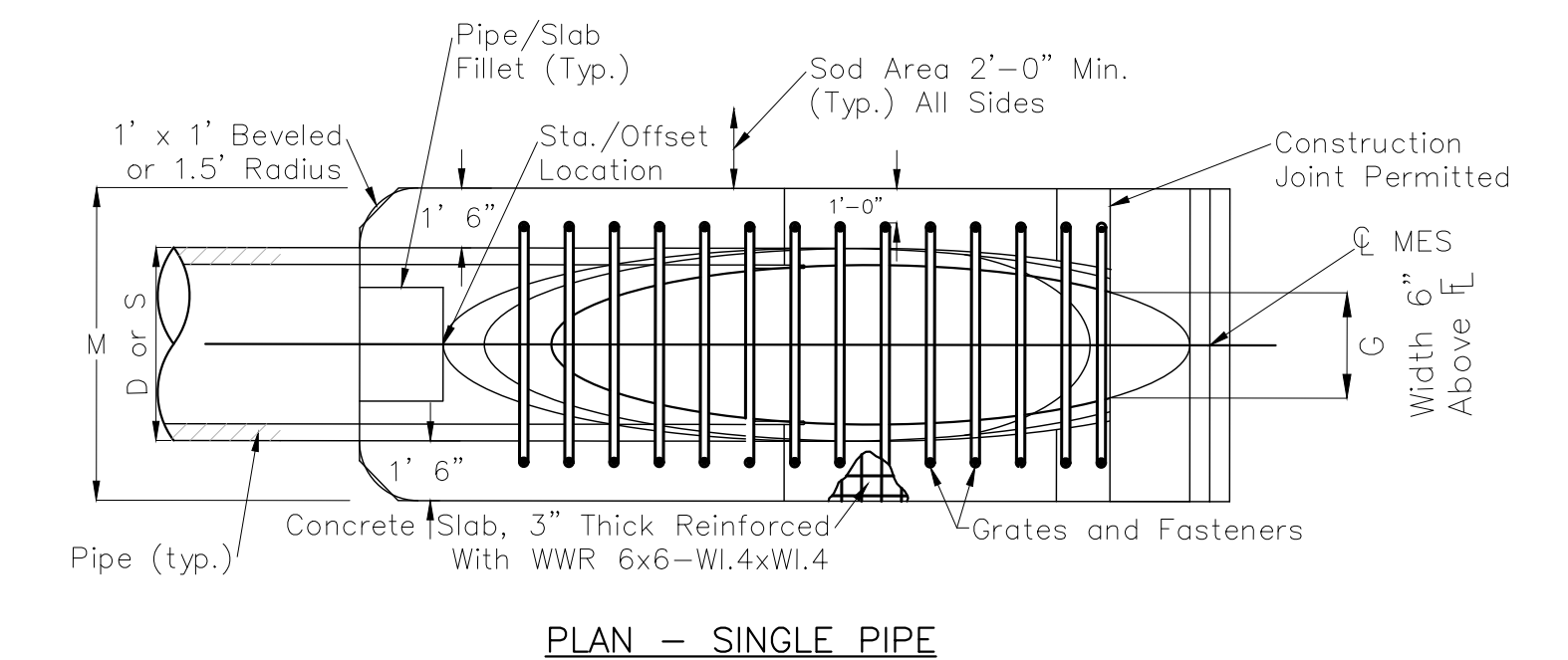
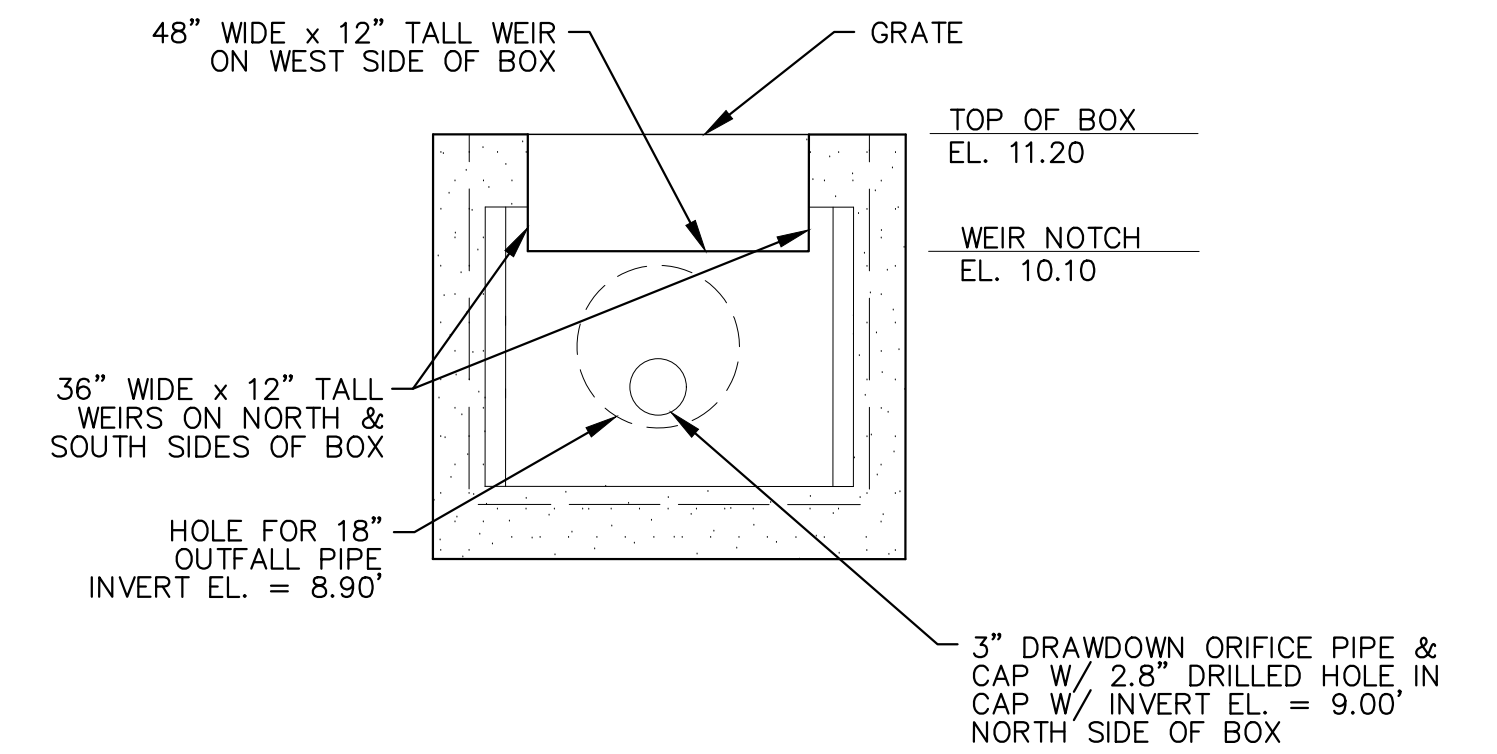
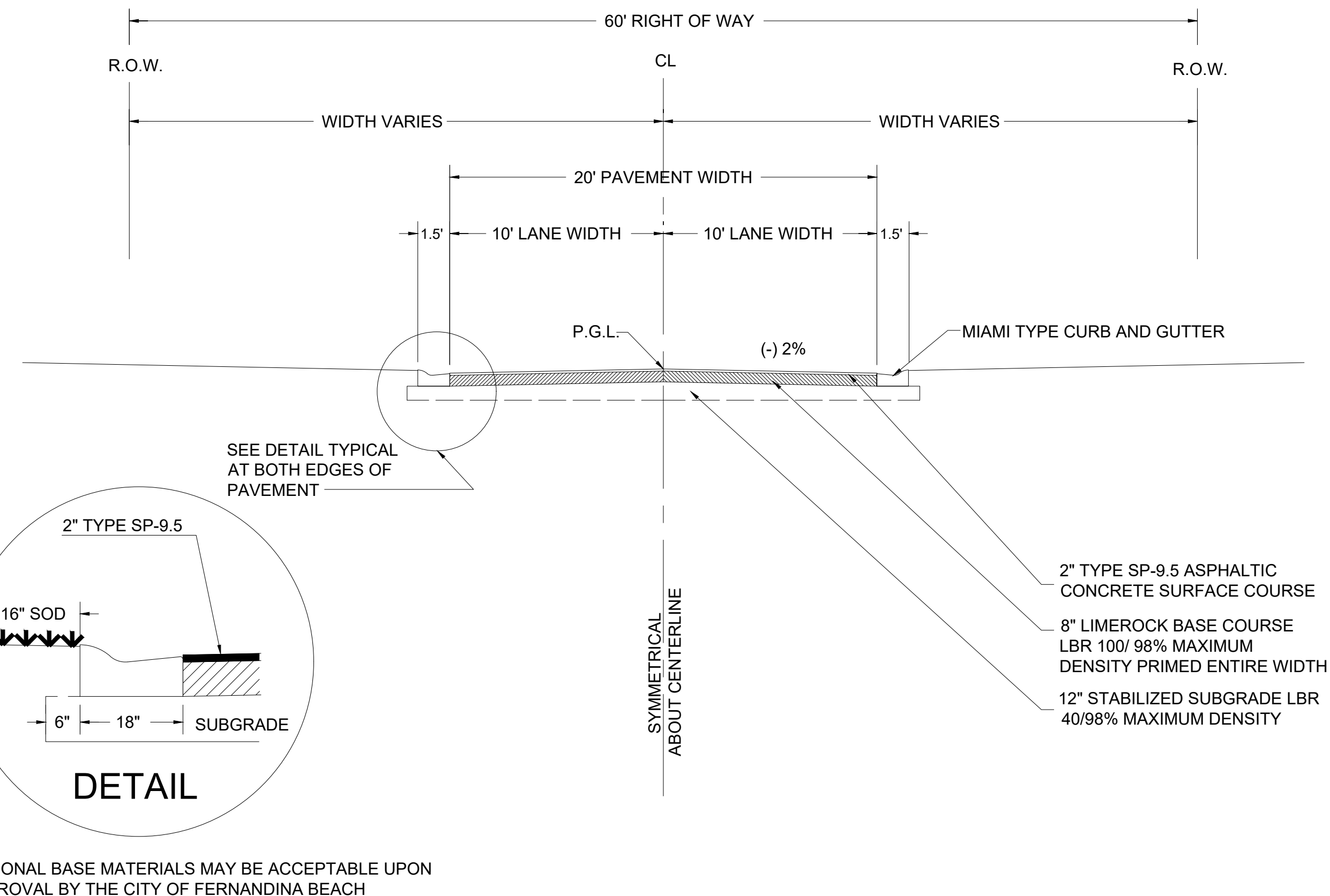
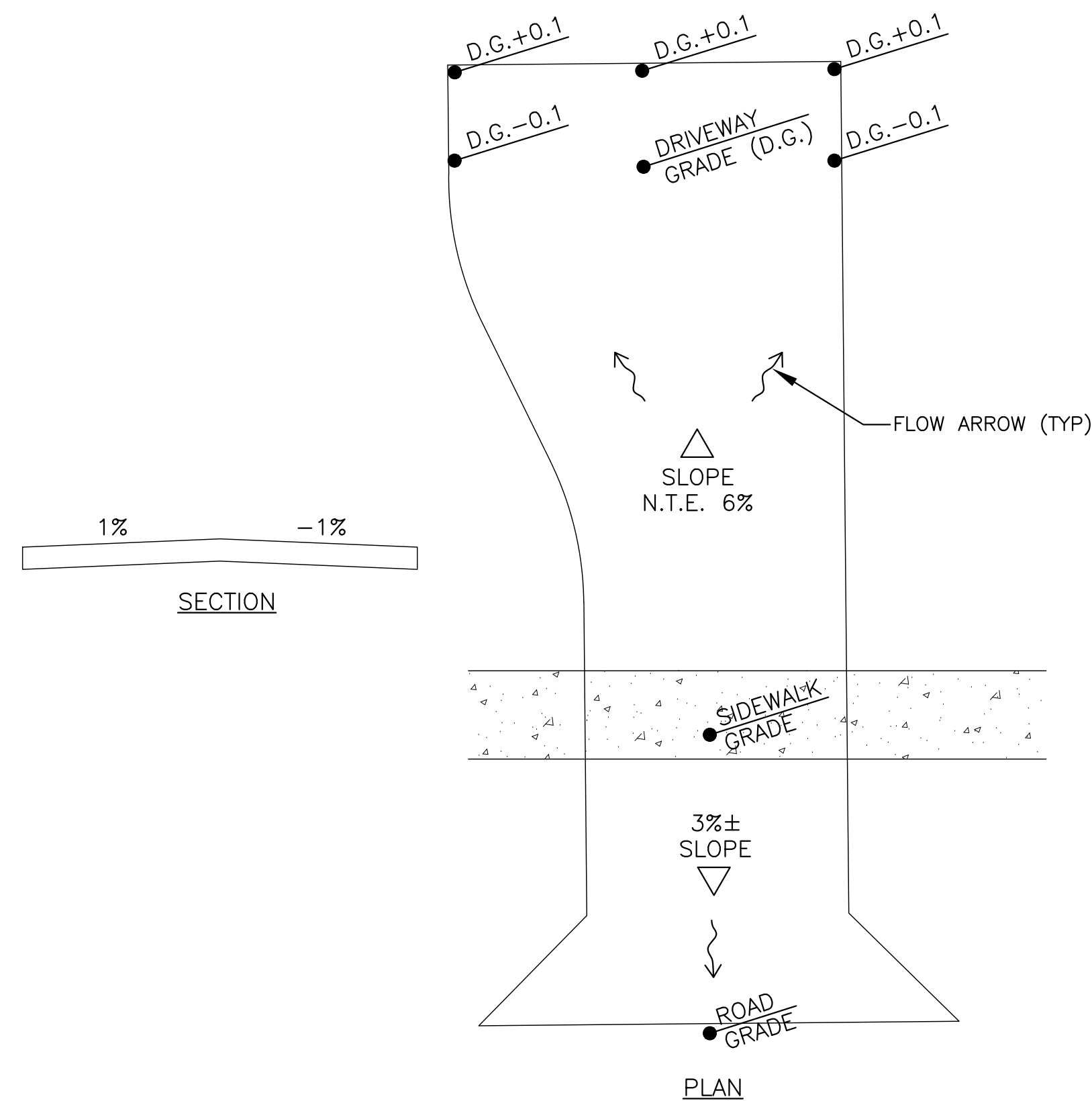
2754 BAILEY ROAD
FERNANDINA BEACH, FLORIDA

ISLAND CIRCLE SUBDIVISION

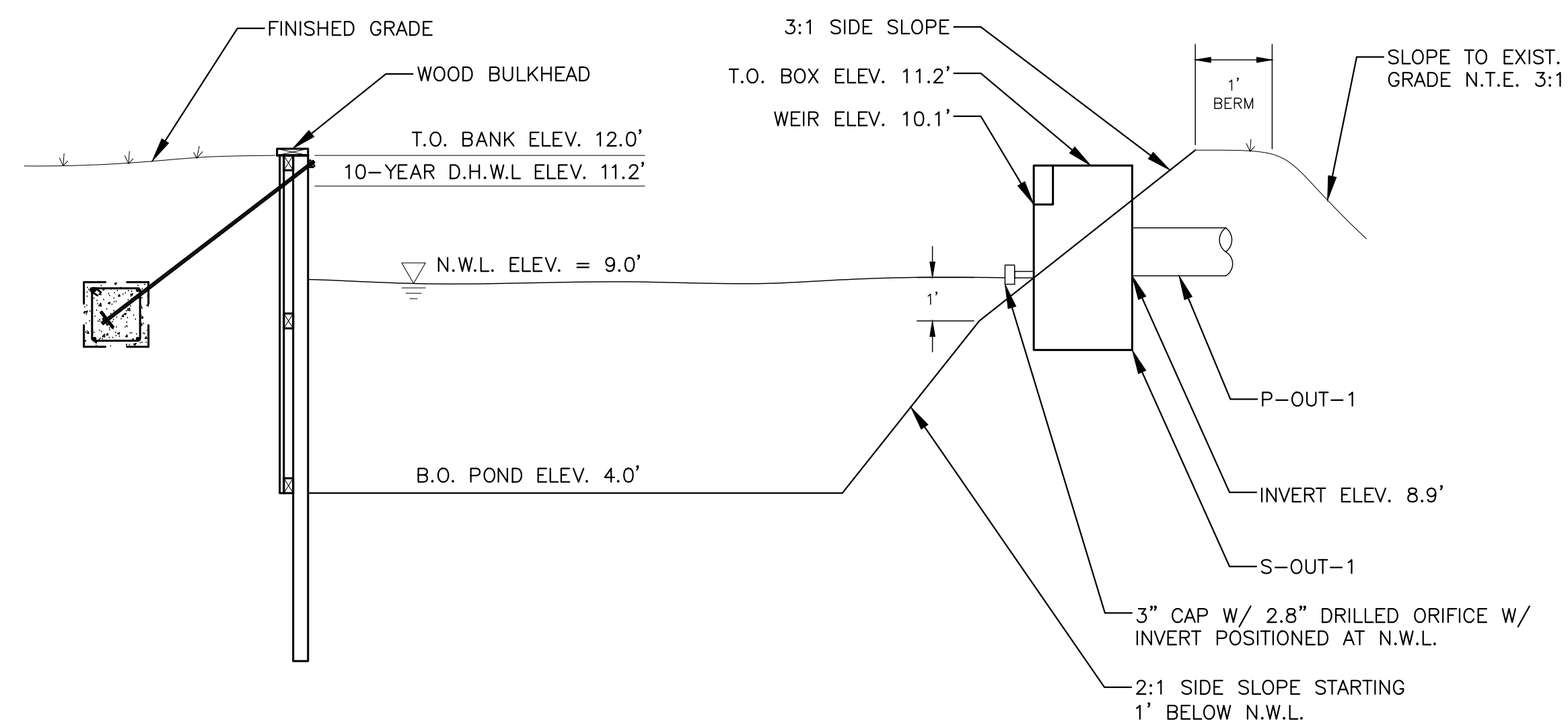
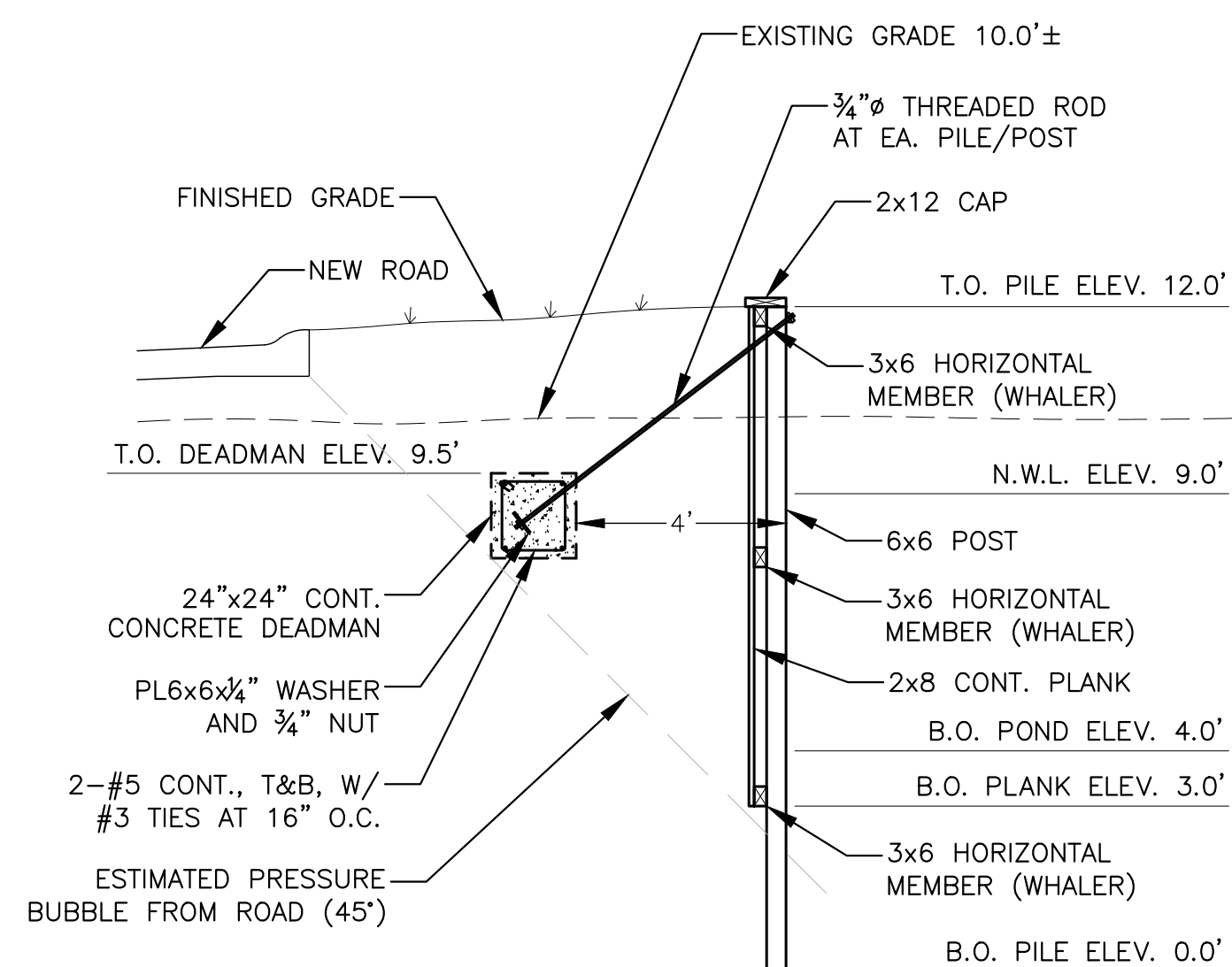
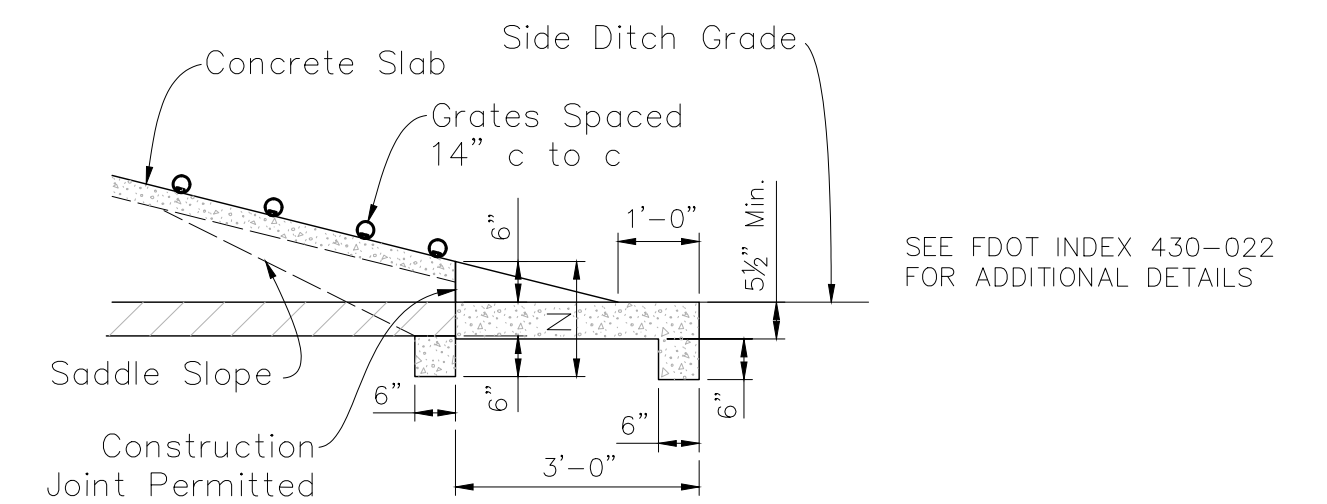
MISCELLANEOUS DETAILS



104 B. CHLETTS, P. F.



DIMENSIONS FOR SINGLE 24" CONCRETE PIPE												
DIA. D	X	A	B	C	E	F	G	H	M	N	3" CONC. SLAB (CY)	SODDING (SY)
24"	3'-5"	2.53'	7.18*	9.71'	7.03*	11'	1.73'	4.0'	5.50'	1.25'	1.02	10
			*6.42'			DIMENSIONS PERMITTED TO ALLOW USE OF 8'						
			STANDARD PIPE			LENGTHS.						



K					Scale:
J					Project Mgr: AG
I					Designed by: AG/EC
H					Drawn by: EC/AG
G					QA/QC: NG
F					
E					
D					
C					
B					
A	EC	03/01/22	ISSUE FOR C.O.F.B. REVIEW		
No.	By	Date	Revision		

Gillette & Associates, Inc.
 20 South 4th Street
 Fernandina Beach, FL 32034

Certificate of Authorization No. 9332

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 AND NOTES TAKE PREFERENCE. DRAWING
 REDUCED IF LESS THAN 22" x 34"

G & A
GILLETTE & ASSOCIATES, INC.
20 SOUTH 4TH STREET
FERNANDINA BEACH, FL 32034
PHONE: (904) 261-8819

POND DETAIL

ARTISAN HOMES LLC

2754 BAILEY ROAD
FERNANDINA BEACH, FLORIDA

ISLAND CIRCLE SUBDIVISION

DRAINAGE DETAILS

REGISTERED DESIGN PROFESSIONAL



Digitaly signed by ASA R
GILLETTE
DN: cn=ASA R GILLETTE, o=US,
email=asa.r.gillette@asciars.com,
c=US
Date: 2022.03.02 14:00:36 -0500

ISSUE DATE: 03/01/22

Sheet
DT-2
Page 13 of 13

Project No. 21-10-36

Technical Review Committee: Application - Submission #9254

Date Submitted: 5/17/2022

Technical Review Committee: Application

USE THIS FORM TO To apply for review of site plans, preliminary subdivision plats, final subdivision plats, minor subdivisions, and amendments to previously issued local development orders, change of use, and nonresidential building expansions.

Fees

☐Initial submittal + two submittals of full site plans. \$400 + 2% of the project construction cost (whichever is greater) for up to a maximum of \$5,500.

☐3rd and subsequent resubmittals. 1/2 of original application fees

☐Change of Use Determination Letter \$50

☐Engineering Review (Consultant Hourly Rate) \$125



November 24 Meeting is cancelled in observance of Thanksgiving Day.

IMPORTANT NOTES

This application will be routed to the various departments for review of the applicable codes and design standards.

****Please note that the TRC now meets at 10:00 AM on the second and fourth Thursdays of each month (unless otherwise indicated) in City Hall Chambers.****

Please see the Land Development Code (LDC) for detailed information:



Requirements for All Site Plans – see LDC section 11.01.04



Requirements for Subdivision Plats – see LDC section 11.01.05



Determination for Completeness & sufficiency – see LDC section 11.03.01



Review and Compliance Report by the Technical Review Committee – see LDC Section 11.03.02



Construction and Improvements – see LDC section 11.05.00



Amendments to Local Development Orders – see LDC section 11.06.00

The LDC is available for review at

www.fbfl.us/LDC

Application Requirements



See Development Plan Submittal Checklist



All applications are subject to a determination of completeness as required by Land Development Code section 11.03.01.



A determination of completeness is a determination that all required documents and plans have been submitted, and whether all fees have been paid.



A determination of completeness is not a determination of compliance with substantive standards and criteria.

Key Contacts

TRC will guide your application from start to finish, engaging other City departments as needed.



PROPERTY INFORMATION

Property information can be found at the Nassau County Property Appraiser’s Website’s Map Search

Site Address*

Amelia Island Parkway/Amelia Road

City*

Fernandina Beach

State*

FL

Zip*

32034

Parcel ID #(s)

06-2N-28-0000-0001-0010

Lot

Block

Subdivision

Zoning District

MU-1

Future Land Use Designation*

Mixed Use

Overlay District*

None

Size of Property (acres)*

2.63

Present Property Use*

vacant

Wetlands*

No

Flood Zone*

X, 0.2% chance flood h

Coastal Construction Control Line (CCCL)*

No

OWNER OF RECORD

As recorded with the Nassau County Property Appraiser

First Name*

Russell

Last Name*

Metz

Company (if applicable)

Lineage Properties II, LLC

Mailing Address*

103 Solana Road

City*

Jacksonville

State*

FL

Zip*

32082

Telephone Number*

Email Address*

OWNER'S AGENT

If other than owner. If an agent will be representing the owner, an Owner's Authorization For Agent Representation form must be included

First Name

Last Name

Company (if applicable)

Mailing Address

City

State

Zip

Telephone Number

Email Address

PROJECT INFORMATION

Project Type*

Site Plan Review

Project Name

Proposed Access Road*

Rd Maintenance Jurisdiction*

County Maintained

Project Estimated Cost*

Description of Request*

Site Plan Review Checklist

☐

The names, address, telephone number, fax number, and email address of the person preparing the plan.

☐

The date of preparation and date(s) of any modifications, a north arrow, and a written and graphic scale.

☐

The legal description of the property, consistent with the required survey.



A vicinity map showing the location of the property.



The location of streams, bodies of water, natural features, roads, rights-of-way, street intersections, and paved areas within the boundaries of the property.



The location of streams, bodies of water, dunes and dune systems, and other natural features within 250 feet of the boundaries of the property.



The location of the mean high-water line, if such line is within the boundaries of the property.



A topographic survey, soils report, grading plan, and an erosion control plan.



A general floodplain map indicating areas subject to inundation and high groundwater levels up to a 100-year flood classification.



A statement indicating the distances to schools and public safety facilities intended to serve the proposed development.



The name, plat book, and page number of any recorded subdivision comprising all or part of the site.



The location and use of any existing and proposed principal or accessory buildings and structures, showing proposed setbacks, building heights, and other dimensional requirements of the zoning district.



Elevations of all proposed structures.



The access points, driveway design, on-site parking, including required parking lot landscaping, internal circulation, sidewalks, and bicycle facilities.



The location of existing and proposed utilities, utility services, and easements.



A tree survey showing protected trees, proposed replacement trees, if required, and landscaping and buffering. (See Section 4.05.00.)



A soil erosion and sediment control plan compliant with Section 3.01.04.



For a PUD site plan, a detailed, written list and explanation of how the proposed PUD differs from any provision of this LDC applicable to the underlying zoning district.



For site plans and PUD site plans where development is proposed in phases, the plans shall include phase lines and the following supporting information:



o Timeline for the development; and



o Benchmarks for monitoring the progress of construction of each phase regarding land clearing, soil stabilization and erosion control, installation of infrastructure, and installation of landscaping.



A summary block containing:



o Land use category from the Future Land Use Map in the comprehensive plan;



o Zoning district;



o Total acreage;



o Total square footage for non-residential uses;



o Total density and number of units, proposed and permissible for residential uses;



o Impervious surface ratio calculation, proposed and permissible;



o Floor area ratio calculation, proposed and permissible;



o Total number of parking spaces, required and provided; and



o Number of trees required to be protected, number of trees remaining on the site, and number of trees to be planted.



Additional plans, documents, or reports that are necessary to support the application shall be submitted. Such plans, documents, or reports may include, but are not limited to:



o Subdivision Plats (subdivision plats shall contain the information set forth in Section 11.01.05.)



o Concurrency analysis (concurrency analysis reports shall contain the information set forth in Section 7.04.00.)



o Traffic analysis reports (requirements for traffic analysis reports are set forth in Section 11.01.06.)



o Parking studies (requirements for parking studies are set forth in Section 7.01.04.)



o Stormwater management plans (requirements for stormwater management plans are set forth in Section 7.03.00.)



o Environmental Impact Study (requirements for environmental impact studies are set forth by regional, State, and federal agencies with jurisdiction and in Chapter 3 of the LDC.)



o Tree Protection Plan (Section 4.05.09.)



o Landscape Plan (Section 4.05.00.)



o Lighting Plan (Section 3.05.00.), Section 7.01.04.B)



o Signage Plan (Section 5.03.00.)



o Proof of Ownership “ provide a legal description, Property Identification Number (PIN) and a copy of the warranty deed.



Concurrency analysis reports shall contain the information set forth in Section 7.04.00. Requirements for traffic analysis reports are set forth in Section 11.01.06. Requirements for parking studies



“ Any development application for development proposed within the Airport Overlay Zone established for the Fernandina Beach Municipal Airport shall provide confirmation of notification to the FAA as r

School Concurrency

New residential development is required to demonstrate compliance with school concurrency as regulated in the City of Fernandina Beach through the City of Fernandina Beach’s Comprehensive Plan Intergovernmental Coordination Element and the Interlocal Agreement Amendment for Public School Facility Planning adopted by the City on May 21, 2019. No new residential rezoning, preliminary plat, site plan or functional equivalent may be approved by the City unless the residential development is exempt from requirements outlined in Section 9.13 of the Amended Interlocal Agreement OR a School Concurrency Reservation Letter has been issued by the School Board indicating that adequate school facilities exist.

Certification*

☒

By signing below, I certify that the information contained in this application is true and correct to the best of my knowledge at the time of the application.

☒

I acknowledge that I understand and have complied with all of the submittal requirements and procedures.

Applicant First Name*

Asa

Applicant Last Name*

Gillette

Today's Date*

5/17/2022

Upload Supporting Documentation*

BeachesDermatology-DrainageReport_051022.pdf

Upload 2

beaches derm_amelia island_civil.pdf

Upload 3

agent form.jpeg

Upload 4

sunbiz.pdf

Upload 5

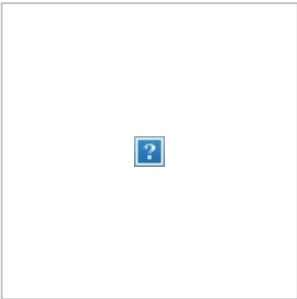
deed.pdf

Upload 6

21310.pdf

Upload 7

Choose File

No file selected

DEPARTMENT OF PLANNING & CONSERVATION

204 Ash Street | Fernandina Beach, FL 32034 | 904 310-3480 | www.fbfl.us/planning



Civil & Environmental Engineering • Mechanical & Structural Engineering • Construction Management

AGENT AUTHORIZATION

I, Russell D. Metz of Lineage Properties II, LLC the owner of the following property(s) do hereby authorize, Gillette & Associates, Inc. to act as my Agent in conjunction with all site plan applications and any other Local, State and/or Federal development permits necessary for development of the following property, without any further notice.

Property ID Number(s): 06-2N-28-0000-0001-0010

Location: Amelia Island Parkway, Fernandina Beach

[Signature]
Signature

5/6/2022
Date

904-273-2717 ext. 314
Telephone Number

STATE OF FLORIDA
COUNTY OF St. Johns

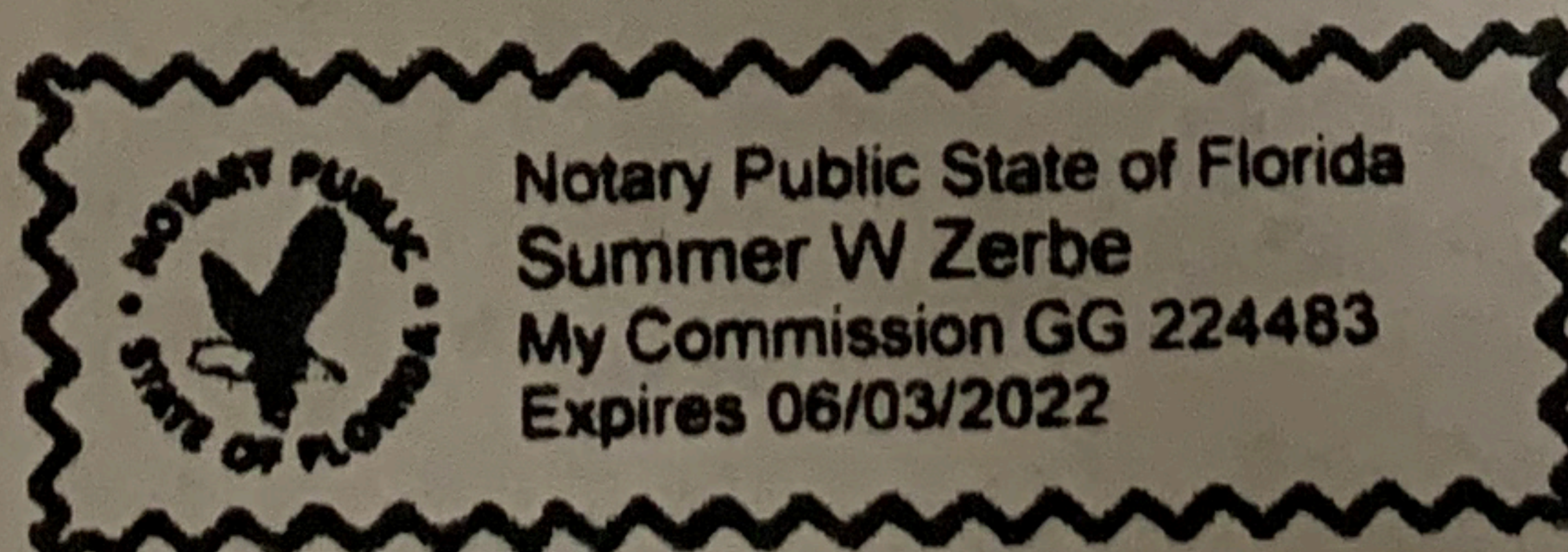
Subscribed and sworn to before me this 6 day of May, 2022.

Who is personally known to me ☒ or has produced identification

Summer W. Zerbe
Notary Public: Signature

Summer W. Zerbe
Printed Name

My Commission Expires: 06/03/2022



Prepared by:
Andrea F. Lennon, P.A.,
3391 South Fletcher Avenue
Fernandina Beach, Florida 32034

File Number: 21-260

Corporate Warranty Deed

This Indenture, made, **November 19, 2021 A.D.**

Between J. H. Cobb Properties, Inc., a Florida corporation whose post office address is: **10138 Whippoorwill Lane, Jacksonville, Florida 32256**, a corporation existing under the laws of the **State of Florida**, Grantor and
Lineage Properties II, LLC, a Florida limited liability company whose post office address is: **103 Solana Road, Suite B, Ponte Vedra Beach, Florida 32082**, Grantee,

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Nassau, State of Florida, to wit:

A part of Sections 6 and 7, Township 2 North, Range 28 East, Nassau County, Florida, being more particularly described as follows: For a Point of Beginning commence at the corner common to Sections 3, 5, 6, 7, said Township 2 North, Range 28 East; thence South 89 degrees 47 minutes 51 seconds East, along the Northerly line of said Section 7, a distance of 363.00 feet to the West right of way line of Amelia Road (as now monumented); thence South 07 degrees 10 minutes 58 seconds West along said West right of way line, a distance of 183.15 feet to the Northerly right of way line of Amelia Island Parkway (a 113.5 foot right of way as now monumented); thence Southwesterly and Northwesterly along said Northerly right of way line the following Three courses: course (1) South 83 degrees 09 minutes 16 seconds West, a distance of 122.84 feet to the Point of Curve of a curve concave Northerly having a radius of 743.25 feet; course (2) Northwesterly 614.66 feet along the arc of said curve, a chord bearing North 73 degrees 09 minutes 15 seconds West and a chord distance of 597.29 feet to the Point of Tangency of said curve: course (3) North 49 degrees 27 minutes 46 seconds West a distance of 39.80 feet to an intersection with the Northerly line of aforesaid Section 6; thence South 89 degrees 47 minutes 51 seconds East, along said Northerly line of Section 6, a distance of 383.77 feet to the Point of Beginning.

A part of Sections 6 and 7, Township 2 North, Range 28 East, Nassau County, Florida, being more particularly described as follows: For a Point of Beginning commence at the corner common to Sections 3, 5, 6, 7, said Township 2 North, Range 28 East; thence South 89 degrees 47 minutes 51 seconds East, along the Northerly line of said Section 7, a distance of 363.00 feet to the West right of way line of Amelia Road (as now monumented); thence South 07 degrees 10 minutes 58 seconds West along said West right of way line, a distance of 183.15 feet to the Northerly right of way line of Amelia Island Parkway (a 113.5 foot right of way as now monumented); thence Southwesterly and Northwesterly along said Northerly right of way line the following Three courses: course (1) South 83 degrees 09 minutes 16 seconds West, a distance of 122.84 feet to the Point of Curve of a curve concave Northerly having a radius of 743.25 feet; course (2) Northwesterly 614.66 feet along the arc of said curve, a chord bearing North 73 degrees 09 minutes 15 seconds West and a chord distance of 597.29 feet to the Point of Tangency of said curve: course (3) North 49 degrees 27 minutes 46 seconds West a distance of 39.80 feet to an intersection with the Northerly line of aforesaid Section 6; thence South 89 degrees 47 minutes 51 seconds East, along said Northerly line of Section 6, a distance of 383.77 feet to the Point of Beginning.

Prepared by:
Andrea F. Lennon, P.A.,
3391 South Fletcher Avenue
Fernandina Beach, Florida 32034

File Number: 21-260

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: 06-2N-28-0000-0001-0010

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

J. H. Cobb Properties, Inc., a Florida
corporation

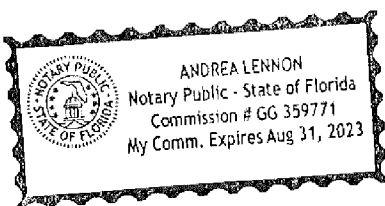
Signed and Sealed in Our Presence:

By: [Signature]
James H. Cobb, III
Its: President

[Signature]
Witness
Print Name: Andrea F. Lennon
[Signature]
Witness
Print Name: Kelly F. Morris

State of Florida
County of Nassau

The foregoing instrument was acknowledged before me by means of [X] physical presence or [] online notarization, this 18 day of November, 2021, by James H. Cobb, III, the President of J. H. Cobb Properties, Inc., a Florida corporation, a corporation existing under the laws of the State of Florida, on behalf of the corporation. He has produced a driver's license as identification.



[Signature] (Seal)
Notary Public
Notary Printed Name: Andrea F. Lennon
My Commission Expires:



[Department of State](#) / [Division of Corporations](#) / [Search Records](#) / [Search by Entity Name](#) /

Detail by Entity Name

Florida Limited Liability Company
LINEAGE PROPERTIES II, LLC

Filing Information

Document Number	L21000053332
FEI/EIN Number	NONE
Date Filed	02/10/2021
State	FL
Status	ACTIVE

Principal Address

2437 FOX FOREST DRIVE
LUTZ, FL 33549

Changed: 02/10/2022

Mailing Address

103 SOLANA ROAD,
SUITE B
PONTE VEDRA BEACH, FL 32082

Registered Agent Name & Address

CLARENCE BOUDREAUX
103 SOLANA ROAD, SUITE B
PONTE VEDRA BEACH, FL 32082

Authorized Person(s) Detail

Name & Address

Title MGR

CLARENCE BOUDREAUX
103 SOLANA ROAD,
SUITE B
PONTE VEDRA BEACH, FL 32082

Title MGR

RUSSELL METZ
103 SOLANA ROAD, SUITE B
PONTE VEDRA BEACH, FL 32082

Annual Reports

Report Year	Filed Date
2022	01/31/2022

Document Images

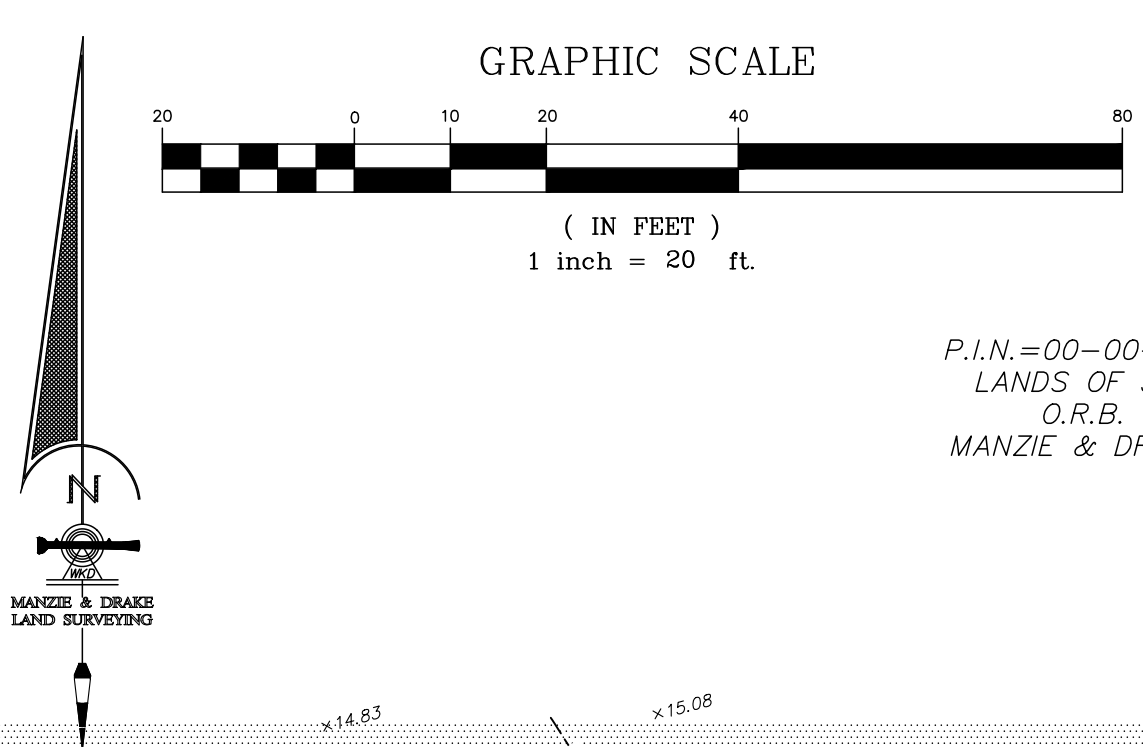
[01/31/2022 -- ANNUAL REPORT](#)

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[02/10/2021 -- Florida Limited Liability](#)

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MAP OF BOUNDARY & TOPOGRAPHIC SURVEY
OFFICIAL RECORDS BOOK 1299, PAGE 527
A PART OF SECTIONS 6 AND 7, TOWNSHIP 2 NORTH, RANGE 28 EAST, NASSAU COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: FOR A POINT OF BEGINNING COMMENCE AT THE CORNER COMMON TO SECTIONS 3, 5, 6, SAID TOWNSHIP 2 NORTH, RANGE 28 EAST; THENCE S 89°47'51" E, ALONG THE NORTHERLY LINE OF SAID SECTION 7, A DISTANCE OF 363.00 FEET TO THE WEST RIGHT-OF-WAY LINE OF AMELIA ROAD (AS NOW MONUMENTED); THENCE S 07°10'58" W ALONG SAID WEST RIGHT-OF-WAY LINE, A DISTANCE OF 183.15 FEET TO THE NORTHERLY RIGHT-OF-WAY LINE OF AMELIA ISLAND PARKWAY (A 113.5 FOOT RIGHT-OF-WAY AS NOW MONUMENTED); THENCE SOUTHWESTERLY AND NORTHWESTERLY ALONG SAID NORTHERLY RIGHT-OF-WAY LINE, THE FOLLOWING THREE COURSES: COURSE (1) S 83°09'16" W, A DISTANCE OF 122.84 FEET TO THE POINT OF CURVE OF A CURVE CONCAVE NORTHERLY HAVING A RADIUS OF 743.25 FEET; COURSE (2) NORTHWESTERLY 614.66 FEET ALONG THE ARC OF SAID CURVE, A CHORD BEARING N 73°09'15" W, AND A CHORD DISTANCE OF 597.29 FEET TO THE POINT OF TANGENCY OF SAID CURVE; COURSE (3) N 49°27'46" W, A DISTANCE OF 39.80 FEET TO AN INTERSECTION WITH THE NORTHERLY LINE OF AFORESAID SECTION 6; THENCE S 89°47'51" E, ALONG SAID NORTHERLY LINE OF SECTION 6, A DISTANCE OF 383.77 FEET TO THE POINT OF BEGINNING, CONTAINING 2.63 ACRES MORE OR LESS.



P.I.N.=00-00-30-0448-0104-0000
LANDS OF 3243 AMELIA RD LLC
O.R.B. 2471, PAGE 203
MANZIE & DRAKE FILE NO. B-1577

SITE BENCHMARK
NAIL & DISK (MANZIE LB 7039)
IN WOODEN UTILITY POLE
(ELEVATION 17.14' N.A.V.D. 1988)
FIELD BOOK X-354, PAGE 21

POINT OF BEGINNING
SECTION CORNER COMMON TO SECTIONS 3,
5, 6 & 7, TOWNSHIP 2 NORTH, RANGE 28
EAST, NASSAU COUNTY, FLORIDA.
FOUND 4"x4" CONCRETE MONUMENT

V A C A N T
O.R.B. 1299
PAGE 527
±2.63 ACRES

AMELIA ROAD
(VARIED WIDTH RIGHT-OF-WAY)

AMELIA ISLAND PARKWAY
(113.5' RIGHT-OF-WAY)

- LEGEND
- OH— = OVERHEAD UTILITY WIRES
 - C— = CONCRETE FLATWORK
 - S— = SEWER CLEANOUT
 - W— = WOOD UTILITY POLE
 - B— = BACKFLOW PREVENTION SYSTEM
 - B— = BENCHMARK
 - C— = SECTION CORNER
 - C— = CENTERLINE
 - A— = CENTRAL ANGLE
 - CB = CHORD BEARING
 - CD = CHORD DISTANCE
 - CH = CHORD
 - x—x— = CHAIN LINK OR WIRE FENCE
 - CMP = CORRUGATED METAL PIPE
 - RCP = REINFORCED CONCRETE PIPE
 - HDPE = HIGH-DENSITY POLYETHYLENE
 - L— = LENGTH
 - NAVD = NORTH AMERICAN VERTICAL DATUM
 - (BR) = BEARING REFERENCE
 - O.R.B. = OFFICIAL RECORDS BOOK
 - R = RADIUS
 - R/W = RIGHT-OF-WAY
 - P.I.N. = PARCEL IDENTIFICATION NUMBER

TREE LEGEND
CDT = CEDAR TREE
CH = CHINESE TALLOW
CHY = BLACK CHERRY TREE
O = OAK TREE
OH = ORNAMENTAL TREE
P = PINE TREE
PM = PALM TREE

TREE DISCLAIMER:
NUMBER OF TREES SHOWN ON SURVEY: 220

SPECIES OF TREES HAVE BEEN IDENTIFIED TO THE BEST OF OUR KNOWLEDGE AND BELIEF; HOWEVER MANZIE & DRAKE LAND SURVEYING WILL ASSUME NO LIABILITY, EXPRESSED OR IMPLIED, FOR THE CORRECTNESS OF SAID SPECIES IDENTIFICATION.

TREES HAVE BEEN LOCATED AS ACCURATELY AS POSSIBLE UNDER CURRENT CONDITIONS AND HAVE A POSITIONAL TOLERANCE OF APPROXIMATELY 2 TO 3 FEET. CRITICAL TREES SHOULD BE VERIFIED PRIOR TO CONSTRUCTION.

- SURVEY NOTES:
- 1) The "Legal Description" hereon is in accord with the description provided by the client
 - 2) Underground improvements were not located or shown.
 - 3) Lands shown hereon were not abstracted by this office for easements, rights-of-way, ownership or other instruments of record.
 - 4) Bearings shown hereon are based on S89°47'51"E for the northerly line of subject property. The bearing reference line is indicated as thus (BR).
 - 5) The property shown hereon lies within flood zone "X" as per F.E.M.A. Flood Insurance Rate Map, Panel "12089C 0377 G", Dated "08/02/2017". Flood Zone information listed above and shown on this survey is provided as a courtesy and is approximate at best. All data should be verified by Nassau County Building Department for accuracy. We assume no liability for its accuracy. Flood Zone information is not covered by the certification hereon and is not required to be shown per Chapter 5J-17, Florida Administrative Code, pursuant to Section 472.027, Florida Statutes.
 - 6) Site Benchmark is as shown hereon.
 - 7) Elevations shown hereon refer to North American Vertical Datum of 1988. (N.A.V.D. '88)
 - 8) The Reference Benchmark is "Fernport" (Elevation = 10.50' N.A.V.D. 1988)

CERTIFIED TO:
BEACHES DERMATOLOGY

THE INFORMATION SHOWN HEREON MEETS THE STANDARDS OF PRACTICE SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

MANZIE & DRAKE LAND SURVEYING
117 South Ninth Street, Fernandina Beach, FL 32034
(904) 491-5700 www.ManzieAndDrake.com
Certificate of Authorization Number "LB 7039"
"OUR SIGHTS ARE ON THE FUTURE, SET YOUR SITES ON US."

SCALE: 1"=20' JOB NO: 21310 DATE: 09/16/2021 CADD: WKD
F.B. NO: X-354 PAGE NO: 21 FIELD CREW: SS FILE NO: C-1092



Civil & Environmental Engineering • Mechanical & Structural Engineering • Construction Management

SITE DRAINAGE EVALUATION

**Beaches Dermatology Medical Offices
FERNANDINA BEACH, FLORIDA**

BY

**Gillette & Associates, Inc.
20 South 4th Street
Fernandina Beach, FL**

Issue Date: May 10, 2022



Digitally signed by ASA R
GILLETTE
DN: cn=ASA R GILLETTE, c=US,
o=Florida,
email=asa@gilletteassociates.com
Date: 2022.05.17 16:39:31 -0400

PROJECT DESCRIPTION

The proposed project consists of two medical office buildings and associated parking being constructed on a 2.63 acre lot, located at Amelia Island Parkway and Amelia Road in the City of Fernandina Beach. The site will be served by a wet detention pond, designed to provide storm water treatment and meet pre vs. post discharge requirements for the mean annual, 10-year, and 25-year storm events.

EXISTING CONDITIONS

The project site is currently undeveloped and wooded. In its pre-developed state, the site has a single drainage basin, PRE-1, which is mostly flat around an elevation of 14.0'. Storm water is currently not contained on the site and runs off into swales along Amelia Island Parkway and Amelia Road. There are no wetlands or surface waters on site.

PROPOSED CONDITIONS

In the proposed post-developed state, the site will contain 1.24 acres of impervious area and a wet detention pond with an area of 14,580 s.f. at top of bank. Stormwater runoff from the proposed improvements is to be routed to the pond through a series of stormwater pipes and structures.

The pond has been designed with a top of bank elevation of 15.0' and a normal water level elevation of 12.5' based on geotechnical data provided in the geotechnical report, which is included at the end of this report. The eastern side of the pond is bulkheaded down to an elevation of 9.0', and the rest of the pond banks slope at 3:1 from the top of bank down to the bottom of the pond at elevation 5.0'. The proposed outfall for the pond is a drop structure on the southwest side of the pond. The weir elevation is 13.5' in an effort to minimize the discharge to the boundary and meet the pre vs. post conditions for the project.

DRAINAGE ANALYSIS

The analysis was conducted over the entire 2.63 acre site. In the proposed post-developed state, the site will contain 1.31 acres of impervious area and a wet detention pond with an area of 14,580 s.f. at top of bank. A composite curve number of 89.3 was used in the ICPR analysis based on a weighted average of the curve numbers for the impervious area, pervious area, and pond area. This calculation is shown in the pond spreadsheet within this report.

A Time of Concentration of 15 minutes was used in the ICPR analysis. The factors considered for this calculation are a Minimum Time of Concentration of 10 minutes, a maximum of 50 feet of overland flow at 1 foot per second, and 580 feet of pipe flow at 2.5 feet per second. The calculation is shown below. After rounding up to the nearest whole number, 15 minutes was determined as the ToC value for the analysis.

$$\text{ToC} = \text{Min. ToC} + [(\text{Length of Overland Flow}/1 \text{ fps}) + (\text{Length of Pipe flow}/2.5 \text{ fps})]/60 \text{ s} = 15 \text{ Min.}$$

The site was modeled with ICPR and the results are summarized on the following page. The pre-development discharge for the site is represented in the Hydrology Report by the "Flow Max cfs" value for the basin "pre-1" for the respective storm event. The post-development discharge is represented by the "Max Outflow cfs" value of the "pond-1" node. A node reach diagram has been included in this report to illustrate how ICPR was modeled.

Summary of Design

Mean Annual Storm Event

Predevelopment Discharge (Total):	4.88 cfs
Post Development Discharge (Total):	4.78 cfs
Max Stage Elevation Pond:	14.08 ft
Top of Bank Elevation Pond:	15.00 ft

10 year Storm Event

Predevelopment Discharge (Total):	7.78 cfs
Post Development Discharge (Total):	6.20 cfs
Max Stage Elevation Pond:	14.47 ft
Top of Bank Elevation Pond:	15.00 ft

25 year Storm Event

Predevelopment Discharge (Total):	11.42 cfs
Post Development Discharge (Total):	7.70 cfs
Max Stage Elevation Pond:	14.90 ft
Top of Bank Elevation Pond:	15.00 ft

POND CALCULATIONS

Beaches Dermatology Wet Pond Post Developed - PSP-1

Basin Calculations

Basin 1

Composite Cn

Total Drainage Area =	2.63	Ac.	
Impervious Area =	1.31	Ac.	57% (not including pond area)
Pond Area =	0.33	Ac.	at N.W.L.
Pervious Area =	0.99	Ac.	
Impervious Cn =	98.0		
Pond Cn =	100.0		
Pervious Cn =	74.0		
Composite Cn =	89.3		

Time of Concentration, Tc

Minimum Time of Concentration =	10	min.
Length of Overland Flow @ 1ft/sec. =	50	ft.
Length of Pipe Flow @ 2.5 ft/sec. =	580	ft.
Tc =	15	min.

Equivalent Runoff Coefficient, Ct

Soil Storage, S = 1000/Cn -10 =	1.20	in.
Precipitation Depth, Pt =	7.68	(Zone 5)

$$Ct = 1 - S/Pt * (1.2 - S/(Pt + .8S)) = 0.83$$

Pond Information

El. (ft.)		Area (Ac.)	Volume (Ac.-ft.)
15.00	TOB	0.33	2.05
14.00		0.31	1.73
13.50	Weir	0.29	1.58
13.00		0.28	1.44
12.50	NWL	0.27	1.30
12.00		0.26	1.17
11.00		0.23	0.92
10.00		0.21	0.70
9.00		0.18	0.51
8.00		0.15	0.34
7.00		0.12	0.20
6.00		0.10	0.09
5.00		0.08	0.00

Treatment Volume Required, TVR

TVR = 1" * (1'/12")*Total Drainage Area =	0.22	Ac.-ft.
or		
TVR = 2.5" * (1'/12")*Total Impervious Drainage Area =	0.27	Ac.-ft.
TVR =	0.27	Ac.-ft.

Treatment Volume Provided, TVP

$$TVP = Vol @ Weir - Vol @ NWL = 0.28 \text{ Ac.-ft. } \text{OK}$$

Permanent Pool Volume Required, PPV

Drainage Area, Da =	2.63 Ac.	
Ct =	0.83	
Wet Season Rainfall Depth, R =	28 in.	
Residence Time, RT =	21 days	OK
Length of Wet Season, WS =	153 days	
PPV Required = Da*Ct*R*RT/(WS*12) =	0.70 Ac.-ft.	

Permanent Pool Volume Provided, PPV

PPV Provided = Vol @ NWL =	1.30 Ac.-ft.	OK
----------------------------	--------------	----

Pond Geometry Checks

Depth =	4.50 ft.	OK	
Mean Depth =	4.81 ft.	OK	OK
Average length =	180 ft.		
Average width =	75 ft.		
Ave Length to Width Ratio	2.4 :1	OK	

Sizing Bleed-Down Orifice for Residency Time

Formulas:

$Q=.6*A(2*32.2*h)^{(1/2)}$

$A = D^2 * Pi / 4$

$t = TV/(2*Q*3600)$

$h = (h1 + h2) / 2$

Where:

- Q=Rate of Discharge (cfs)
- A=Orifice Area (ft^2)
- D = Orifice Diameter (ft)
- Pi = 3.14159
- h = Depth of Water Above the Flowline of the Orifice (ft)
- t = Recovery Time (hrs)
- TV = Treatment Volume Required (ft^3)
- h1 = elevation @ TV - NWL Elevation
- h2 = elevation when 1/2 TV has been released - NWL Elevation

Treatment Volume (Ac.-ft.)	0.27	
Weir Elevation (ft.)	13.50	(minimum diameter for 6 in^2 area is 2.8 inches)
NWL Elevation (ft.)	12.50	is value < 2.8 " in diameter? YES
Orifice Diameter (in.)	1.6	Orifice Diameter 2.8

Calculated Values:

h (ft)	0.73
A (ft^2)	0.01
Q (cfs)	0.06
t	28.84

t Should be between 24 and 30 hrs. TRUE (minimum diameter for 6 in^2 area is 2.8 inches)

ICPR ANALYSIS
(Mean Annual, 10-year, and 25-year Storm Events)

```
=====
==== Basins =====
=====
```

```

Name: DR-1                      Node: pond 1                      Status: Onsite
Group: BASE                     Type: SCS Unit Hydrograph CN

Unit Hydrograph: Uh484          Peaking Factor: 484.0
Rainfall File: Flmod           Storm Duration(hrs): 24.00
Rainfall Amount(in): 9.500     Time of Conc(min): 15.00
Area(ac): 2.630               Time Shift(hrs): 0.00
Curve Number: 89.30          Max Allowable Q(cfs): 999999.000
DCIA(%): 57.00
```

```
-----
Name: pre-1                      Node: BNDY                      Status: Onsite
Group: BASE                     Type: SCS Unit Hydrograph CN

Unit Hydrograph: Uh323          Peaking Factor: 323.0
Rainfall File: Flmod           Storm Duration(hrs): 24.00
Rainfall Amount(in): 9.500     Time of Conc(min): 14.00
Area(ac): 2.630               Time Shift(hrs): 0.00
Curve Number: 74.00          Max Allowable Q(cfs): 999999.000
DCIA(%): 0.00
```

```
=====
==== Nodes =====
=====
```

```

Name: BNDY                      Base Flow(cfs): 0.000          Init Stage(ft): 12.000
Group: BASE                     Warn Stage(ft): 14.000
Type: Time/Stage
```

Time(hrs)	Stage(ft)
0.00	12.000
12.00	13.000
24.00	14.000

```
-----
Name: pond 1                    Base Flow(cfs): 0.000          Init Stage(ft): 12.500
Group: BASE                     Warn Stage(ft): 15.000
Type: Stage/Area
```

Stage(ft)	Area(ac)
5.000	0.0800
6.000	0.1000
7.000	0.1200
8.000	0.1500
9.000	0.1800
10.000	0.2100
11.000	0.2300
12.000	0.2600
13.000	0.2800
14.000	0.3100
15.000	0.3300


```
=====
==== Drop Structures =====
=====
```

Name: S-OUT-1	From Node: pond 1	Length(ft): 15.00
Group: BASE	To Node: BNDY	Count: 1

UPSTREAM	DOWNSTREAM	Friction Equation: Automatic
Geometry: Circular	Circular	Solution Algorithm: Most Restrictive
Span(in): 18.00	18.00	Flow: Both
Rise(in): 18.00	18.00	Entrance Loss Coef: 0.000
Invert(ft): 12.500	12.400	Exit Loss Coef: 1.000
Manning's N: 0.013000	0.013000	Outlet Ctrl Spec: Use dc or tw
Top Clip(in): 0.000	0.000	Inlet Ctrl Spec: Use dc
Bot Clip(in): 0.000	0.000	Solution Incs: 10

Upstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

Downstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

*** Weir 1 of 2 for Drop Structure S-OUT-1 ***

Count: 1	Bottom Clip(in): 0.000
Type: Horizontal	Top Clip(in): 0.000
Flow: Both	Weir Disc Coef: 3.200
Geometry: Rectangular	Orifice Disc Coef: 0.600
Span(in): 30.00	Invert(ft): 13.500
Rise(in): 12.00	Control Elev(ft): 13.500

TABLE

*** Weir 2 of 2 for Drop Structure S-OUT-1 ***

Count: 1	Bottom Clip(in): 0.000
Type: Vertical: Mavis	Top Clip(in): 0.000
Flow: Both	Weir Disc Coef: 3.200
Geometry: Circular	Orifice Disc Coef: 0.600
Span(in): 2.80	Invert(ft): 12.500
Rise(in): 2.80	Control Elev(ft): 12.500

TABLE

```
=====
==== Hydrology Simulations =====
=====
```

Name: 10-year
Filename: X:\CIVIL\meek\Beaches Derm\civil\Drainage\ICPR\10year.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Flmod
Rainfall Amount(in): 7.25

Time(hrs)	Print Inc(min)
-----	-----
30.000	5.00

Name: 25-year
Filename: X:\CIVIL\meek\Beaches Derm\civil\Drainage\ICPR\25-year.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Flmod
Rainfall Amount(in): 9.50

Time(hrs)	Print Inc(min)
30.000	5.00

Name: mean
Filename: X:\CIVIL\meek\Beaches Derm\civil\Drainage\ICPR\mean.R32
Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: FLMOD
Rainfall Amount(in): 5.40

Time(hrs)	Print Inc(min)
30.000	5.00

==== Routing Simulations =====

Name: 10-year Hydrology Sim: 10-year
Filename: X:\CIVIL\meek\Beaches Derm\civil\Drainage\ICPR\10year.I32
Execute: Yes Restart: No Patch: No
Alternative: No
Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 0.000
Start Time(hrs): 0.000 End Time(hrs): 30.00
Min Calc Time(sec): 0.2500 Max Calc Time(sec): 300.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
11.000	30.000
16.000	5.000
24.000	15.000
30.000	30.000

Group	Run
BASE	Yes

Name: 25-year Hydrology Sim: 25-year
Filename: X:\CIVIL\meek\Beaches Derm\civil\Drainage\ICPR\25-year.I32
Execute: Yes Restart: No Patch: No
Alternative: No
Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 0.000
Start Time(hrs): 0.000 End Time(hrs): 30.00
Min Calc Time(sec): 0.2500 Max Calc Time(sec): 300.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
11.000	30.000
16.000	5.000
24.000	15.000
30.000	30.000

Group	Run

BASE Yes

Name: mean Hydrology Sim: mean
Filename: X:\CIVIL\meek\Beaches Derm\civil\Drainage\ICPR\mean.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 0.000
Start Time(hrs): 0.000 End Time(hrs): 30.00
Min Calc Time(sec): 0.2500 Max Calc Time(sec): 300.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
11.000	30.000
16.000	5.000
24.000	15.000
30.000	30.000

Group	Run
-----	----
BASE	Yes

Nodes

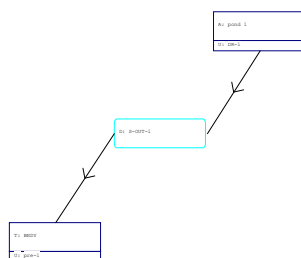
A Stage/Area
V Stage/Volume
T Time/Stage
M Manhole

Basins

O Overland Flow
U SCS Unit CN
S SBUH CN
Y SCS Unit GA
Z SBUH GA

Links

P Pipe
W Weir
C Channel
D Drop Structure
B Bridge
R Rating Curve
H Breach
E Percolation
F Filter
X Exfil Trench



Simulation	Basin	Group	Time Max hrs	Flow Max cfs	Volume in	Volume ft3
10-year	DR-1	BASE	12.03	12.728	6.641	63401.484
10-year	pre-1	BASE	12.07	7.779	4.262	40686.554
25-year	DR-1	BASE	12.03	16.864	8.872	84699.077
25-year	pre-1	BASE	12.07	11.423	6.288	60031.343
mean	DR-1	BASE	12.03	9.313	4.815	45971.915
mean	pre-1	BASE	12.07	4.879	2.688	25660.251

Name	Group	Simulation	Max Time Flow hrs	Max Flow cfs	Max Delta Q cfs	Max Time US Stage hrs	Max US Stage ft	Max Time DS Stage hrs	Max DS Stage ft
S-OUT-1	BASE	10-year	12.33	6.200	0.191	12.34	14.471	24.03	14.000
S-OUT-1	BASE	25-year	12.36	7.703	0.229	12.36	14.898	24.05	14.000
S-OUT-1	BASE	mean	12.32	4.782	0.147	12.32	14.077	24.02	14.000

Name	Group	Simulation	Max Time Stage hrs	Max Stage ft	Warning Stage ft	Max Delta Stage ft	Max Surf Area ft2	Max Time Inflow hrs	Max Inflow cfs	Max Time Outflow hrs	Max Outflow cfs
BNDY	BASE	10-year	24.03	14.000	14.000	0.0069	0	12.08	13.215	0.00	0.000
pond 1	BASE	10-year	12.34	14.471	15.000	-0.0050	13914	12.00	12.392	12.33	6.200
BNDY	BASE	25-year	24.05	14.000	14.000	0.0069	0	12.08	18.042	0.00	0.000
pond 1	BASE	25-year	12.36	14.898	15.000	0.0050	14286	12.00	16.428	12.36	7.703
BNDY	BASE	mean	24.02	14.000	14.000	0.0069	0	12.08	9.162	0.00	0.000
pond 1	BASE	mean	12.32	14.077	15.000	-0.0050	13571	12.08	9.076	12.32	4.782

STORMWATER PIPE CALCULATIONS

Gillette & Associates, Inc.

PROJ #:
DATE: 05/10/22

STORM SEWER TABULATION FORM
PROJECT: Beaches Dermatology - F.B.
BY: AG
SHEET 1 OF 2

COUNTY: Nassau

MINOR LOSSES HAVE BEEN CONSIDERED

STRUCT. NO.	STRUCT. TYPE	LINE TYPE	LINE LENGTH (FT)	DRAINAGE AREA (ACRES)		SUBTOTAL CA	TIME OF CONC. (MIN)	TIME OF FLOW IN SECTION (MIN)	i (IN/HR)	TOTAL CA	TOTAL RUNOFF (CFS)	INLET ELEV. (FT)	H.G. ELEV.			DIA. (IN.)	SLOPE (%)	V (FPS)	CAPACITY (CFS)	NOTES & REMARKS			
				C=	0.90								CROWN ELEV.										
													UPPER END	LOWER END	FALL (FT.)								
																					INVERT ELEV.		
S-1	C.B.	HDPE	126	0.20	0.20	0.18	10.00	0.70	7.01	0.18	1.26	15.00	14.95	14.92	0.03	18	0.01	0.7	1.3				
S-2													12.75	12.43	0.32								
													11.25	10.93	0.32		0.25	3.0	5.3				
S-2	C.B.	HDPE	105	0.37	0.71	0.64	10.70	0.58	6.84	0.64	4.37	15.00	14.92	14.83	0.09	24	0.04	1.4	4.4				
S-3													12.43	12.25	0.18								
S-3	C.B.	HDPE	51	0.21	0.92	0.83	11.28	0.28	6.71	0.83	5.56	14.90	14.83	14.73	0.10	24	0.06	1.8	5.6				
S-4													12.25	12.16	0.09								
													10.25	10.16	0.09		0.17	3.0	9.4				
S-4	MH	HDPE	64	0.17	1.09	0.98	11.56	0.36	6.66	0.98	6.53	14.80	14.73	14.67	0.06	30	0.03	1.3	6.5				
S-5													12.16	12.08	0.08								
S-5	C.B.	HDPE	84	0.17	1.26	1.13	11.92	0.47	6.58	1.13	7.46	14.70	9.66	9.58	0.08	30	0.13	3.0	14.7				
S-6													14.67	14.58	0.09		0.03	1.5	7.5				
S-6	C.B.	HDPE	79	0.20	1.46	1.31	12.39	0.44	6.49	1.31	8.52	14.60	12.08	11.97	0.11	30	0.13	3.0	14.7				
S-7													9.58	9.47	0.11								
S-7	MH	HDPE	33	0.00	1.46	1.31	12.83	0.18	6.40	1.31	8.41	14.50	14.58	14.48	0.10	30	0.04	1.7	8.5				
S-8													11.97	11.87	0.10								
													9.47	9.37	0.10		0.13	3.0	14.7				
													14.48	14.40	0.08		0.04	1.7	8.4				
													11.87	11.82	0.04								
													9.37	9.32	0.04		0.13	3.0	14.7				
																				Tailwater 14.40			
S-9	C.B.	HDPE	93	0.14	0.14	0.13	10.70	0.52	6.84	0.13	0.86	15.00	14.93	14.92	0.01	18	0.01	0.5	0.9				
S-2													12.75	12.51	0.24								
													11.25	11.01	0.24		0.25	3.0	5.3				
																				Tailwater 14.92			
																				(utilizes S-2 hydraulic grade)			
S-10	C.B.	HDPE	63	0.17	0.17	0.15	11.56	0.35	6.66	0.15	1.02	14.90	14.75	14.73	0.02	18	0.01	0.6	1.0				
S-4													12.65	12.49	0.16								
													11.15	10.99	0.16		0.25	3.0	5.3				
																				Tailwater 14.73			
																				(utilizes S-4 hydraulic grade)			

GEOTECHNICAL REPORT



OF JAX, INC.

BEACHES DERMATOLOGY

103B Solano Road
Ponte Vedra Beach, Florida 32082

November 16, 2021

Attn: Ms. Summer Zerbe

Re: Report of Preliminary Geotechnical Exploration & Engineering Evaluation
Proposed Beaches Dermatology - Amelia Island Parkway
Fernandina Beach, Florida
AGES Job No. J20090, Report No. 001

Dear Ms. Zerbe:

AGES OF JAX (AGES), Inc. has completed a preliminary geotechnical exploration of the subsurface conditions beneath the building and pavement construction areas for the referenced project. Our services were performed in general accordance with our Proposal No. 21JP8989 dated October 5, 2021. Formal authorization of our services was provided by Ms. Summer Zerbe of Beaches Dermatology on October 11, 2021. This report includes: (1) A brief description of the planned construction, (2) A summary of the field exploration and laboratory testing performed and results that were obtained, (3) Preliminary building structure foundation design and site preparation recommendations, (4) Guideline pavement design and construction recommendations, (5) Preliminary evaluation of the suitability of potential borrow soils located in possible pond areas at the site, and (6) Soil parameters for use in storm-water pond design and permitting.

We appreciate the opportunity to be of service as the geotechnical consultant during this phase of your project. Should you have any questions concerning this report or if we may be of any further service to you, please feel free to contact us.

Very truly yours,



AGES of JAX, INC.

Timothy R. Wheeler, P.E.
Principal Engineer/Vice-President
Registered, Florida No. 42802

TRW/trw/J20090.wpd

Attachments

Email Distribution: Addressee (1); Gillette & Associates, Inc. - Mr. Asa Gillette, P.E. (1)

AGES of JAX, Inc.

P O Box 24008, Jacksonville FL 32241-4008
9556 Historic Kings Road South, Suite 201 ● Jacksonville, Florida 32257-2010
(904) 886-0766 ● (904) 880-5190 (FAX)



OF JAX, INC.

**REPORT OF PRELIMINARY GEOTECHNICAL EXPLORATION
& ENGINEERING EVALUATION
PROPOSED BEACHES DERMATOLOGY
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA
AGES JOB NO. J20090, REPORT NO. 001**

FOR

**BEACHES DERMATOLOGY
103B SOLANO ROAD
PONTE VEDRA BEACH, FLORIDA 32082**

ATTN: MS. SUMMER ZERBE

NOVEMBER 16, 2021

AGES of JAX, Inc.

**P O Box 24008, Jacksonville FL 32241-4008
9556 Historic Kings Road South, Suite 201 ● Jacksonville, Florida 32257-2010
(904) 886-0766 ● (904) 880-5190 (FAX)**

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REPORT ATTACHMENTS

1.0 PROJECT BACKGROUND INFORMATION

Project information has been provided by Ms. Summer Zerbe of Beaches Dermatology during recent conversations and Email correspondence. In Email correspondence dated October 4, 2021, we were provided with: (1) A Boundary & Topographic Survey, prepared by Manzie & Drake, dated 9/16/21, and (2) Two Site Plans, designated as Option 1 and Option 2, both dated March 15, 2021, prepared by Gillette & Associates, Inc. which presented the site boundaries, existing site features, proposed building layout, planned parking lot/driveway configuration and proposed storm-water management facility locations.

Based upon the information provided, we understand that the proposed development will be located at the northwest corner of Amelia Island Parkway and Amelia Road in Fernandina Beach, Florida. We have assumed that the proposed buildings will be one(1) story wood and/or concrete masonry block framed structures supported by shallow foundation systems. The parking areas will be constructed using asphaltic concrete pavement. We also understand that Option 1 will utilize a single wet pond design for the storm-water runoff storage and treatment which will be located in the western end of the site and Option 2 will include two(2) dry pond design storm-water management facilities in the eastern end of the site. For the purposes of this report, we have been informed that Option 1 using one(1) wet pond will be used as the storm-water management facility.

Detailed structural loading information has not been provided at this time. However, we have assumed maximum individual column loads of 50 kips, maximum wall loads on the order of 2 klf and total floor slab loads of 150 pounds per square foot or less. The proposed building finish floor topography data has not been provided. However, we have estimated that two feet or less of elevating fill will be added in the building and pavement construction areas.

2.0 PRELIMINARY GEOTECHNICAL EXPLORATION

2.1 Purpose of Exploration

The objective of this preliminary geotechnical exploration was to obtain site and subsurface data for use in: (1) The evaluation of the site with respect to the proposed construction, (2) Development of preliminary foundation design and site preparation recommendations, (3) Development of guideline pavement design and construction recommendations, (4) Evaluation of storm-water management facility fill/backfill material suitability, and (5) The design and permitting of the storm-water management system. The preliminary geotechnical exploration therefore consisted of field exploratory borings and a geotechnical engineering analysis of the collected test data.

2.2 Subsurface Testing

The field testing program was conducted during the period of October 29, 2021 through November 13, 2021 and consisted of five(5) 20-foot deep penetration test borings (B1 to B5) in the proposed building construction areas, five(5) 6-foot deep auger borings (A1 to A5) within the proposed roadway pavement construction areas and one(1) 15-foot auger boring within the proposed storm-water management facility construction areas(LA1). The borings were performed at preliminary locations selected by our principal engineer.

The field testing locations were established by our personnel using hand held GPS based measuring equipment devices which has a horizontal accuracy of 10 to 15 feet. The boring locations are shown on the attached Field Exploration Plan and should be considered approximate. The ground surface elevations at our boring locations were not provided to us.

The attached Test Boring Records and Auger Boring Records present the descriptions of the subsurface soils encountered, the groundwater levels encountered and the penetration resistance values recorded when drilling and sampling the test borings. The stratification lines and depth designations on the boring records represent the approximate boundary between the various soils encountered, as determined in the field by our drillers, and the transition from one strata to the next should be considered approximate. A brief discussion of the drilling, sampling, and field testing techniques used during the geotechnical exploration is provided in the attached Field Exploration Procedures section.

Representative soil samples obtained during our field exploration were returned to our laboratory. The samples were examined by a geotechnical engineer and were visually classified in general accordance with ASTM D 2488 (Unified Soil Classification System) and AASHTO M-145 (AASHTO Soil Classification System).

3.0 GEOTECHNICAL FINDINGS

3.1 Site Conditions

The subject site was visited on several occasions during the period of October 29, 2021 to November 13, 2021 by **AGES** personnel to identify site conditions which could impact our site preparation and roadway design recommendations. The proposed development will be located at the northwest corner of Amelia Island Parkway and Amelia Road in Fernandina Beach, Florida. The site is moderately to significantly wooded and undeveloped. The site topography was found to be relatively level and lower than the adjacent roadway. Standing water was not observed at the time of our visit(s). Surface soils consisted of root matt, topsoil and slightly silty to silty fine sands.

3.2 Subsurface Conditions

The subsurface conditions outlined below highlight the major subsurface stratifications encountered during our geotechnical exploration of the site. When reviewing the boring records and the subsurface conditions outlined below, it should be understood that the subsurface conditions will vary across the proposed construction area and between the boring locations.

3.2.1 Soil Conditions - The penetration test borings (B1 to B5) typically encountered alternating strata of very loose to dense brown to dark brown, grey, grey brown and dark grey brown fine sands(SP), slightly silty fine sands (SP-SM) and silty fine sands(SM) which extended to the maximum boring termination depth of 20 feet. Penetration resistance values within this stratum ranged from 2 to 31 blows/foot.

The roadway auger borings (A1 to A5) typically encountered alternating strata of brown to dark brown, grey brown, dark grey brown and grey fine sands(SP/A-3) and slightly silty fine sands (SP-SM/A-3) which extended to the maximum boring termination depth of 6 feet.

The storm-water management facility lake auger boring(LA1) typically encountered alternating strata of dark grey brown and brown slightly silty fine sands (SP-SM/A-3) which extended to the maximum boring termination depth of 15 feet.

Slightly silty fine sands with some large wood pieces(SP-SM/Pt/A-8) were encountered at boring B2(8.0'- 12.0').

3.2.2 Groundwater Conditions - The groundwater level depths encountered at the time of drilling ranged from depths of approximately 1.5 to 2.5 feet below the ground surface. Based upon soil coloration, review of the USDA Websoil Survey and other considerations, we estimate that the seasonal high groundwater level, as described on the Auger Boring Records for the lake auger boring

location, will vary from depths of 1.0 to 1.5 feet. The groundwater level should be expected to fluctuate due to seasonal climatic variations, changes in surface water runoff patterns across the site, construction activity and the subsequent development as planned, and other interrelated factors. Since groundwater variations are anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based on the assumption that variations will occur.

4.0 PRELIMINARY GEOTECHNICAL ENGINEERING EVALUATION

4.1 Basis of Evaluation & Recommendations

Our geotechnical engineering evaluation of the site and subsurface conditions with respect to the planned construction, and our recommendations for foundation design and site preparation/earthwork construction, are based upon: (1) Our site observations; (2) The field and laboratory test data obtained during this geotechnical exploration of the site; and (3) our understanding of the project information and structural loading conditions as presented in this report. If the structural information or the Overall Site Plan becomes finalized, please contact us so we may review our recommendations. If any site or subsurface conditions are encountered during construction which appear to deviate from data obtained during this geotechnical exploration (as documented herein), please contact us so that we may visit the site, observe the differing conditions, and evaluate the new information relative to our geotechnical evaluation and recommendations.

4.2 Preliminary Geotechnical Engineering Evaluation

Assuming that the site preparation procedures outlined in this report are implemented, the proposed one story wood framed and/or concrete block structures can be supported upon properly designed and constructed conventional shallow foundation systems. Shallow foundation systems may be designed using an allowable soil bearing pressure of 2,500 psf. Provided that the design and construction recommendations outlined subsequently are implemented, we estimate that total structural settlement due to fill and structural loads should be on the order of one inch or less. In addition, post construction differential settlements should be within tolerable magnitudes. The settlements should occur concurrently with construction due to the generally sandy nature of the soils encountered. Site preparation necessary for proper foundation performance includes items as listed below and on the following pages:

- * Dependent upon the groundwater levels encountered at the time of construction, temporary de-watering will be required in almost areas of the site. The groundwater should be controlled at all times at a depth of at least two feet below the construction level. Groundwater draw-downs on the order of one to two feet, if required, can probably be best achieved using temporary perimeter drainage ditches, four to five feet deep, which are graded: (1) to a positive gravity outfall away from the site; or (2) to sumps where the collected groundwater and surface water runoff can be removed by pumping. It should be anticipated that "sock" pipe and well point systems will be needed for deeper groundwater draw-downs if necessary;
- * Clearing any conflicting wooded areas, stripping any existing vegetation and removing surficial topsoil from within the proposed building and pavement areas;
- * Excavate the residential structure construction areas to remove slightly silty fine sands with some large wood pieces which were encountered at boring B2(8.0'- 12.0'). Excavation depths may be deeper in some

areas. The removal operation should be monitored by a qualified senior engineering technician or engineer to verify that the deleterious organic materials have been completely removed.

- * Proof-rolling the site with heavy rubber tired construction equipment to identify areas requiring over-excavation and replacement of surficial soft unsuitable soils.
- * Compaction of the exposed granular soils to densities equivalent to 95 percent of the Modified Proctor maximum dry density (ASTM D 1557), or placement of an initial lift of structural fill in areas of the site in which clayey soils are exposed after the clearing and stripping operations.
- * Placement and compaction of any required structural fill to densities of at least 95 percent of the Modified Proctor maximum dry density (ASTM D 1557) within the building and pavement areas and to densities of at least 98 percent of the Modified Proctor maximum dry density within the upper 12 inches of subgrade soils beneath the pavement areas.

4.3 Roadway Pavement Evaluation

The soil conditions appear favorable for flexible pavements constructed in general accordance with local and Florida Department of Transportation (FDOT) standards. The primary geotechnical factors affecting roadway design, construction and performance are: (1) The presence of near surface very loose to loose fine sands which will require compaction and stabilization, and (2) Significant potential for elevated seasonal high groundwater conditions depending on the roadway location and the planned roadway grading.

We typically recommend that the pavement base course bottom elevation be set at least 24 inches above the anticipated post development seasonal high groundwater level and 30 inches above weak or compressible clayey strata. This separation may be achieved by a combination of under cutting, filling or installing permanent surface drainage improvements. Alternatively, in areas where groundwater is anticipated to occur within 24 inches of the bottom of a flexible pavement base course and filling is not a cost effective or technically desirable alternative, a properly designed and constructed under-drain system can be used to maintain the recommended separation between the pavement base course and the groundwater level. We highly recommend that the civil grading plans be reviewed to evaluate the need for an under-drain system.

4.4 Stormwater Management Pond Evaluation

The development will include construction of one(1) wet detention storm-water management facility which could be excavated to approximate depths of 12 to 15 feet below the existing ground surface. All of the borings mentioned in Section 2 above were conducted to provide subsurface soil data for use in: (1) Design and permitting of the pond (i.e. current and estimated seasonal high groundwater levels), and (2) Evaluation of the suitability of the pond/borrow areas for your use as fill/backfill.

4.3.1 Seasonal High Groundwater Estimates: Seasonal high groundwater levels are the groundwater levels encountered during an average wet season. These levels can estimated based upon current groundwater levels, soil staining observations, evaluation of the controlling natural and man-made water features and other considerations. Based upon a review of the borings conducted

and a review of the USDA Websoil Survey Data, we estimate that the seasonal current groundwater elevation in the planned pond/borrow areas will vary from 1.0 to 1.5 feet. The estimated seasonal high groundwater levels at each boring location are included on the Auger Boring Records.

4.3.2 Borrow Source Evaluation The evaluation of borrow soil suitability depends upon: (1) The planned soil usage, (2) Proposed construction methods, (3) The time of year during which site work construction will occur, (4) Economic considerations, and (5) Other factors. From a building pad perspective, we typically recommend that all fill and backfill soils consist of clean sands (fine sand to slightly silty fine sand) containing less than 12% fines (SP and SP-SM). Higher fines content sandy soils including: silty sands and clayey sands (SM and SC) can also be used in building pad development. However, these soils are more moisture sensitive and re-compaction in Florida during the wet summer months and/or use of the fill on wet sites will significantly increase re-compaction effort and could result in construction delays.

Based upon a review of the boring LA1, it is our opinion that the slightly silty fine sands(SP-SM, A-3) encountered in the upper 15 feet of profile will be usable as structural fill/backfill. During the pond excavation process, any encountered silty fine sands will be blended with the sandier soils which will help reduce their fines content to acceptable levels.

5.0 FOUNDATION DESIGN & SITE PREPARATION RECOMMENDATIONS

5.1 Foundation Design Recommendations

5.1.1 Foundation Type - We consider a conventional shallow foundation system applicable for this project if the site preparation recommendations presented in section 5.2 of this report are performed.

5.1.2 Bearing Pressure & Depth - The maximum allowable soil bearing pressure for use in shallow foundation design should not exceed 2,500 psf. The foundations should be designed based upon the maximum load which could be imposed by all loading conditions. The exterior footings should bear at a depth of at least 12 inches below the exterior final grades and the interior footings should also bear at least 12 inches below final grades. These minimum bearing depths should provide the necessary confinement for the soils at the foundation bearing levels.

5.1.3 Foundation Size - The minimum widths recommended for isolated column and continuous wall footings are 24 and 18 inches, respectively. Even though the maximum allowable soil bearing pressure may not be achieved, these minimum width recommendations should control the minimum size of the foundations.

5.1.4 Bearing Material - The foundations may bear in either the compacted suitable natural granular soils or compacted structural backfill or fill. The bearing level soils, after compaction, should exhibit densities equivalent to 95 percent of the modified Proctor maximum dry density (ASTM D 1557) to a depth of at least one-foot below the foundation bearing levels.

5.2 Site Preparation Recommendations

5.2.1 Temporary Groundwater Control - During our geotechnical exploration, the groundwater level was encountered at depths of 1.5 to 2.5 feet below the existing site grades. Dependent upon the groundwater levels encountered at the time of construction, temporary de-watering will be required in most areas of the site. The groundwater should be controlled at all times at a depth of at least two feet below the construction level. Groundwater drawdowns on the order of one to two feet, if required, can probably be best achieved using temporary perimeter drainage ditches, four to five feet deep, which

are graded: (1) to a positive gravity outfall away from the site; or (2) to sumps where the collected groundwater and surface water runoff can be removed by pumping. It should be anticipated that "sock" pipe and well point systems will be needed for deeper groundwater draw-downs as necessary.

5.2.2 Unsuitable Organic Soil Removal and Backfill Placement - Initial site preparation should consist of the removal of the slightly silty fine sands with some large wood pieces which were encountered at boring B2(8.0'- 12.0'). Excavation depths may be deeper in some areas. The removals should then be performed within and to a distance of at least five feet beyond the perimeter of the planned building construction areas. The removal operation should be monitored by a qualified senior engineering technician or engineer to verify that the deleterious organic materials have been completely removed. The perimeter areas should be graded to help direct surface water runoff away from the planned construction areas. Over-excavation should be performed using a backhoe to reduce the potential for disturbance to and pumping of soils that exist below the recommended level of over-excavation. Disturbance of these soils may result in the need for further over-excavation.

Prior to performing compaction operations as described below, we recommend an initial 12 inch lift of relatively dry, clean structural backfill be placed above soils exposed by the clearing/stripping or over-excavation operations to serve as a working platform for subsequent fill placement. The initial lift of fill should then be compacted with light walk behind vibratory sleds or rollers until densities equivalent to at least 95 percent of the Modified Proctor (ASTM D1557) maximum density are achieved in the upper 6-inches. Subsequent backfill may be placed in 4 to 6-inch loose lifts and compacted to at least 95 percent of the Modified Proctor maximum dry density using the light weight equipment. After backfilling to a significant height (3 to 4 feet) above the water table, backfill lifts can be increased to 12-inches in thickness.

5.2.3 Proof-Rolling - After the above clearing/stripping and grading operations, we recommend that the exposed soils be proof-rolled with a heavily loaded dump truck or other heavy rubber-tired construction vehicle. Any areas of loose or "pumping" soils should be over-excavated and backfilled with properly compacted structural fill. Densities equivalent to 95 percent of the Modified Proctor maximum dry density (ASTM D 1557) should be achieved in the backfilled soils.

5.2.4 Site & Fill Compaction - After completing any initial site preparation which includes unsuitable soil replacement and backfilling, the exposed sandy soils should be compacted to densities of at least 95 percent of the Modified Proctor maximum dry density using a vibratory drum roller which has a minimum at-drum weight on the order of 4 to 5 tons and a minimum drum diameter on the order of three to four feet. It is recommended the very loose to loose fine sands in the proposed building construction areas be densified to a depth of at least 2 feet below the stripped surface in the proposed building construction areas. The large roller will be required to improve the density of the upper approximate 2 feet of loose sands below the stripped surface in the building areas.

Once the site compaction verification has been performed and the site compaction effort has been deemed to be adequate, the structural fill required to raise the site to the planned finished grades may then be placed in loose lifts not exceeding 12 inches in thickness, and should be compacted to densities as recommended above. Loose lifts of backfill and fill should not exceed eight inches in areas of the site in which lightweight vibratory equipment is used. Structural backfill and fill is defined as a non-plastic, inorganic, granular soil containing less than 10 percent material passing the No. 200 mesh sieve (i.e. a relatively clean sand).

5.2.5 Disturbed Soil Conditions - Should the near-surface soils and/or structural fill material experience "pumping" and subsequent soil strength loss during site work construction, work upon these areas should be immediately terminated and: (1) the disturbed soils removed and backfilled with "dry" structural fill soils, (i.e. percent water content on the order of five to 10 percent) which are then

compacted; or (2) the excess moisture content within the disturbed soils allowed to dissipate before recompacting. Furthermore, the groundwater table should be checked and controlled as necessary to help insure proper draw-down of any high groundwater conditions that may be causing the "pumping" conditions during compaction or construction activity upon these soils.

5.2.6 Foundation Areas - After placement and compaction of all structural fill in the building areas, the foundations may be excavated to their planned bearing levels using a "smoothed" bucket backhoe. All fine sandy soils existing at the bearing level should be compacted to densities equivalent to 95 percent of the modified Proctor maximum dry density to a depth of at least 12 inches below the bearing level. For confined footing areas compaction of the fine sandy bearing level soils can probably be best achieved using lightweight, walk-behind sleds, rollers, or tampers having a total weight of at least 100 pounds.

5.3 Additional Field Exploration

Slightly silty fine sands with some large wood pieces were encountered at boring B2(8.0'- 12.0'). We recommend a series of backhoe-excavated test pits be performed in the vicinity of the appropriate borings to delineate the vertical and lateral extent of these soils and to allow a better determination of the content of these soils and, therefore, to better evaluate the need for over-excavation beneath the construction areas. **AGES** can perform these additional services if requested.

6.0 GUIDELINE PAVEMENT DESIGN AND CONSTRUCTION RECOMMENDATIONS

6.1 General

The subsurface conditions at this site appear favorable for use of flexible asphalt pavement systems throughout the site. Guideline pavement design and construction recommendations applicable to this project are presented below. The pavement thickness recommendations and construction recommendations presented in this report are based upon our past experience on similar paving projects. Detailed flexible pavement design calculations have not been performed since detailed traffic loading information is unavailable at this time. We request that **AGES** be retained to review the final pavement design, paving and drainage plans and specifications to evaluate whether our guideline recommendations have been properly interpreted and implemented into the project design.

6.2 Guideline Pavement Design Recommendations

The recommended flexible pavement structure thicknesses for the subject project are provided in the following table.

PAVEMENT COURSE	Automobiles & Light Truck Traffic	Moderate Truck Traffic
Asphalt Wearing Surface(In.)*	1.5	2.0
Limerock Base Course (In.)**	6.0	8.0
Prepared Subgrade(In.)***	12.0	12.0

* Superpave 9.5 Asphaltic Concrete or Type S-III, as defined by the Florida DOT Standard Specifications for Road and Bridge Construction Manual.

** Florida Limerock, exhibiting an LBR Value of 100, and meeting the specifications outlined in the Florida DOT manual (Section 200).

*** Stabilized and compacted subgrade should exhibit a Maximum LBR value of at least 40. Subgrade stabilization should be performed in accordance with Section 160 of the standard FDOT specifications.

6.3 Guideline Pavement Construction Recommendations

6.3.1 Pavement Subgrade Preparation - Site preparation in the pavement areas should be performed in accordance with the guideline procedures outlined above, and should include: (1) Establishment of proper temporary groundwater control, (2) Clearing/stripping topsoil and surface vegetation, (3) Compaction of the exposed soils using a large vibratory drum roller, and (3) Placement/compaction of backfill and any required structural fill (if any) to densities equivalent to 95 percent of the Modified Proctor maximum dry density. In addition, the upper one foot of sub-grade in the pavement areas should be compacted to 98 percent of the Modified Proctor maximum dry density. Shell and or clay additives may be required to obtain the required design LBR's required. Also, the pavement structure should be designed to provide at least 24-inches of fine sands separating the limerock subgrade and any underlying clayey/silty soils. This will help to prevent "pumping" and reduce the chance of premature pavement failures which are caused by "perched" groundwater levels.

6.3.2 Limerock Base Course Preparation - The limerock base course should be placed and compacted to densities equivalent to 100 percent of the Modified Proctor maximum dry density (ASTM D 1557). The surface of the limerock base course material should be "primed" with any of the following cutback asphalt grades: RC-70, RC-250, MC-70, or MC-250. Approximately 0.15 gallons per square yard should be applied to the base course surface. Care should be exercised to prevent over-priming of the base course surface. Prior to asphalt placement, the prime coat should be fully set and cured. The specifications governing the placement of prime coats on pavement base course surfaces and sanding is outlined in Section 300 of the Florida DOT Standard Specifications for Road and Bridge Construction (1986).

The primed base course should be sanded if: (1) more than one day (24 hours) will elapse prior to laying the asphalt wearing surface and excess prime is not absorbed, or (2) construction equipment will operate over the unprotected primed base course. If sanding is required, approximately 10 pounds of sand per square yard of surface (i.e., about 1/4 inch of sand) should be spread over the primed base course. The sand surface coating should contain less than 10 percent fines (i.e., material passing the No. 200 sieve) and be composed of non-elastic, inorganic, granular soil.

6.3.3 Asphalt Preparation - The placement and compaction of an asphaltic concrete wearing surface should be performed in general accordance with specifications outlined in the Florida DOT Road and Bridge Construction manual. Prior to asphalt placement, the base course should be broomed to remove any excess sand and then tacked with a bituminous binder.

6.4 Permanent Groundwater Control Measures

Groundwater control is of utmost importance in maintaining the integrity of flexible pavement structures. In areas where groundwater is anticipated to exist within 24 inches of the bottom of the pavement base course, a properly designed and constructed under drain system will be necessary to maintain this 24-inch separation between the pavement base course and the groundwater level. After finalizing the civil grading plans, we would be pleased to review the plans and evaluate the need for an under drain system. If required, we can design an under drain system for this site.

7.0 QUALITY CONTROL TESTING GUIDELINES

Prior to initiating compaction operations, we recommend that representative samples of the structural fill material and exposed in-place fine sandy soils be collected and tested to determine their compaction and classification characteristics. The maximum dry density, optimum moisture content

gradation and plasticity characteristics should be determined. These tests are needed for compaction quality control of the structural fill and existing soils and to determine if the fill material is acceptable.

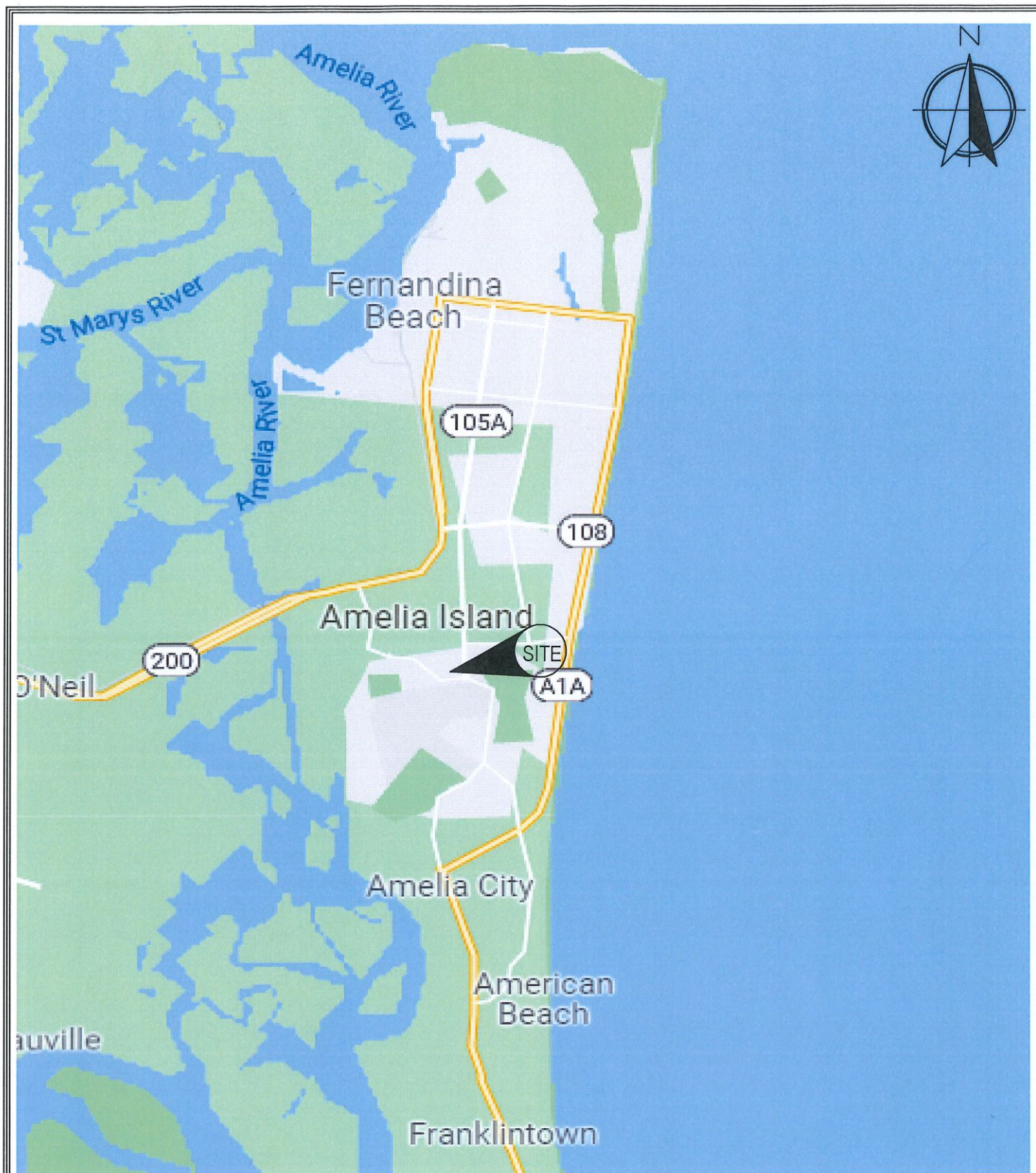
A representative number of field in-place density tests should be performed in the compacted existing soils and in each one foot lift of structural fill/backfill to confirm that the required degree of compaction has been obtained. At each test level, we recommend one density test be performed for every 5,000 square feet of building area (minimum of three locations). In-place density tests should also be performed at representative locations in the bearing level soils of the footings. We recommend that at least one density test be performed at a minimum of 25 percent of the isolated column footing bearing areas, and for every 100 linear feet of continuous footing bearing surface. In addition, in-place density tests should be performed at one location for every 10,000 square feet of pavement area (minimum of three locations).

8.0 CONSTRUCTION PLANS & SPECIFICATIONS REVIEW

We recommend that **AGES** be provided the opportunity to review the plans and the specifications prepared from the recommendations presented in this report. If necessary, we will suggest any modifications or additional work (including the performance of additional geotechnical engineering services) that may be required to verify that our recommendations have been properly interpreted and implemented. Our report has been written in a guideline recommendation format and is not appropriate for use as (or inclusion into) the specifications without being reworded in a specification type format. It is recommended that this report not be made part of the contract documents. However, it should be made available to prospective contractors for information purposes only.

DATA ATTACHMENTS

Site Location Map
Field Exploration Plan
Key to Soil Classification
Test Boring Records
Auger Boring Records
Field Exploration Procedures



REFERENCE:

Google.com



AGES of JAX Inc.
9556 Historic Kings Rd.
Jacksonville, Florida 32257

SITE LOCATION MAP

Beaches Dermatology
Fernandina Beach, Florida

DRAWN: JLK

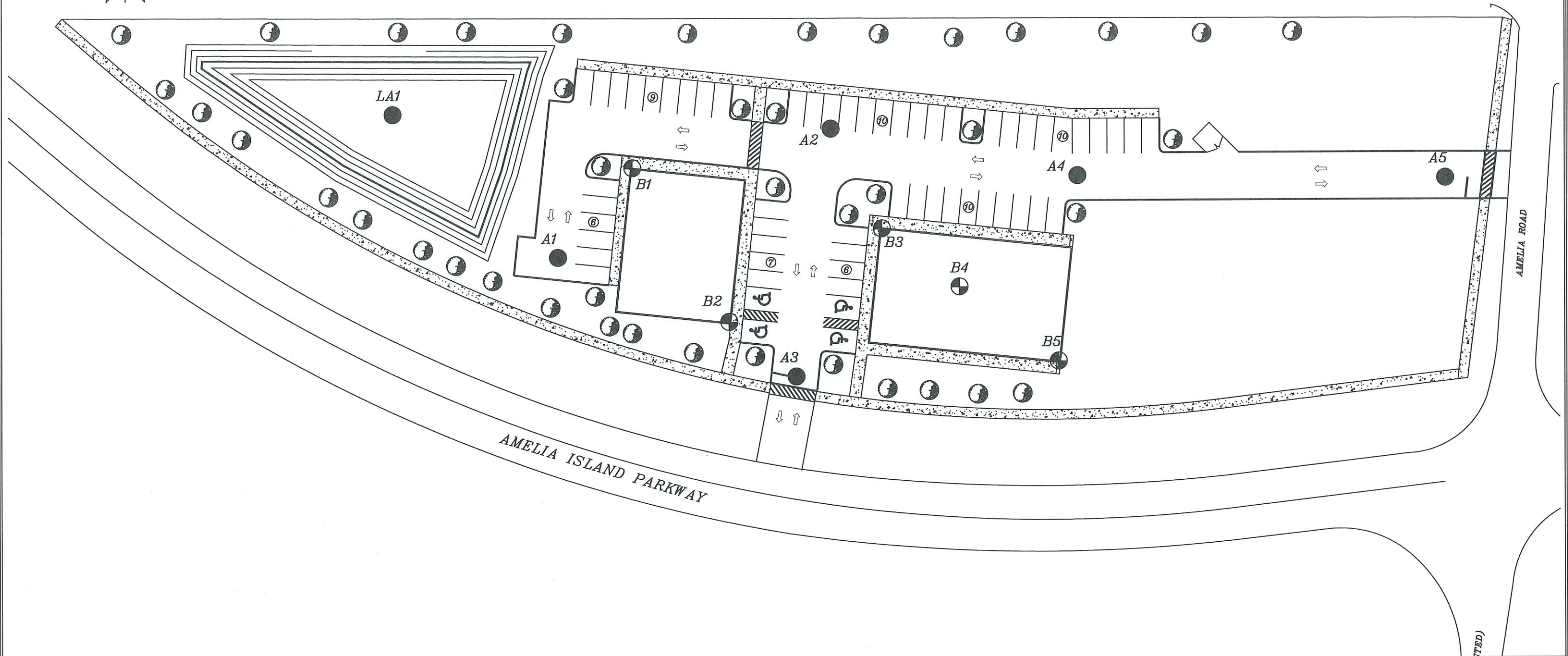
DATE: 11/11/21

SCALE:

CHECKED: TRW

JOB NO: J20090

NTS



LEGEND:

- ⊕ Penetration Test Boring Location (approximate)
- Auger Boring Location (approximate)

REFERENCE:

Preliminary Site Plan prepared by Gillette & Associates, dated 03/15/21.

NOTES:

The boring locations were established by AGES personnel using handheld GPS Locator Units. Boring locations as shown on this plan are approximate.



AGES of JAX Inc.
9556 Historic Kings Rd.
Jacksonville, Florida 32257

FIELD EXPLORATION PLAN

Beaches Dermatology
Fernandina Beach, Florida

DRAWN: JLK	DATE: 11/11/21	SCALE:
CHECKED: TRW	JOB NO: J20090	1"=50'

KEY TO SOIL CLASSIFICATION

CORRELATION OF N-VALUE WITH RELATIVE DENSITY & CONSISTENCY

SANDS AND GRAVEL	
NO. OF BLOWS, N*	RELATIVE DENSITY
0 - 4	VERY LOOSE
5 - 10	LOOSE
11 - 20	FIRM
21 - 30	VERY FIRM
31 - 50	DENSE
OVER 50	VERY DENSE

SILTS AND CLAYS	
NO. OF BLOWS, N*	CONSISTENCY
0 - 2	VERY SOFT
3 - 4	SOFT
5 - 8	FIRM
9 - 15	STIFF
16 - 30	VERY STIFF
31 - 50	HARD
OVER 50	VERY HARD

PARTICLE SIZE IDENTIFICATION (UNIFIED SOIL CLASSIFICATION SYSTEM)

CATEGORY	DIMENSIONS
Boulders	Diameter exceeds 12 inches
Cobbles	3 to 12 inches
Gravel	Coarse - 0.75 to 3 inches in diameter
Sand	Coarse - 2.0 mm to 4.76 mm diameter Medium - 0.42 mm to 2.0 mm diameter
Silt and Clay	Less than 0.074 mm (Invisible to the naked eye)

MODIFIERS

These modifiers provide our estimate of the amount of minor constituents (sand silt or clay size particles) in the soil sample.

PERCENTAGE OF MINOR CONSTITUENT	MODIFIERS
5% to 12%	Slightly Silty, Slightly Clayey, Slightly Sandy Silty, Clayey, Sandy Very Silty, Very Clayey, Very Sandy
12% to 30%	
30% to 50%	

These modifiers provide our estimate of the amount of other components in the soil sample.

APPROXIMATE CONTENT OF OTHER COMPONENTS (SHELL, GRAVEL, ETC.)	MODIFIERS	APPROXIMATE CONTENT OF ORGANIC COMPONENTS
0% to 5%	TRACE FEW SOME MANY	1% to 2%
5% to 12%		2% to 4%
12% to 30%		4% to 8%
30% to 50%		>8%



TEST BORING RECORD

JOB NO J20090

BORING NO B1

Sheet 1 of 1

Boring Begun 10/29/21

Boring Completed 10/29/21

Driller NW, AW & JM

Soil Inspected By T. Wheeler

Project Beaches Dermatology

Boring Location (See Field Exploration Plan)

Ground Elevation NA Datum NA

Groundwater Depth 2.0' Time: Drilling Date: 10/29/21

Lat/Long N 30.61895° / W 81.45882°

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST					
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT	
	0								
		Root Matt - 6"							
		VERY LOOSE Dark Brown Silty Fine SAND with Trace Roots (SM)	1	1	1	1	2		2
			2	3	3	3	3		6
		LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)							
	5		3	3	2	3	6		5
		DENSE Dark Brown Slightly Silty Fine SAND (SP-SM)	4	15	15	16	17		31
			5	7	11	12	21		23
	10								
		VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)							
			6	9	12	9			21
	15								
		FIRM Grey Fine SAND (SP)							
	20		7	7	8	6			14
		BORING TERMINATED							
	25								
	30								

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

J20090 B1



TEST BORING RECORD

JOB NO J20090

BORING NO B2
 Sheet 1 of 1
 Boring Begun 10/29/21
 Boring Completed 10/29/21
 Driller NW, AW, JW & JM
 Soil Inspected By T. Wheeler

Project Beaches Dermatology
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.5' Time: Drilling Date: 10/29/21
 Lat/Long N 30.61876° / W 81.45856°

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST					
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT	
	0								
		Root Matt - 6"							
		LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)	1	2	2	3	2		5
		VERY LOOSE Dark Brown Slightly Silty Fine SAND with Trace Roots (SP-SM)	2	3	3	1	3		4
	5	LOOSE Brown Slightly Silty Fine SAND with Trace Roots (SP-SM)	3	3	4	5	9		9
		VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)	4	7	10	12	13		22
			5	7	10	13	13		23
	10	VERY FIRM Brown Slightly Silty Fine SAND with Some Large Wood Pieces (SP-SM)							
	15	LOOSE Brown Slightly Silty Fine SAND (SP-SM)	6	4	3	2			5
		FIRM Grey Fine SAND (SP)							
	20		7	7	8	9			17
		BORING TERMINATED							
	25								
	30								

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.



TEST BORING RECORD

JOB NO J20090

Project Beaches Dermatology
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.5' Time: Drilling Date: 10/29/21
 Lat/Long N 30.61892° / W 81.45841°

BORING NO B3
 Sheet 1 of 1
 Boring Begun 10/29/21
 Boring Completed 10/29/21
 Driller NW, AW, JW & JM
 Soil Inspected By T. Wheeler

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST				BLOW COUNT
				BLOWS PER 6-IN. INTERVAL				
	0	VERY LOOSE Dark Grey Brown Fine SAND with Trace Roots (SP)	1	1	1	1	1	2
		VERY LOOSE Dark Brown Slightly Silty Fine SAND with Trace Roots (SP-SM)	2	2	1	2	2	3
	5	FIRM to VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)	3	3	6	8	7	14
			4	13	9	9	11	18
			5	9	12	12	13	24
	10	FIRM Brown Slightly Silty Fine SAND (SP-SM)	6	5	6	7		13
	15							
		FIRM Grey Brown Slightly Silty Fine SAND (SP-SM)	7	6	6	5		11
	20	BORING TERMINATED						
	25							
	30							

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.



TEST BORING RECORD

JOB NO J20090

BORING NO B4
 Sheet 1 of 1
 Boring Begun 10/29/21
 Boring Completed 10/29/21
 Driller NW, AW, JW & JM
 Soil Inspected By T. Wheeler

Project Beaches Dermatology
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 1.5' Time: Drilling Date: 10/29/21
 Lat/Long N 30.61886° / W 81.45824°

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST					
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT	
	0								
		LOOSE Dark Grey Brown Fine SAND with Trace Roots (SP)	1	1	2	4	3		6
		LOOSE Dark Brown Slightly Silty Fine SAND (SP-SM)							
		LOOSE Brown Slightly Silty Fine SAND (SP-SM)	2	5	6	4	5		10
	5	FIRM Brown Slightly Silty Fine SAND with Trace Roots (SP-SM)	3	3	6	13	17		19
			4	15	13	15	15		28
		VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)	5	15	15	15	17		30
	10								
	15	LOOSE to FIRM Brown Slightly Silty Fine SAND (SP-SM)	6	3	4	6			10
	20		7	5	6	10			16
		BORING TERMINATED							
	25								
	30								

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.



TEST BORING RECORD

JOB NO J20090

Project Beaches Dermatology
 Boring Location (See Field Exploration Plan)
 Ground Elevation NA Datum NA
 Groundwater Depth 2.0' Time: Drilling Date: 10/29/21
 Lat/Long N 30.61870° / W 81.45806°

BORING NO B5
 Sheet 1 of 1
 Boring Begun 10/29/21
 Boring Completed 10/29/21
 Driller NW, AW, JW & JM
 Soil Inspected By T. Wheeler

ELEV. (FT)	DEPTH (FT)	MATERIAL DESCRIPTION (USCS CLASSIFICATION)	SAMPLE NO.	STANDARD PENETRATION TEST				
				BLOWS PER 6-IN. INTERVAL				BLOW COUNT
	0							
		LOOSE Dark Grey Brown Fine SAND with Trace Roots (SP)	1	2	3	5	5	8
		LOOSE Brown Slightly Silty Fine SAND (SP-SM)	2	3	4	5	5	9
	5		3	6	8	8	9	16
		FIRM to VERY FIRM Brown Slightly Silty Fine SAND (SP-SM)	4	11	13	13	14	26
			5	5	15	14	15	29
	10							
		LOOSE Brown Slightly Silty Fine SAND (SP-SM)	6	4	5	5		10
	15							
		FIRM Grey Slightly Silty Fine SAND (SP-SM)	7	9	7	6		13
	20	BORING TERMINATED						
	25							
	30							

REMARKS:

BORING & SAMPLING, ASTM D1586/CORE DRILLING, ASTM D213
 BLOW COUNT IS THE NUMBER OF BLOWS OF A 140 LB. HAMMER
 FALLING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.



AUGER BORING RECORDS
 Beaches Dermatology
 Fernandina Beach, Florida
 AGES Job No. J20090; Report No. 001

Date Performed: 10/29/21 to 11/13/21

Performed By: A. Wheeler/D. Anderson

Auger ID Lat/Long	Depth (Feet) ^a	SOIL DESCRIPTION (USCS / AASHTO CLASSIFICATION)
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Roadway Parking and Entrance Drive Pavement Areas

A1 N 30.61884° W 81.45888°	0.0 - 1.0	Grey Brown Slightly Silty Fine SAND (SP-SM/A-3)
	1.0 - 2.0	Dark Brown Slightly Silty Fine SAND (SP-SM/A-3)
	2.0 - 6.0	Brown Slightly Silty Fine SAND (SP-SM/A-3)
	A.B.T.b	GWLC= 2.0'
A2 N 30.61907° W 81.45884°	0.0 - 0.3	Topsoil - 3 Inches
	0.3 - 1.5	Dark Grey Slightly Silty Fine SAND (SP-SM/A-3)
	1.5 - 4.0	Dark Brown Slightly Silty Fine SAND (SP-SM/A-3)
	4.0 - 6.0	Brown Slightly Silty Fine SAND (SP-SM/A-3)
	A.B.T.b	GWLC= 2.5'
A3 N 30.618679° W 81.458491°	0.0 - 1.0	Grey Brown Slightly Silty Fine SAND (SP-SM/A-3)
	1.0 - 4.0	Brown Fine SAND (SP/A-3)
	4.0 - 6.0	Dark Brown Slightly Silty Fine SAND (SP-SM/A-3)
	A.B.T.b	GWLC= 2.0'
A4 N 30.618965° W 81.458029°	0.0 - 0.5	Dark Grey Slightly Silty Fine SAND w/Trace Small Roots (SP-SM/A-3)
	0.5 - 1.0	Brown Slightly Silty Fine SAND (SP-SM/A-3)
	1.0 - 2.5	Dk. Brown Cemented Slightly Silty Fine SAND (SP-SM/A-3)(Hardpan)
	2.5 - 6.0	Brown Slightly Silty Fine SAND (SP-SM/A-3)
	A.B.T.b	GWLC= 2.5'
A5 N 30.61895° W 81.457423°	0.0 - 1.0	Grey Brown Slightly Silty Fine SAND (SP-SM/A-3)
	1.0 - 2.0	Dark Brown Slightly Silty Fine SAND (SP-SM/A-3)
	4.0 - 6.0	Brown Slightly Silty Fine SAND (SP-SM/A-3)
	A.B.T.b	GWLC= 2.0'

Storm-water Management Facility

LA1 N 30.61904° W 81.45922°	0.0 - 0.5	Root Matt - 6"
	0.5 - 3.0	Dark Grey Brown Slightly Silty Fine SAND with Some Roots (SP-SM/A-3)
	3.0 - 8.0	Brown Slightly Silty Fine SAND with Trace Roots (SP-SM/A-3)
	8.0 - 15.0	Brown Slightly Silty Fine SAND (SP-SM/A-3)
	A.B.T.b	GWLC= 1.5'
		Estimated Seasonal High GWL = 1.0 to 1.5 feet'

a - Depth measured below ground surface existing at boring location at time of drilling

b - Auger Boring Terminated (A.B.T.)

c - Groundwater Level (GWL) depth below existing ground surface, recorded at time of drilling, unless otherwise noted
 J20090 aug.wpd

FIELD EXPLORATION PROCEDURES

Penetration Borings

The penetration borings were made in general accordance with ASTM D 1586-67, "Penetration Test and Split-Barrel Sampling of Soils". The borings were advanced to the water table by augering and, after encountering the groundwater table, further advanced by rotary drilling techniques using a circulating bentonite fluid for borehole flushing and stability. At two-foot intervals within the upper 10 feet and at five-foot intervals thereafter, the drilling tools were removed from the borehole and a split-barrel sampler inserted to the borehole bottom and driven 18 inches into the material using a 140-pound SPT hammer falling, on the average, 30 inches per hammer blow. The number of hammer blows for the final 12 inches of penetration is termed the "penetration resistance, blow count, or N-value". This value is an index to several in-place geotechnical properties of the material tested, such as relative density and Young's Modulus.

After driving the sampler 18 inches (or less, if in hard rock or rock-like material) at each test interval, the sampler was retrieved from the borehole and a representative sample of the material within the split-barrel was placed in a glass jar and sealed. After completing the drilling operations, the samples for the borings were transported to our laboratory where they were examined by our engineer in order to verify the driller's field classifications. The samples will be kept in our laboratory for a period of two months after submittal of formal written report, unless otherwise directed by the client.

Auger Borings

The auger borings were performed manually using a post-hole auger. The auger borings were drilled in general accordance with ASTM D 1452-80 ("Soil Investigation and Sampling by Auger Borings"). Representative samples of the soils brought to the ground surface by the augering process were placed in glass jars, sealed, and transported to our laboratory where they were examined by a geotechnical engineer to verify the driller's field classification.

NOTE: SOME DETAILS IN THESE DRAWINGS ARE TO BE CONSIDERED TYPICAL AND MAY NOT BE CALLED OUT ON PLANS. CONTRACTOR SHALL BECOME FAMILIAR WITH AND FOLLOW ALL DETAILS DURING CONSTRUCTION IN ALL TYPICAL AREAS.

REGISTERED DESIGN PROFESSIONAL	ASA R. GILLETTE, P.E. FLORIDA P.E. NO. 56177
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ENGINEERING PLANS

FOR

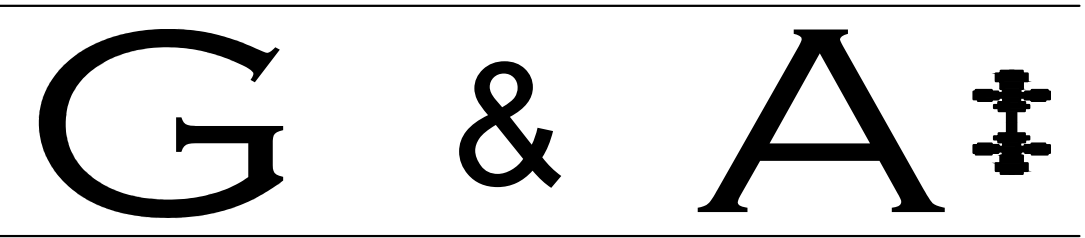
AMELIA ISLAND MEDICAL OFFICES

FOR

BEACHES DERMATOLOGY

GILLETTE & ASSOCIATES, INC.

CIVIL & ENVIRONMENTAL ENGINEERING
MECHANICAL & STRUCTURAL ENGINEERING
PERMITTING & CONSTRUCTION MANAGEMENT
20 SOUTH 4TH STREET AMELIA ISLAND, FLORIDA 32034
PHONE: 904/261-8819



PERMITTING AGENCIES

CITY OF FERNANDINA BEACH
ST. JOHNS RIVER WATER MANAGEMENT DISTRICT
NASSAU COUNTY (RIGHT OF WAY)
F.D.E.P. (UTILITIES)

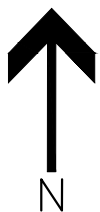
G&A Project
No.: XX-XX-XX

ISSUE DATE: MAY 12, 2022

AMELIA ISLAND MEDICAL OFFICES

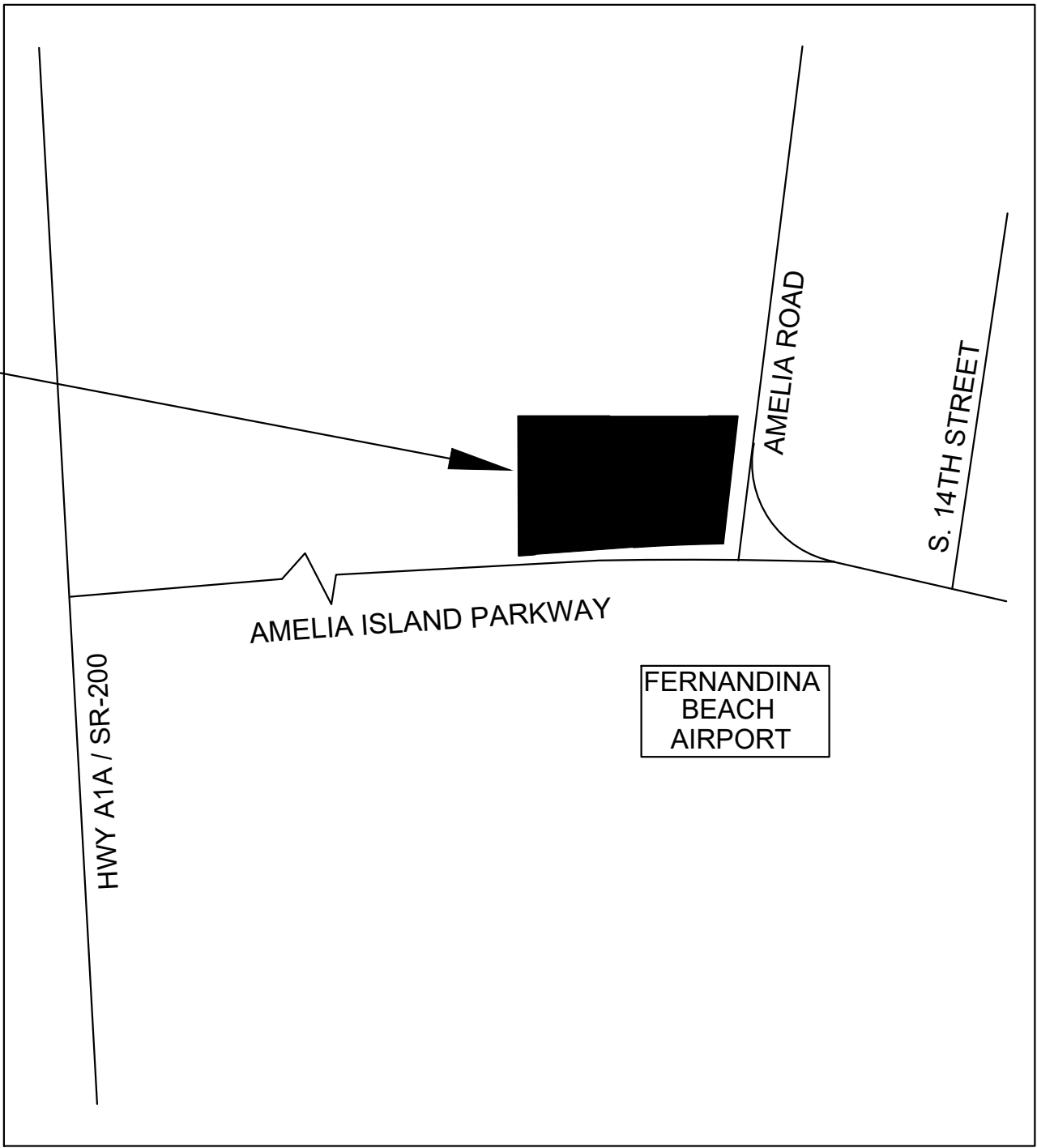
FOR
BEACHES DERMATOLOGY

RELEASED FOR CONSTRUCTION BY



SITE LOCATION MAP

SITE



GILLETTE & ASSOCIATES, INC. CIVIL, ENVIRONMENTAL, MECHANICAL, & STRUCTURAL ENGINEERING CERTIFICATE OF AUTHORIZATION NO. 9332 ASA R. GILLETTE, P.E. * FL PE NO. 56177	REV A	DATE 05/16/22	ITEM C.O.F.B. TRC SUBMITAL #1
20 S. 4th STREET * FERNANDINA BEACH, FL 32034 PHONE: 904-261-8819 * www.gilletteassociates.com			

AMELIA ISLAND MEDICAL OFFICE BEACHES DERMATOLOGY AMELIA ISLAND PARKWAY FERNANDINA BEACH, FLORIDA

DATE: 5/16/22	CVR
SCALE: NONE	Page
DRAWING IS REDUCED IF LESS THAN 22" x 34"	
COVER SHEET	
North / Elev Key Sheet	

PERMIT REVIEW – NOT FOR CONSTRUCTION

GENERAL NOTES

SITE / GEOMETRY

1. ALL WORK SHALL BE PERFORMED IN A SAFE MANNER. ALL SAFETY RULES AND GUIDELINES OF O.S.H.A. SHALL BE FOLLOWED. THE CONTRACTOR SHALL BE WHOLLY RESPONSIBLE FOR ANY INJURIES OF HIS EMPLOYEES, AND ANY DAMAGE TO PRIVATE PROPERTY OR PERSONS DURING THE COURSE OF THIS PROJECT. ALL COSTS ASSOCIATED WITH COMPLYING WITH REGULATIONS AND THE FLORIDA TRENCH SAFETY ACT MUST BE INCLUDED IN THE CONTRACTOR'S PRICE FOR PERFORMING THE WORK.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VISITING THE JOB SITE PRIOR TO PREPARING THE BID FOR THE PURPOSE OF FAMILIARIZING HIMSELF WITH THE NATURE AND THE EXTENT OF THE WORK AND LOCAL CONDITIONS, EITHER SURFACE OR SUBSURFACE, WHICH MAY AFFECT THE WORK TO BE PERFORMED, AND THE EQUIPMENT, LABOR AND MATERIALS REQUIRED. FAILURE TO DO SO WILL NOT RELIEVE THE CONTRACTOR OF COMPLETE PERFORMANCE UNDER THIS CONTRACT. THE CONTRACTOR IS URGED TO TAKE COLOR PHOTOGRAPHS ALONG THE ROUTE OF THIS PROJECT TO RECORD EXISTING CONDITIONS PRIOR TO CONSTRUCTION, AND TO AID IN RESOLVING POSSIBLE FUTURE COMPLAINTS THAT MAY OCCUR DUE TO CONSTRUCTION OF THE PROJECT. ALL COSTS ASSOCIATED WITH PRELIMINARY INVESTIGATION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

3. ALL IMPROVEMENTS SHOWN ARE TO BE WARRANTED BY THE CONTRACTOR TO THE DEVELOPER.

4. ELEVATIONS ARE BASED ON NGVD DATUM OF 1929.

5. TOPOGRAPHIC AND BOUNDARY INFORMATION WAS PROVIDED BY: MRE OF JAX ENTERPRISES, LLC, 6005 POWERS AVENUE, SUITE 104, JACKSONVILLE, FL 32217

6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY BUILDING, RIGHT-OF-WAY PERMITS NOT PROVIDED AND INSURANCE REQUIRED FOR THE PROJECT.

7. THE CONTRACTOR SHALL COORDINATE THE WORK WITHIN CITY, COUNTY OR STATE RIGHT-OF-WAY WITH THE PROPER AGENCIES FOR MAINTENANCE OF TRAFFIC AND METHOD OF CONSTRUCTION & REPAIR.

7. THE CONTRACTOR SHALL ADVISE THE OWNER'S AUTHORIZED REPRESENTATIVE OF PLANNED WORK SCHEDULE. THE CONTRACTOR SHALL COORDINATE THEIR CONSTRUCTION WITH ALL OTHER CONTRACTORS. IN THE EVENT OF ANY CONFLICT WHATSOEVER, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD, AND THE OWNER PRIOR TO PROCEEDING WITH CONSTRUCTION.

8. "AS-BUILT" DRAWINGS – AS-BUILTS ARE REQUIRED TO BE SIGNED AND SEALED BY A FLORIDA REGISTERED LAND SURVEYOR. THEREFORE, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO CONTRACT WITH A LAND SURVEYOR REGISTERED IN THE STATE OF FLORIDA FOR THE PREPARATION, FIELD LOCATIONS, CERTIFICATION, AND SUBMITTAL OF AS-BUILT DRAWINGS.

9. "AS-BUILT" RECORD DATA AND INFORMATION SHALL BE MAINTAINED BY THE CONTRACTOR. RECORD DRAWINGS SHALL BE SUPPLIED TO THE ENGINEER.

10. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF ALL SURVEY AND PROPERTY MONUMENTS. IF A MONUMENT IS DISTURBED, THE CONTRACTOR SHALL CONTRACT WITH THE SURVEYOR OF RECORD FOR REINSTALLATION OF THE MONUMENT.

11. ALL DEBRIS RESULTING FROM ALL ACTIVITIES SHALL BE DISPOSED OF OFF-SITE BY CONTRACTOR ON A DAILY BASIS.

12. ALL EXCESS SUITABLE AND UNSUITABLE MATERIALS SHALL BE REMOVED FROM THE SITE BY THE CONTRACTOR UNLESS DIRECTED OTHERWISE BY THE ENGINEER.

13. ALL EXISTING TREES LISTED TO REMAIN SHALL BE PRESERVED AND PROTECTED.

14. THE LOCATION OF ALL EXISTING UTILITIES, STRUCTURES, AND IMPROVEMENTS ON THE DRAWINGS ARE BASED ON LIMITED INFORMATION AND MAY NOT HAVE BEEN FIELD VERIFIED. THE LOCATIONS ARE APPROXIMATE. THE CONTRACTOR SHALL NOTIFY RESPECTIVE UTILITY OWNERS AND FIELD VERIFY LOCATIONS OF EXISTING UTILITIES AND OTHER IMPROVEMENTS PRIOR TO COMMENCING ANY CONSTRUCTION. IF THE LOCATIONS SHOWN ARE CONTRARY TO ACTUAL FIELD LOCATIONS, THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER OF THE DISCREPANCY. THE DISCREPANCY SHOULD BE RESOLVED PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION WHEN WORKING IN AREAS NEAR EXISTING UTILITY IMPROVEMENTS AND SHALL BE RESPONSIBLE FOR REPAIRS OR PAYMENTS FOR ALL DAMAGE MADE TO EXISTING UTILITIES OR OTHER IMPROVEMENTS. PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION, THE CONTRACTOR SHALL VERIFY ALL GRADES, INVERTS, AND TYPE OF MATERIAL OF EXISTING UTILITIES TO WHICH THEY SHALL CONNECT.

15. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER ON ALL MATERIALS, FOR REVIEW AND APPROVAL, PRIOR TO PURCHASE OR CONSTRUCTION OF ANY UTILITY OR STRUCTURE.

17A. UNSUITABLE MATERIALS UNDER WATER, SEWER, OR STORM PIPE AND/OR STRUCTURES SHALL BE REMOVED AND REPLACED WITH SELECTED BACKFILL COMPACTED TO 98% OF ITS MAXIMUM DENSITY AS DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557).

17B. ALL FINISH FLOORS SHOWN ON THE PLANS ARE MINIMUM ELEVATIONS

18. ALL WATER & SEWER CONSTRUCTION SHALL BE ACCOMPLISHED BY AN UNDERGROUND UTILITY CONTRACTOR LICENSED UNDER THE PROVISIONS OF CHAPTER 489 FLORIDA STATUTES.

19. WHEN DEWATERING CAPACITY REQUIRES A PERMIT (CUP), IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN THE PERMIT THROUGH THE ST. JOHNS RIVER WATER MANAGEMENT DISTRICT.

20. WATER MAINS SHALL BE LAID AT LEAST 10 FEET HORIZONTALLY FROM ANY EXISTING OR PROPOSED SANITARY HAZARD. THE DISTANCE SHALL BE MEASURED EDGE TO EDGE. WHERE PARALLEL WATER AND SEWER MAINS HAVE LESS THAN 10 FOOT HORIZONTAL SEPARATION, 20 FOOT LENGTHS OF C900 (DR18) WATER QUALITY PIPE SHALL BE USED WITH THE JOINTS STAGGERED AT 10 FOOT INTERVALS. THE PIPES SHALL BE PLACED IN SEPARATE TRENCHES WITH THE WATER MAIN AT LEAST 18 INCHES ABOVE THE SEWER.

21. WATER MAINS CROSSING SANITARY HAZARDS SHALL BE LAID TO PROVIDE A MINIMUM VERTICAL DISTANCE OF 18 INCHES BETWEEN THE OUTSIDE OF THE WATER MAIN AND THE OUTSIDE OF THE SEWER MAIN. WHERE WATER AND SEWER MAINS CROSS WITH BETWEEN 18 INCHES AND 6 INCHES OF VERTICAL CLEARANCE, A 20 FOOT SECTION OF C900 (DR18) WATER QUALITY PIPE SHALL BE CENTERED ON THE POINT OF THE CROSSING. THE CONTRACTOR SHALL FIELD VERIFY THE VERTICAL SEPARATION. THE MINIMUM SEPARATION BETWEEN WATER AND SEWER PIPES IS TO BE 6 INCHES OUTSIDE DIAMETER TO OUTSIDE DIAMETER.

22. NO WATER MAIN PIPE SHALL PASS THROUGH OR COME IN CONTACT WITH ANY PART OF A SEWER MANHOLE.

23. BLANK

24. TRACER WIRE SHALL BE PROVIDED ON ALL WATER MAINS AND FORCE MAINS. TRACER WIRE SHALL BE #10 UF SOLID COPPER WIRE, COATED, AND SUITABLE FOR BURIAL. COATING SHALL BE BLUE FOR WATER AND GREEN FOR SEWER MAIN CONNECTIONS. WIRE SHALL BE ATTACHED DIRECTLY TO TOP OF PIPE AND BROUGHT INTO EACH VALVE BOX AND COILED TO ALLOW FOR EASY ACCESS AND EXTENSION ABOVE FINISHED GRADE FOR ATTACHMENT OF LOCATING EQUIPMENT.

25. ALL PIPE LENGTHS ARE APPROXIMATE DIMENSIONS. ALL DRAINAGE STRUCTURES SHALL BE CONSTRUCTED TO CONFORM WITH TYPICAL SECTIONS AND DETAILS AS SHOWN ON THE MISCELLANEOUS DETAILS SHEET AND IN ACCORDANCE WITH THE SPECIFICATIONS.

26. ALL UNDERGROUND UTILITIES MUST BE INSTALLED PRIOR TO PREPARATION OF SUBGRADE FOR PAVEMENT.

27. GRADES SHOWN ON PLANS ARE EDGE OF PAVEMENT FINISHED GRADES UNLESS OTHERWISE NOTED.

28. SHOULD THE SURFACE OR SUBSURFACE CONDITIONS VARY FROM WHAT IS SHOWN ON THE PLANS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER.

29. ALL AREAS DISTURBED DURING CONSTRUCTION SHALL BE SEEDED AND MULCHED UNLESS OTHERWISE SPECIFICALLY STATED.

30. THE CONTRACTOR IS RESPONSIBLE FOR THE CONTROL OF SEDIMENT-LADEN RUNOFF RESULTING FROM STORM EVENTS DURING THE CONSTRUCTION PHASE. EROSION CONTROL FACILITIES SHOULD BE INSTALLED EARLY DURING THE CONSTRUCTION PERIOD SO AS TO PREVENT THE TRANSPORT OF SEDIMENT INTO SURFACE WATERS. RE-VEGETATION AND STABILIZATION OF DISTURBED AREAS SHOULD BE ACCOMPLISHED IMMEDIATELY TO REDUCE THE POTENTIAL FOR FURTHER SOIL EROSION.

31. ANY DISCREPANCIES ON THE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND COUNTRY/CITY BEFORE COMMENCING WORK.

32. MINIMUM COVER OVER PIPES SHALL BE 36" UNLESS OTHERWISE INDICATED.

33. ALL UNDERGROUND UTILITY COMPANIES SHALL BE CONTACTED AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION.

34. IN ADDITION TO THE SPECIFICATIONS AND DETAILS PROVIDED FOR THIS CONTRACT, THE NASSAU COUNTY STANDARD DETAILS AND SPECIFICATIONS (LATEST REVISION) SHALL ALSO BE USED FOR ANY ITEMS NOT COVERED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN A SET OF STANDARD DETAILS AND SPECIFICATIONS FROM NASSAU COUNTY.

35. BACKFILL MATERIAL SHALL BE COMPACTED TO 100% (2%) PER CITY OF FB REQUIREMENTS UNDER EXISTING AND PROPOSED ROADWAYS.

36. NO FIELD CHANGES OR DEVIATIONS FROM DESIGN TO BE MADE WITHOUT PRIOR APPROVAL OF THE ENGINEER.

37. STATIONING REFERS TO THE CENTERLINE OF THE ROADWAY EXCEPT AS NOTED ON PLANS. WHERE PAVEMENT SPLITS, THE STATIONING FOLLOWS THE INSIDE EDGE OF PAVEMENT UNTIL ROADWAY TRANSITIONS BACK TO THE TYPICAL ROADWAY SECTION

38. ALL CONSTRUCTION, MATERIAL, AND WORKMANSHIP ARE TO BE IN ACCORDANCE WITH CITY OF FB AND COUNTY STANDARDS

WATER NOTES

1. WATER MAINS SHALL HAVE A MINIMUM OF 36" COVER UNDER PAVEMENT OR 30" COVER FROM FINISHED GRADE UNLESS OTHERWISE SHOWN. ALL WATER MAINS SHALL BE FLUSHED IN ACCORDANCE WITH, AND UNDER THE DIRECTION OF THE UTILITY COMPANY.

2. THRUST RESTRAINTS ARE REQUIRED IN ACCORDANCE WITH THE UTILITY COMPANY STANDARDS WHERE WATER MAINS ARE TERMINATED AND AT ALL BENDS AND TEES.

3. TIE RODS SHALL BE INSTALLED ON ALL VALVES LOCATED AT FITTINGS. ALL 10" AND LARGER VALVES AND LARGER SHALL REQUIRE (4) TIE RODS. MEGA LUG JOINT RESTRAINTS MAY BE USED IN LIEU OF THRUST BLOCKS OR TIE RODS.

4. UNLESS OTHERWISE NOTED, WATER MAINS ARE TO BE PVC DR18, DR-25, C-900 (BLUE), CLASS 150.

5. ALL WATER MAINS SHALL BE BACTERIOLOGICAL AND PRESSURE TESTED IN ACCORDANCE WITH AWWA C-600. NO CONNECTION TO EXISTING POTABLE WATER SYSTEM SHALL BE ALLOWED UNTIL ALL PROPOSED WATER MAINS HAVE BEEN PRESSURE TESTED, DISINFECTED, AND CLEARED FOR SERVICE. THE UTILITY COMPANY'S AUTHORIZED REPRESENTATIVE MUST BE NOTIFIED 48 HOURS PRIOR TO PERFORMING THE PRESSURE TEST AND MUST BE PRESENT.

6. THE CONTRACTOR SHALL AVOID SERVICE INTERRUPTIONS AND MAINTAIN ANY EXISTING WATER SERVICE TO MEET THE SYSTEM DEMANDS AT ALL TIMES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFICATION OF AFFECTED CUSTOMERS OF THE UTILITY A MINIMUM OF 48 HOURS IN ADVANCE OF ANY INTERRUPTION OF SERVICE. BOIL WATER NOTICES SHALL BE DELIVERED IN ACCORDANCE WITH THE UTILITY COMPANY BOIL WATER NOTICE REQUIREMENTS.

7. ALL MAIN FLUSHING POINTS SHALL BE SAME SIZE AS MAIN UP TO 8 INCH WATER MAINS. FLUSHING POINTS FOR MAINS LARGER THAN 8 INCH SHALL BE REDUCED TO 8 INCH.

8. ALL WATER MAINS SHALL BE DISINFECTED USING LIQUID CHLORINE OR HYPOCHLORITES IN ACCORDANCE TO AWWA C651-86. WATER IN MAIN SHALL MAINTAIN A CHLORINE RESIDUAL OF 50 MG/L FOR A MINIMUM OF 24 HOURS. WATER MAIN SHALL NOT BE PLACED IN SERVICE UNTIL CLEARANCE HAS BEEN PROVIDED BY THE APPROPRIATE REGULATING AGENCY AND CITY OFFERNANDINA BEACH UTILITIES DEPARTMENT.

9. IF SOLVENT CONTAMINATION IS FOUND IN THE PIPE TRENCH, WORK SHALL BE STOPPED AND THE PROPER AUTHORITIES NOTIFIED. BASED ON APPROVAL OF THE PERMITTING AGENCY, DUCTILE IRON PIPE, FITTINGS, AND SOLVENT RESISTANT GASKET MATERIAL SUCH AS FLUOROCARBON SHALL BE USED IN THE CONTAMINATED AREA. THE DUCTILE IRON PIPE SHALL EXTEND AT LEAST 100 FEET BEYOND ANY SOLVENT NOTED. ANY CONTAMINATED SOIL THAT IS EXCAVATED SHALL BE PLACED ON AN IMPERMEABLE MAT AND COVERED WITH A WATER PROOF COVERING. THE PROPER AUTHORITIES WILL BE NOTIFIED AND THE CONTAMINATED SOIL HELD FOR PROPER DISPOSAL.

10. 2" FLUSHING HYDRANTS SHALL BE INSTALLED AT ALL DEAD END LINES AND MAIN LINE STUB-OUTS.

11. A SEPARATE WATER SERVICE SHALL BE REQUIRED FOR EACH UNIT SERVED AND SHALL BE INSTALLED 3 FEET EITHER SIDE OF PROPERTY LINE. ALL WATER SERVICES SHALL BE MARKED WITH A "W" ETCHED ON THE CURB. ALL VALVES SHALL BE MARKED WITH A "V" ETCHED ON THE CURB.

12. ALL BACKFLOW PREVENTION ASSEMBLIES SHALL BE INSTALLED IN ACCORDANCE WITH THE UTILITY COMPANY'S STANDARDS.

13. ALL BACKFLOW PREVENTION ASSEMBLIES SHALL BE INSTALLED AT THE CONSUMER'S METER OR AT THE PROPERTY LINE WHEN A METER IS NOT USED.

14. BACKFLOW PREVENTION ASSEMBLIES SHALL BE TESTED BY A CERTIFIED BACKFLOW TECHNICIAN. TEST RESULTS SHALL BE SUBMITTED TO AND APPROVED BY THE UTILITY COMPANY PRIOR TO ACCEPTANCE OF INSTALLATION.

15. MAXIMUM DEPTH ON WATER SERVICES TO METER TO BE 36".

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SITE DETAILS

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SITE DETAILS

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WATER AND SEWER DETAILS

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DT-4

LIFT STATION DETAILS

FIRE NOTES

1. WHERE UNDERGROUND WATER MAINS AND HYDRANTS ARE TO BE PROVIDED, THEY SHALL BE INSTALLED, COMPLETED, AND IN SERVICE PRIOR TO COMMENCING CONSTRUCTION WORK ON ANY STRUCTURE. (NFPA 1, 18.4.3.1.3, 2009 EDITION)

2. CLEARANCES OF SEVEN AND ONE HALF FEET (7 FT-6 IN.) IN FRONT OF AND TO THE SIDES OF THE FIRE HYDRANT, WITH A FOUR FOOT CLEARANCE TO THE REAR OF THE HYDRANT. (NFPA 1, 18.3.4.1, 2009)

3. NEW FIRE HYDRANT(S) SHALL BE INSTALLED SO THAT POSITIONED NOT MORE THAN 7 FEET 6 INCHES AWAY FROM CURB OR BERM OF THE ROADWAY.

PERMIT REVIEW – NOT FOR CONSTRUCTION

NOTE: SOME DETAILS IN THESE DRAWINGS ARE TO BE CONSIDERED TYPICAL AND MAY NOT BE CALLED OUT ON PLANS. CONTRACTOR SHALL BECOME FAMILIAR WITH AND FOLLOW ALL DETAILS DURING CONSTRUCTION IN ALL TYPICAL AREAS.

REGISTERED DESIGN PROFESSIONAL

ASA R. GILLETTE, P.E.
FLORIDA P.E. NO. 56177

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C.O.F.B. TRC SUBMITTAL #1

05/16/22

A

GILLETTE & ASSOCIATES, INC.

CIVIL, ENVIRONMENTAL, MECHANICAL, & STRUCTURAL ENGINEERING

CERTIFICATE OF AUTHORIZATION NO. 9332
ASA R. GILLETTE, P.E. * FL PE NO. 56177

20 S. 4th STREET * FERNANDINA BEACH, FL 32034

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AMELIA ISLAND MEDICAL OFFICE

BEACHES DERMATOLOGY

AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

DATE: 5/16/22

SCALE: NONE

DRAWING IS REDUCED IF LESS THAN 22" x 34"

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& NOTES

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PERMIT REVIEW – NOT FOR CONSTRUCTION

----- 4.5' -----

EXISTING VALVE

----- WM -----

EXISTING WATER LINE

----- SAN -----

EXISTING SANITARY SEWER

EXISTING FIRE HYDRANT

----- SAN -----

PROPOSED SANITARY SEWER

----- WM -----

PROPOSED WATER LINE

PROPOSED FIRE HYDRANT

EXISTING EDGE OF PAVEMENT

EXISTING PROPERTY LINE

15.3

PROPOSED SPOT ELEVATION

S-5

DRAINAGE STRUCTURE

P-1

PIPE

15.0

PROPOSED CONTOUR

PROPOSED EDGE OF PAVEMENT

PROPOSED PROPERTY LINE

PROPOSED R.O.W. LINE

----- 16 -----

EXISTING CONTOUR

1. THE CONTRACTOR SHALL PROVIDE, TO THE ENGINEER OF RECORD, A SCHEDULE OF INVERT ELEVATIONS OF ALL SANITARY MANHOLES, PRIOR TO THE PLACEMENT OF THE LIME ROCK BASE COURSE. THIS SCHEDULE TO BE PROVIDED BY THE REGISTERED LAND SURVEYOR SUBMITTING THE "AS-BUILT" DRAWINGS FOR THIS PROJECT.

2. SANITARY SEWER SERVICES SHALL BE 6" PVC WITH A MINIMUM SLOPE OF 0.006 FEET PER FOOT AND SHALL BE TERMINATED AT THE RIGHT-OF-WAY LINE WITH A DEPTH OF 24" TO 48" FROM THE FINAL FINISHED GRADE OF THE DEVELOPED LOTS. DEPTHS GREATER THAN 48" FROM FINAL FINISHED GRADE SHALL BE READJUSTED BY THE DEVELOPER AT HIS COST.

3. GRAVITY SEWER TO BE CLEANED & TV TESTED BY DEVELOPER AT DEVELOPER'S EXPENSE

4. ALL MANHOLE FRAMES AND COVERS TO BE ADJUSTED TO FINAL FINISHED GRADE.

5. A SEPARATE SEWER SERVICE (6" MIN.) SHALL BE PROVIDED TO THE RIGHT-OF-WAY FOR EACH LOT BEING SERVED. AN "S" SHALL BE ETCHED INTO THE CURB DIRECTLY OVER EACH SERVICE LINE OR IN THE STREET SIDE OF A SIDEWALK IN LINE WITH THE CLEANOUT.

6. GRAVITY SEWER TO BE GREEN IN COLOR

7. NEW GRAVITY SEWER SYSTEM SHALL BE PLUGGED AT NEAREST EXISTING DOWNSTREAM MANHOLE AND SHALL NOT BE REMOVED WITHOUT AUTHORIZATION FROM THE UTILITY COMPANY. EXISTING MANHOLES TO BE COREBORED AND ARRANGEMENTS MADE TO BE REFURBISHED WITH COMPATIBLE MATERIALS IF THE EXISTING MANHOLE IS ALLOWED TO BE USED.

8. FINISHED FLOOR ELEVATIONS SHALL BE GREATER (HIGHER) THAN THE ELEVATION OF THE MANHOLE NEAREST THE SERVICE LATERAL IN ORDER TO AVOID A POTENTIAL BACKUP OF SEWERAGE IN THE HOUSE OR BUILDING.

9. ALL GRAVITY SEWER PIPE TO A 10 FOOT DEPTH OF COVER SHALL BE SDR-26 PVC PIPE (MINIMUM). GRAVITY SEWER PIPE WITH COVER OF 15 FEET (AND IN EASEMENTS) AND GREATER SHALL BE C900 - DR18. JOINT SHALL BE OF THE BELL AND SPIGOT TYPE AND MEET THE REQUIREMENTS OF THE PIPE REFERRED BEING USED.

10. WHEN A PROPERTY LINE CAN NOT BE DEFINED, THE SEWER CLEANOUT SHALL BE PLACED IN AN UNPAVED AREA AT A DISTANCE NO GREATER THAN 30 FEET FROM THE SEWER MAIN.

11. NOTE: DR-18 TO BE USED ON SEWER LINE CROSSING WHERE STORM DRAIN IS WITHIN 24 INCHES ABOVE GRAVITY ALSO JOINT TO BE CENTERED AT CROSSING

12. ALL MANHOLES TO HAVE MINIMUM 24" HINGED RING AND COVER EAST JORDAN OR APPROVED EQUIVALENT

13. SEWER PLUGS USED DURING THE CONSTRUCTION OR CONNECTION TO A SYSTEM SHALL HAVE THE CONTRACTOR'S NAME PERMANENTLY STAMPED.

14. THE MANHOLE LINER SHALL BE SPECTRA SHIELD OR APPROVED EQUIVALENT.

15. IN ACCORDANCE WITH THE DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) REQUIREMENTS, LEAKAGE TESTING SHALL BE PERFORMED ON ALL SEGMENTS OF THE SEWER SYSTEM. TESTING MAY INCLUDE APPROPRIATE WATER OR LOW PRESSURE AIR TESTING. TEST METHODS SHALL TAKE INTO CONSIDERATION THE RANGE IN GROUNDWATER ELEVATIONS DURING THE TEST AND ANTICIPATED DURING THE DESIGNED LIFE OF THE SEWER. NO LEAKAGE ALLOWED IN SYSTEM.

16. ALL FORCE MAIN PIPING TO BE DR-18 GREEN OR BROWN.

17. CONTACT ANDRE DESILET, COFB UTILITY DIRECTOR AT (904-310-3420) FOR PRE-CONSTRUCTION AND COORDINATION MEETING PRIOR TO WORK COMMENCING.

NOTE: SOME DETAILS IN THESE DRAWINGS ARE TO BE CONSIDERED TYPICAL, AND MAY NOT BE CALLED OUT ON PLANS. CONTRACTOR SHALL BECOME FAMILIAR WITH AND FOLLOW ALL DETAILS DURING CONSTRUCTION IN ALL TYPICAL AREAS.

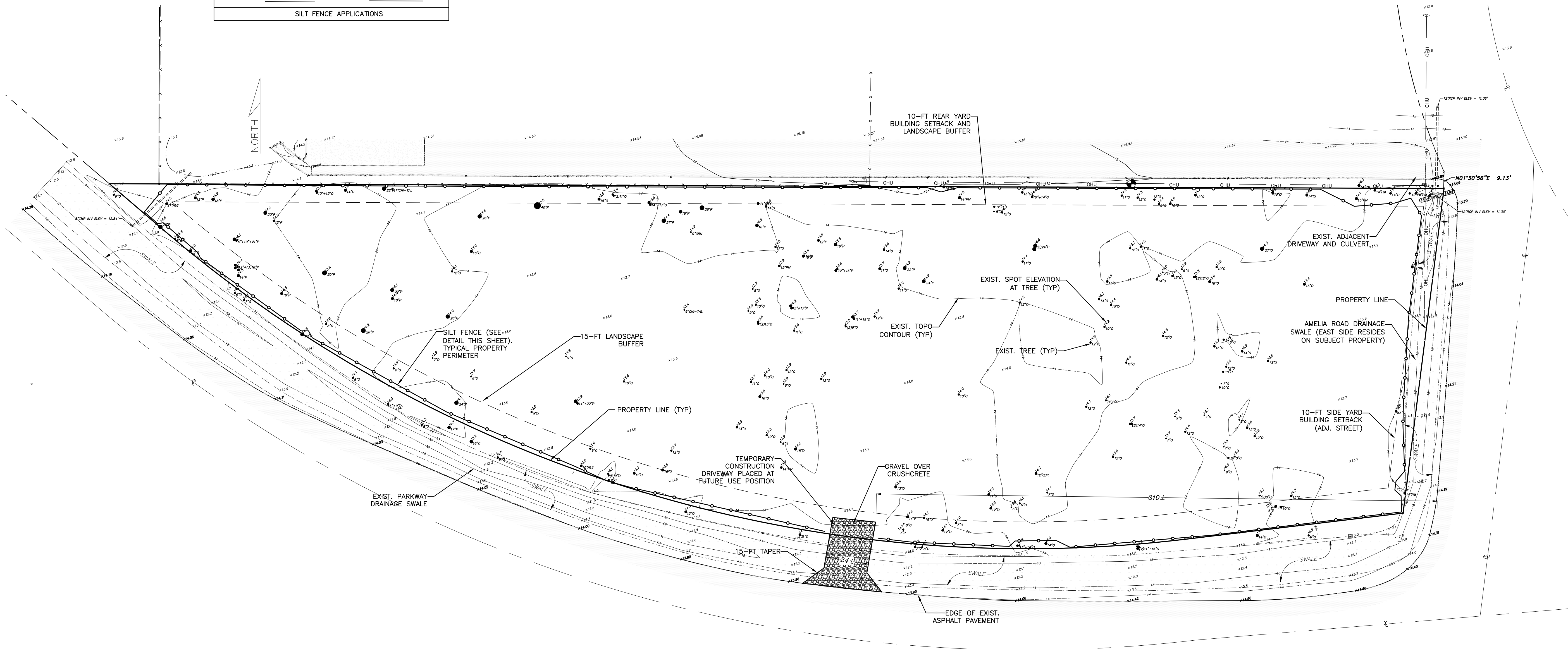
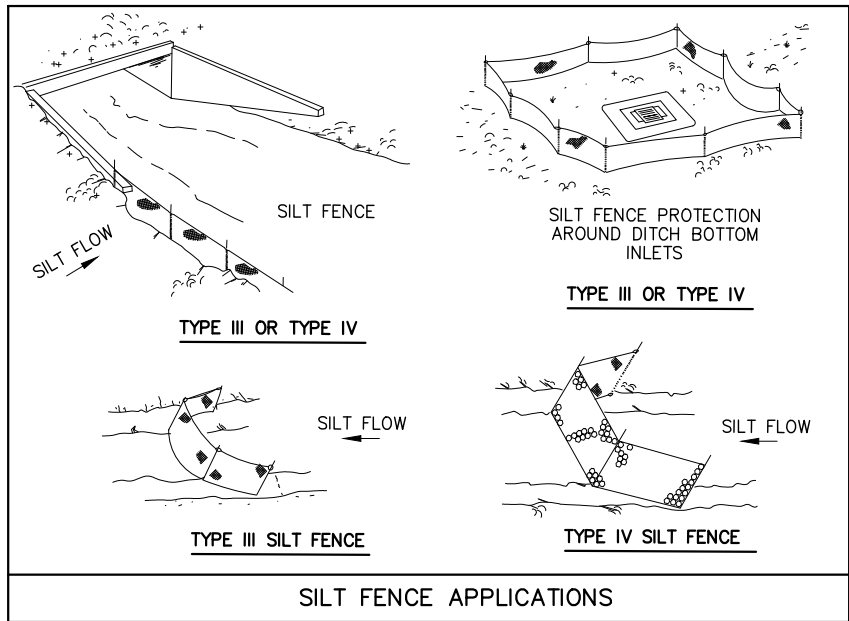
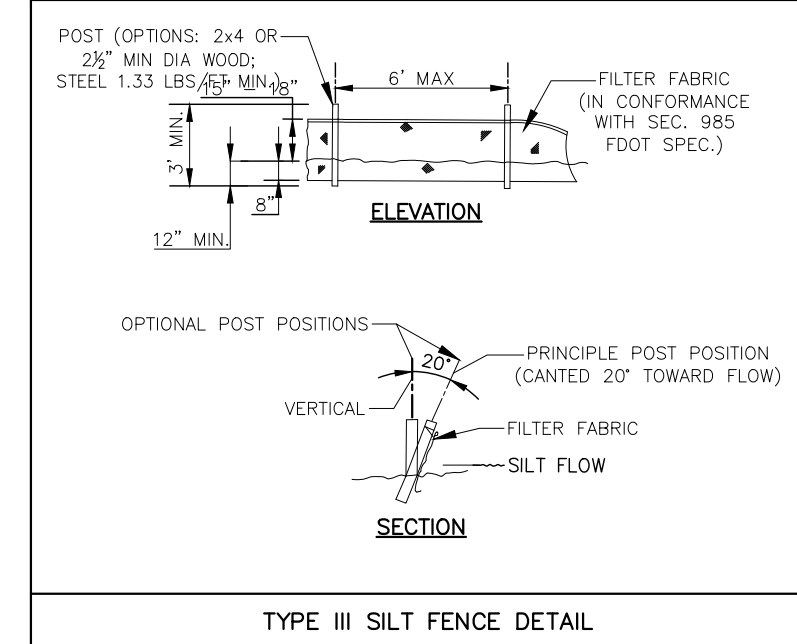
REGISTERED DESIGN PROFESSIONAL
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**AMELIA ISLAND MEDICAL OFFICE
BEACHES DERMATOLOGY**
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

DATE: 5/16/22
SCALE: 1"=30'
DRAWING IS REDUCED IF LESS THAN 22" x 34"
PRE-DEVELOPED PLAN
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PRE-DEVELOPED SITE PLAN
SCALE: 1:30

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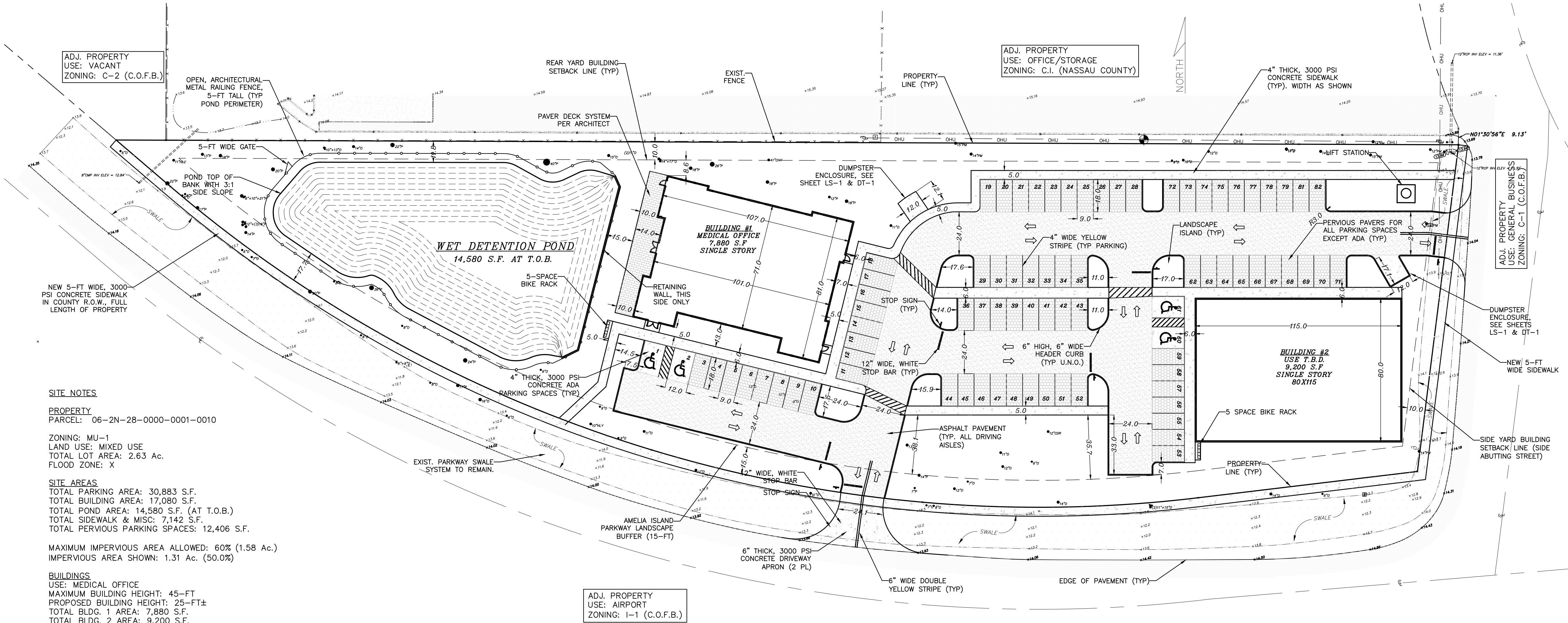
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GEOMETRY PLAN
North / Elev Key Sheet
GE-1
Page



GEOMETRY PLAN
SCALE: 1:30

SITE NOTES

PROPERTY
PARCEL: 06-2N-28-0000-0001-0010

ZONING: MU-1
LAND USE: MIXED USE
TOTAL LOT AREA: 2.63 Ac.
FLOOD ZONE: X

SITE AREAS
TOTAL PARKING AREA: 30,883 S.F.
TOTAL BUILDING AREA: 17,080 S.F.
TOTAL POND AREA: 14,580 S.F. (AT T.O.B.)
TOTAL SIDEWALK & MISC: 7,142 S.F.
TOTAL PERVIOUS PARKING SPACES: 12,406 S.F.

MAXIMUM IMPERVIOUS AREA ALLOWED: 60% (1.58 Ac.)
IMPERVIOUS AREA SHOWN: 1.31 Ac. (50.0%)

BUILDINGS
USE: MEDICAL OFFICE
MAXIMUM BUILDING HEIGHT: 45-FT
PROPOSED BUILDING HEIGHT: 25-FT±
TOTAL BLDG. 1 AREA: 7,880 S.F.
TOTAL BLDG. 2 AREA: 9,200 S.F.
MAXIMUM FLOOR TO AREA RATIO: 0.5
PROPOSED FLOOR TO AREA RATIO: 0.15

SETBACKS
FRONT YARD: 0
SIDE YARD: 0
REAR YARD: 10-FT
SIDE ABUTTING A STREET: 10-FT

UTILITIES:
- ELECTRIC: FLORIDA PUBLIC UTILITIES
- WATER: CITY OF FERNANDINA BEACH
- SEWER: CITY OF FERNANDINA BEACH
- GAS: FLORIDA PUBLIC UTILITIES
- CABLE/PHONE: VARIES

PARKING:
- BLDG 1: MEDICAL OFFICE: 1 SPACE PER 250 S.F.
(7,880 S.F.)/(250 SF/SPACE) = 31 SPACES
- BLDG 2: MEDICAL OFFICE: 1 SPACE PER 250 S.F.
(9,200)/(250 SF/SPACE) = 37 SPACES
TOTAL SPACES ALLOWED: 68
TOTAL SPACES W/10% OVERAGE: 75

TOTAL SPACES SHOWN: 82
ADA SPACES: 4
- NOTE: SITE IS OVERPARKED; VARIANCE REQUIRED FOR SEVEN (7) ADD'L SPACES.

LANDSCAPE & TREE PROTECTION REQUIREMENTS:

- MIN. 20% OF TOTAL GROSS AREA TO BE LANDSCAPED
- MIN. ONE (1) SHADE TREE FOR EACH 1,500 SF OF SITE
- 50% OF THE TOTAL D.B.H. OF THE PROTECTED TREES SHALL BE EXEMPT FROM REPLACEMENT.
- IMPERVIOUS IMPROVEMENTS (OR W/IN 5-FT THEREOF) REQUIRE REPLACEMENT OF 50% OF THE TOTAL D.B.H. OF THE PROTECTED TREES.

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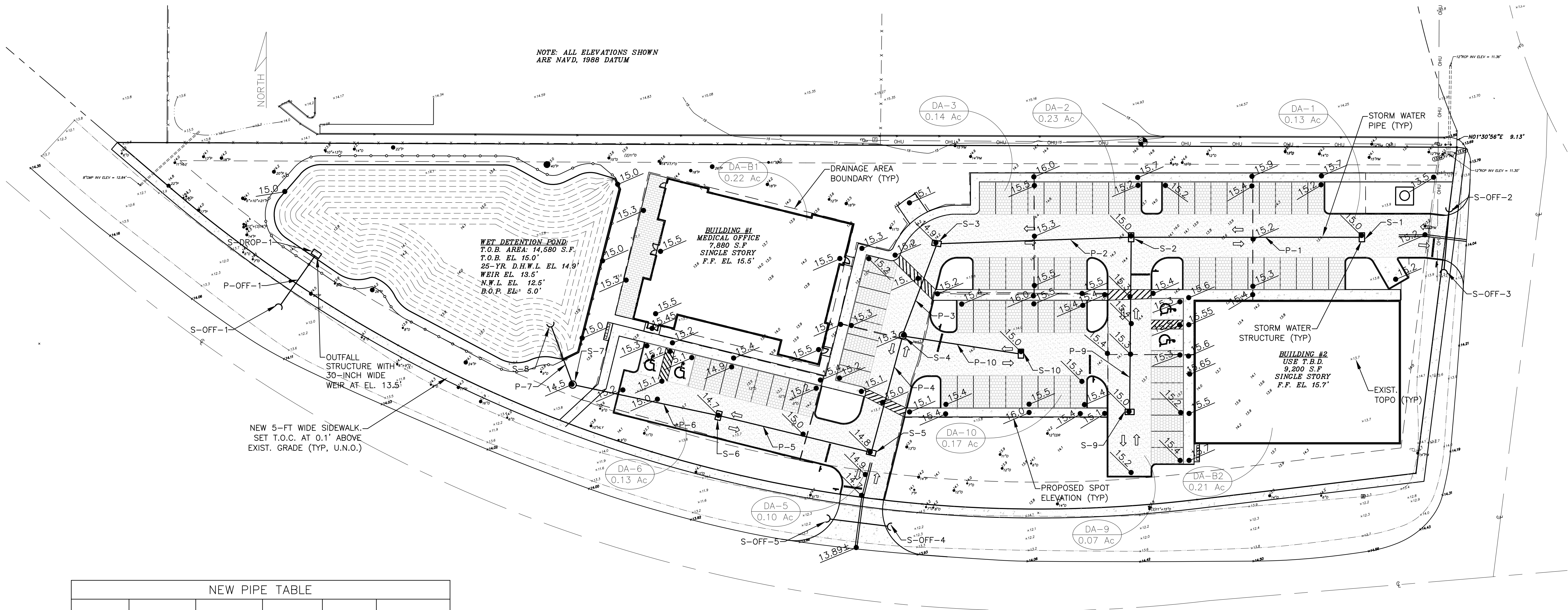
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A	05/16/22	C.O.F.B. TRC SUBMITAL #1

GILLETTE & ASSOCIATES, INC.
CIVIL, ENVIRONMENTAL, MECHANICAL, & STRUCTURAL ENGINEERING
CERTIFICATE OF AUTHORIZATION NO. 9332
ASA R. GILLETTE, P.E. * FL PE NO. 56177
20 S. 4th STREET * FERNANDINA BEACH, FL 32034
PHONE: 904-261-8819 * www.gilletteassociates.com

**AMELIA ISLAND MEDICAL OFFICE
BEACHES DERMATOLOGY**
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

DATE: 5/16/22
SCALE: 1"=30'
DRAWING IS REDUCED IF LESS THAN 22" x 34"
DRAINAGE PLAN
North / Elev Key Sheet
DR-1
Page

NOTE: ALL ELEVATIONS SHOWN ARE NAVD, 1988 DATUM



PAVING & DRAINAGE PLAN
SCALE: 1:30

NEW PIPE TABLE					
NAME	FROM	TO	DIAMETER (IN)	LENGTH (FT)	MATERIAL
P-1	S-1	S-2	18	126	PE
P-2	S-2	S-3	24	105	PE
P-3	S-3	S-4	24	51	PE
P-4	S-4	S-5	30	64	PE
P-5	S-5	S-6	30	84	PE
P-6	S-6	S-7	30	79	PE
P-7	S-7	S-8	30	33	PE
P-9	S-9	S-2	18	93	PE
P-10	S-10	S-4	18	63	PE
P-OFF-1	S-DROP-1	S-OFF-1	24	33	PE
P-OFF-2	S-OFF-2	S-OFF-3	18	35	PE
P-OFF-3	S-OFF-4	S-OFF-5	18	35	PE

NOTE: PIPE LISTED ABOVE TO BE PER FDOT PARAMETERS AND SPECIFICATIONS

NEW STRUCTURE TABLE						
NAME	TYPE	TOP/ INLET ELEV.	INVERT ELEVATION			
			N	S	E	W
S-1	C	15.00	—	—	—	11.25
S-2	E	15.00	—	11.01	10.93	10.43
S-3	E	14.90	—	10.25	10.25	—
S-4	M/H	14.80	10.16	9.66	10.99	—
S-5	E	14.70	9.58	—	—	9.58
S-6	E	14.60	—	9.47	9.47	—
S-7	M/H	14.50	9.37	—	9.37	—
S-8	MES	—	—	9.32	—	—
S-9	C	15.00	11.25	—	—	—
S-10	C	14.90	—	—	—	11.15
S-DROP-1	C	14.90	—	—	12.50	—
S-OFF-1	MES	—	—	—	—	12.00
S-OFF-2	MES	—	—	12.10	—	—
S-OFF-3	MES	—	12.20	—	—	—
S-OFF-4	MES	—	—	—	12.20	—
S-OFF-5	MES	—	—	—	—	12.10

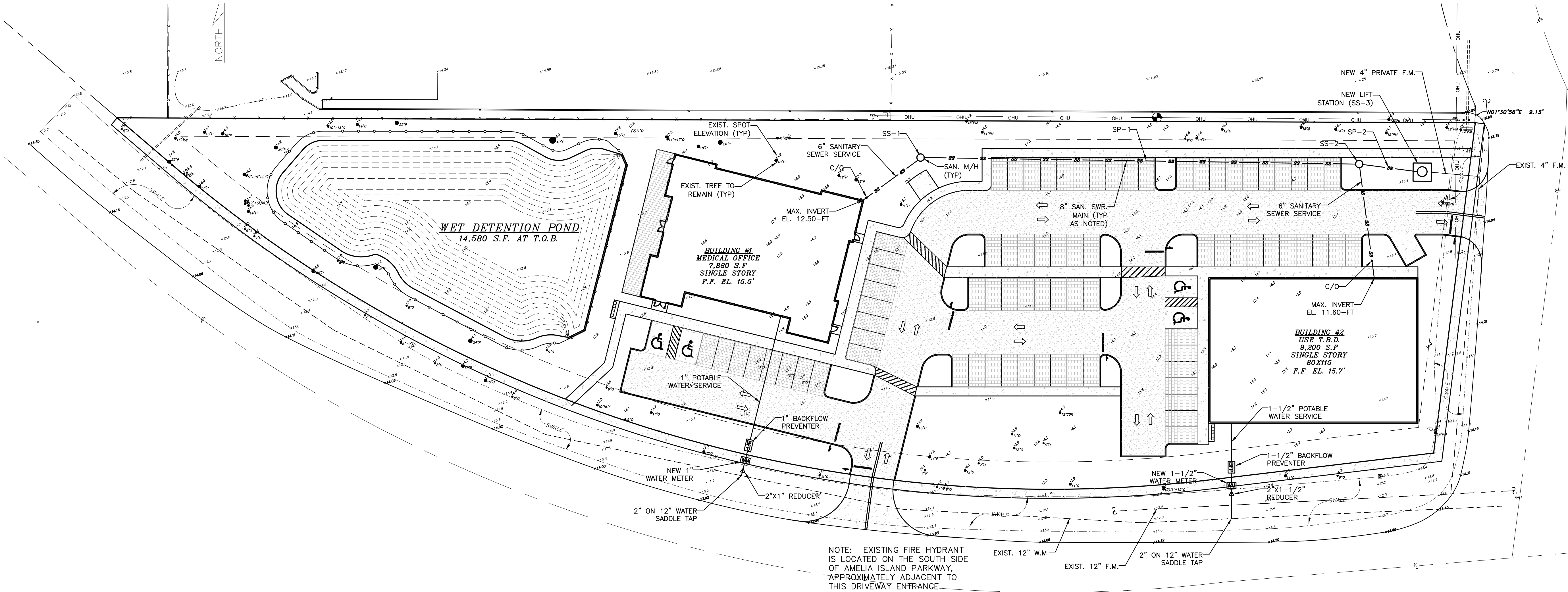
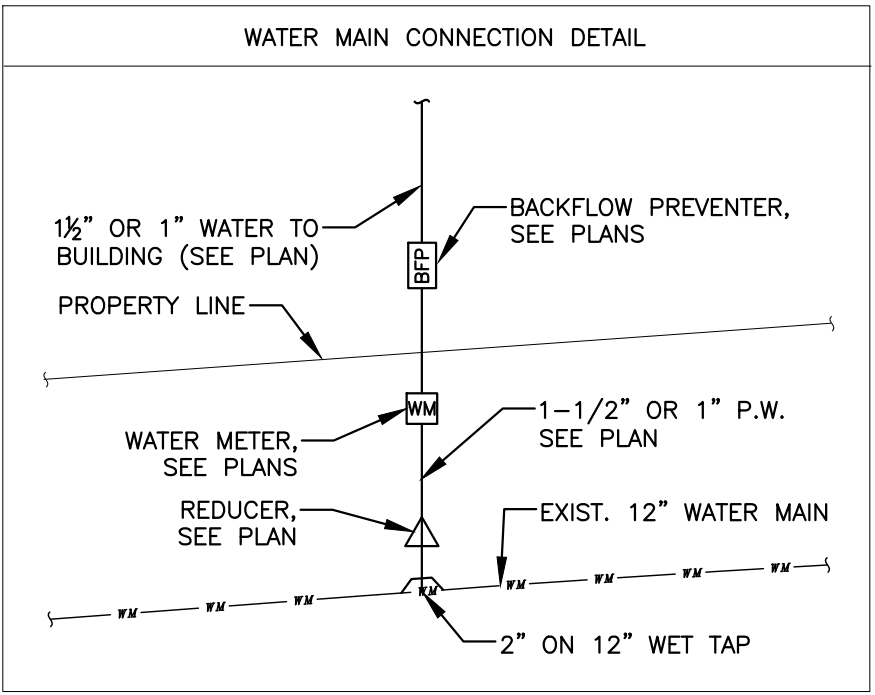
NOTE: STRUCTURE TYPES LISTED ABOVE TO BE PER FDOT PARAMETERS AND SPECIFICATIONS

NOTE: SOME DETAILS IN THESE DRAWINGS ARE TO BE CONSIDERED TYPICAL, AND MAY NOT BE CALLED OUT ON PLANS. CONTRACTOR SHALL BECOME FAMILIAR WITH AND FOLLOW ALL DETAILS DURING CONSTRUCTION IN ALL TYPICAL AREAS.

REGISTERED DESIGN PROFESSIONAL
ASA R. GILLETTE, P.E.
FLORIDA P.E. NO. 56177

SANITARY STRUCTURE TABLE						
NAME	TYPE	TOP/ INLET ELEV.	INVERT ELEVATION			
			N	S	E	W
SS-1	M/H	14.0	—	12.10	12.00	—
SS-2	M/H	14.0	—	10.95	10.95	11.05
SS-3	L.S.	14.0	—	—	10.83	—

SANITARY PIPE TABLE					
NAME	FROM	TO	SIZE	LENGTH	MATERIAL
SP-1	SS-1	SS-2	8"	238'	SDR-9
SP-2	SS-2	SS-3	8"	30'	SDR-9



NOTE: EXISTING FIRE HYDRANT IS LOCATED ON THE SOUTH SIDE OF AMELIA ISLAND PARKWAY, APPROXIMATELY ADJACENT TO THIS DRIVEWAY ENTRANCE.

UTILITY PLAN
SCALE: 1:30

GILLETTE & ASSOCIATES, INC.
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AMELIA ISLAND MEDICAL OFFICE
BEACHES DERMATOLOGY
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

DATE: 5/16/22
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UTILITY PLAN
North / Elev Key Sheet
UT-1
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**AMELIA ISLAND MEDICAL OFFICE
BEACHES DERMATOLOGY**
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

Beaches Dermatology - Amelia Island Parkway - Tree Mitigation Table									
Tree ID	Survey Label	Species	DBH (inches)	Health Status	Mitigable DBH (inches)	Retained? (Y/N)	Retained DBH (inches)	Arborist Species	
531	14"PM	PM	14	29.5		0	Y	0	Sabal palmetto
533	13"O	WO	13	26.5		0	Y	0	Quercus nigra
534	14"PM	PM	14	33.5		14	N	0	Sabal palmetto
535	12"PM	PM	12	33.5		12	Y	12	Sabal palmetto
536	12"PM	PM	12	32.5		12	Y	12	Sabal palmetto
537	12"PM	PM	12	29.5		0	N	0	Sabal palmetto
538	14"O	LO	14	26.5		0	N	0	Quercus virginiana
539	14"PM	PM	14	29.5		0	N	0	Sabal palmetto
540	14"PM (offsite)	PM	14	29.5		0	N	0	Sabal palmetto
541	15"PM	PM	15	39.5		15	Y	15	Sabal palmetto
542	14"O	LO	14	36.5		14	Y	14	Quercus virginiana
543	22"P	SP	22	41.5		22	Y	22	Pinus elliptici
544	11"HLV	AH	11	44.5		11	Y	11	Ilex opaca
545	6"O (offsite)	WO	6	39		6	Y	6	Quercus nigra
660	13"P	SP	13	36		13	Y	13	Pinus elliptici
671	8"P	SP	8	33		8	Y	8	Pinus elliptici
672	8"O	WO	8	36		8	Y	8	Quercus nigra
673	17"P	SP	17	44.5		17	Y	17	Pinus elliptici
674	18"P	SP	18	44.5		18	Y	18	Pinus elliptici
676	6"O (offsite)	WO	6	39		6	Y	6	Quercus nigra
677	15"O	WO	15	20.5		0	N	0	Quercus nigra
678	10"O	WO	10	23		0	N	0	Quercus nigra
679	18"O	WO	18	33.5		0	N	0	Quercus nigra
680	9"O	WO	9	26		0	N	0	Quercus nigra
681	14"O (offsite)	WO	14	33.5		14	Y	14	Quercus nigra
682	(218"O)	WO	16	36		0	N	0	Quercus nigra
683	9"O	WO	9	23		0	N	0	Quercus nigra
684	6"O	LAU	6	20		0	N	0	Quercus laurifolia
685	10"O	WO	10	26		0	N	0	Quercus nigra
686	13"O	WO	13	33.5		13	N	0	Quercus nigra
687	10"O	WO	12	30.5		12	N	0	Quercus nigra
688	7"O	WO	7	35		7	N	0	Quercus nigra
689	8"O	WO	8	29		0	N	0	Quercus nigra
690	13"O	WO	13	26.5		0	N	0	Quercus nigra
691	16"O	WO	16	23.5		0	N	0	Quercus nigra
692	27"O	LO	27	41.5		27	N	0	Quercus virginiana
693	11"O	WO	11	32		11	Y	11	Quercus nigra
694	10"O	WO	10	29		0	N	0	Quercus nigra
695	18"O	LO	18	41.5		18	N	0	Quercus virginiana
696	(2112"O)	LO	24	38.5		24	N	0	Quercus virginiana
697	15"O	WO	15	20.5		15	N	0	Quercus nigra
698	12"O	WO	12	36		12	N	0	Quercus nigra
699	6"O	LO	6	39		6	N	0	Quercus virginiana
700	14"O	LO	14	33.5		14	N	0	Quercus virginiana
701	12"O	WO	12	33.5		12	N	0	Quercus nigra
702	10"O	WO	10	26		0	N	0	Quercus nigra
703	7"O	WO	7	26		0	N	0	Quercus nigra
704	10"O	WO	10	36		10	N	0	Quercus nigra
705	7"O	WO	7	23		0	N	0	Quercus nigra
706	12"O	WO	12	36		12	N	0	Quercus nigra
707	7"O	WO	7	33		7	N	0	Quercus nigra
708	9"O	WO	9	33		9	N	0	Quercus nigra
709	(2114"O)	WO	28	27.5		0	N	0	Quercus nigra
710	15"O	WO	15	27.5		0	N	0	Quercus nigra
711	(2111+15"O off)	WO	37	23.5		0	Y	0	Quercus nigra
712	14"O	WO	14	33.5		14	Y	14	Quercus nigra
713	11+14"O	WO	25	27.5		0	N	0	Quercus nigra
714	6"O	WO	6	23		0	N	0	Quercus nigra
715	6"O	LO	6	39		6	Y	6	Quercus virginiana
716	7"O	WO	7	26		0	N	0	Quercus nigra
717	12"O	WO	12	33		12	Y	12	Juniperus virginiana
718	11"O	WO	11	30		11	Y	11	Quercus nigra
719	12"O	WO	12	39		12	Y	12	Quercus nigra
720	7"O	WO	7	33		7	Y	7	Quercus nigra
721	12"O	WO	12	33		12	Y	12	Quercus nigra
722	15"O	WO	15	27.5		0	N	0	Quercus nigra
723	8"O (offsite)	WO	8	42		8	Y	8	Quercus nigra
724	7"O (offsite)	LO	11	36		11	Y	11	Quercus virginiana
725	7"P	SP	7	36		7	Y	7	Pinus elliptici
726	14"P	SP	14	39		14	Y	14	Pinus elliptici
727	8"O	WO	8	36		8	Y	8	Quercus nigra
728	13"O	WO	13	36.5		13	Y	13	Quercus nigra
729	10"O	WO	10	42		10	N	0	Quercus nigra
730	12"O	WO	12	36		12	N	0	Quercus nigra
731	(218"O)	WO	16	20		0	N	0	Quercus nigra
732	12"O	WO	12	39		12	N	0	Quercus nigra
733	11"O	WO	11	33		11	N	0	Quercus nigra
734	12"O	WO	12	30		12	N	0	Quercus nigra
735	10"O	WO	10	39		10	N	0	Quercus nigra
736	12"O	WO	12	23		0	N	0	Quercus nigra
737	14"O	WO	15	33.5		15	N	0	Quercus nigra
738	6"O	WO	6	23		0	N	0	Quercus nigra
739	7"O	LO	7	36		7	N	0	Quercus virginiana
740	14"O	WO	14	36.5		14	N	0	Quercus nigra
741	11"O	WO	11	33		11	N	0	Quercus nigra
742	12"O	WO	12	39		12	N	0	Quercus nigra
743	12"O	WO	12	29		0	Y	0	Quercus nigra
744	10"O	WO	10	36		10	Y	10	Quercus nigra
745	9"O	WO	9	33		9	Y	9	Quercus nigra
746	10"O	WO	12	23		0	N	0	Quercus nigra
747	12"O	WO	12	26		0	N	0	Quercus nigra
748	11"O	WO	11	23		0	N	0	Quercus nigra
749	13"O	WO	13	23.5		0	N	0	Quercus nigra
750	14"O	WO	14	36.5		14	N	0	Quercus nigra
751	WO	WO	10	39		10	N	0	Quercus nigra
752	11"O	LO	11	33		11	N	0	Quercus virginiana
753	(2124"P	SP	48	24.5		0	N	0	Pinus elliptici
754	10+14"O	WO	24	20.5		0	N	0	Quercus nigra
755	15"O	WO	15	23.5		0	N	0	Quercus nigra
756	12"O	WO	12	33		12	N	0	Quercus nigra
757	8"O	WO	8	26		0	N	0	Quercus nigra
758	12"O	WO	12	23		0	N	0	Quercus nigra
759	14"PM	PM	14	36.5		14	Y	14	Sabal palmetto

NOTE: SOME DETAILS IN THESE DRAWINGS ARE TO BE CONSIDERED TYPICAL, AND MAY NOT BE CALLED OUT ON PLANS. CONTRACTOR SHALL BECOME FAMILIAR WITH AND FOLLOW ALL DETAILS DURING CONSTRUCTION IN ALL TYPICAL AREAS.

REGISTERED DESIGN PROFESSIONAL
ASA R. GILLETTE, P.E.
FLORIDA P.E. NO. 56177

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**AMELIA ISLAND MEDICAL OFFICE
BEACHES DERMATOLOGY**
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

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SCALE: 1"=30'
DRAWING IS REDUCED IF LESS THAN 22" x 34"
LANDSCAPE PLAN
North / Elev Key Sheet
LS-1
Page

XERISCAPE LANDSCAPING SCHEDULE			
DESIGN	STANDARD	POINTS AVAILABLE	POINTS USED
IRRIGATION	MOISTURE SENSING CONTROLLER OTHER THAN RAIN SWITCH	5	5
	LANDSCAPE PLAN DEPICTS LOW, MODERATE AND HIGH WATER USAGE ZONES AND GROUPING OF PLANTS INTO HYDROZONES	5	
GRASS	25% TO 50% OF THE GRASS AREAS USE DROUGHT TOLERANT SPECIES	5	
	51% OR MORE OF THE GRASS AREAS USE DROUGHT TOLERANT SPECIES	10	10
	LESS THAN 75% OF THE LANDSCAPED AREA CONTAINS SOD	10	10
SHRUBS	COMPACTED MULCHED PLANTING BEDS AT LEAST 3" DEEP ARE USED IN ALL PLANTING AREAS EXCEPT GROUND COVER	10	10
	25% TO 50% OF THE REQUIRED SHRUBS ARE DROUGHT TOLERANT SPECIES	5	
TREES	51% OR MORE OF THE REQUIRED SHRUBS ARE DROUGHT TOLERANT SPECIES	10	10
	25% TO 50% OF THE REQUIRED TREES ARE DROUGHT TOLERANT SPECIES	5	5
	51% OR MORE OF THE REQUIRED TREES ARE DROUGHT TOLERANT SPECIES	10	
	25% INCREASE OVER THE REQUIRED NUMBER OF SHADE TREES IN VEHICULAR USE AREAS, PROVIDED THE PLANTING AREA IS LARGE ENOUGH TO ENSURE THAT THE ADDITIONAL TREES WILL THRIVE	5	
	50% INCREASE OVER THE REQUIRED NUMBER OF SHADE TREES IN VEHICULAR USE AREAS, PROVIDED THE PLANTING AREA IS LARGE ENOUGH TO ENSURE THAT THE ADDITIONAL TREES WILL THRIVE	10	

LANDSCAPE BUFFER REQUIREMENTS
PER THE CITY OF FERNANDINA BEACH LAND DEVELOPMENT CODE 4.05.12:
PARCEL ZONING DISTRICT: MU-1
NORTH PROPERTY LINE
ZONING DISTRICT: C.I. (NASSAU COUNTY) & C-2 (F.B.)
REQUIRED BUFFER: TYPE A
MINIMUM WIDTH: 10'
BUFFER WIDTH PROVIDED: 10'
LENGTH: 745'
REQUIRED PLANTS/100':
- 2 UNDERSTORY TREES
- 15 UNDERSTORY TREES REQ'D.
PROPOSED/EXISTING PLANTS:
- 20 SAVED SHADE TREES
- 0 UNDERSTORY TREES (AT THIS TIME)
SOUTH PROPERTY LINE
ZONING DISTRICT: I-1
REQUIRED BUFFER: TYPE B
MINIMUM WIDTH: 15'
BUFFER WIDTH PROVIDED: 15'
LENGTH: 720'
REQUIRED PLANTS/100':
- 4 UNDERSTORY TREES
- 29 UNDERSTORY TREES REQ'D.
PROPOSED/EXISTING PLANTS:
- 13 SAVED SHADE TREES
- 13 UNDERSTORY TREES
EAST PROPERTY LINE
ZONING DISTRICTS: C-2
REQUIRED BUFFER: NONE REQUIRED

LANDSCAPING NOTES
1. PLANT MATERIALS SHALL MEET THE STANDARDS FOR FLORIDA NO.1 OR BETTER, AS SET OUT IN GRADES AND STANDARDS FOR NURSERY PLANTS, DEPARTMENT OF AGRICULTURE, STATE OF FLORIDA.
2. SHRUBS SHALL BE AT LEAST EIGHTEEN (18) INCHES IN HEIGHT AT THE TIME OF INSTALLATION.
3. SHRUBS SHALL BE INSTALLED AT LEAST THIRTY-SIX (36) INCHES APART, MEASURED ON CENTER.
4. ALL SHRUBS AND INTERIOR LANDSCAPING TO RECEIVE 100% IRRIGATION COVERAGE.
5. ALL AREAS SHOWN WITHOUT IMPROVEMENTS AROUND EXISTING TREES SHALL BE LEFT IN THE NATURAL STATE (UNDISTURBED).
6. GRASSSED AREAS SHALL BE PLANTED WITH SPECIES NORMALLY GROWN ON PERMANENT LAWNS IN NASSAU COUNTY. GRASSSED AREAS MAY BE SODDED, PLUGGED, SPRIGGED, OR SEEDED, PROVIDED THAT FULL COVERAGE SHALL BE ACHIEVED WITHIN ONE (1) YEAR AFTER PLANTING. SOLID SOD SHALL BE USED IN SWALES OR AREAS SUBJECT TO EROSION. GRASS SOD SHALL BE CLEAN AND REASONABLY FREE OF WEEDS AND NOXIOUS PESTS OR DISEASES. GRASS SEED SHALL BE DELIVERED TO THE JOB SITE IN BAGS WITH FLORIDA DEPARTMENT OF AGRICULTURE TAGS ATTACHED INDICATING THE QUALITY CONTROL PROGRAM. INSTALLATION AND MAINTENANCE OF ALL PLANTINGS SHALL BE IN ACCORDANCE WITH ARTICLES 4.05.03 (D) AND 4.05.03 (E) OF THE CITY OF FERNANDINA BEACH LAND DEVELOPMENT CODE.
7. OTHER ACCEPTABLE CANOPY TREE SPECIES ARE LAUREL OAK, SOUTHERN MAGNOLIA, LOBLOLLY PINE AND RED MAPLE.
8. OTHER ACCEPTABLE UNDERSTORY TREE SPECIES ARE EASTERN REDBUD, DAHOON HOLLY, AMERICAN HOLLY AND CHERRY LAUREL.

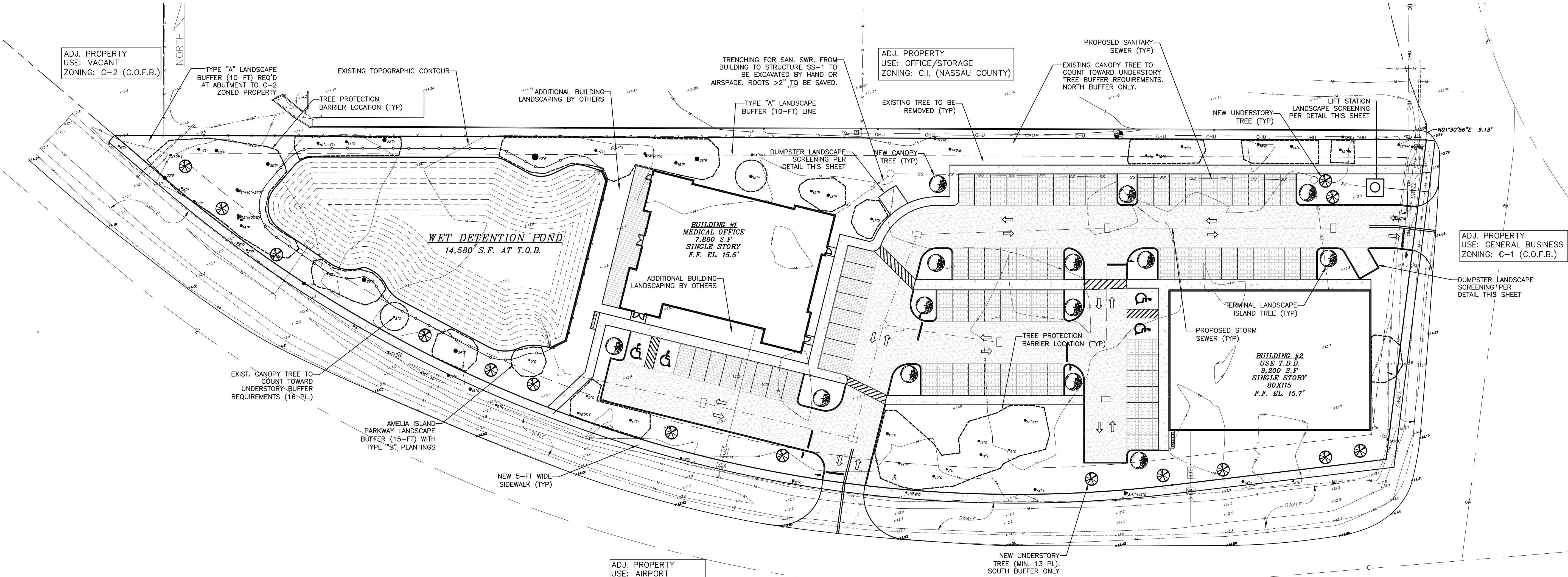
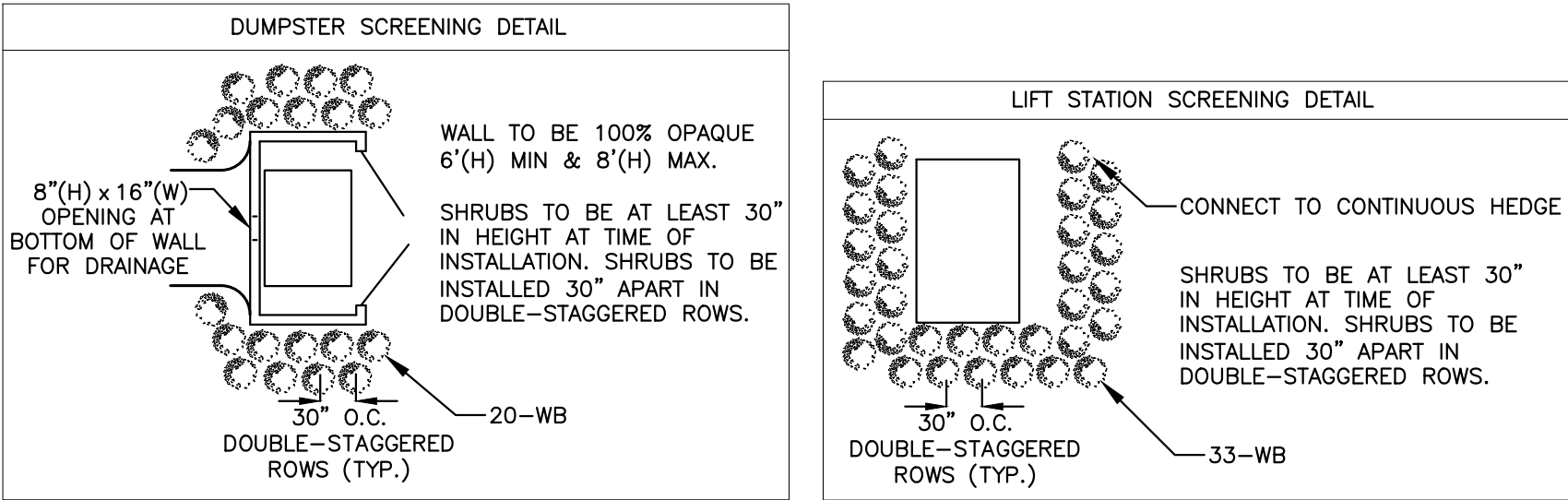
NEW PLANT/MATERIAL SCHEDULE				
SYMBOL	QTY.	KEY	SPECIES	SIZE/REMARKS
	14	LO	QUERCUS VIRGINIANA LIVE OAK	2.5" DBH AND 8' HT. MIN. (CANOPY TREE). SEE ALSO LANDSCAPE NOTES
	15	FC	CORNUS FLORIDA FLOWERING DOGWOOD	2.5" DBH AND 8' HT. MIN. (UNDERSTORY TREE). SEE ALSO LANDSCAPE NOTES.
	SEE DUMPSTER & LIFT STATION SCREENING DETAILS, SHEET LS-1	WB	VIBURNUM OBOVATUM WALTER'S VIBURNUM	18" HT AND 15" SPREAD MIN. FOR HEDGE 30" HT MIN. FOR SCREENING SHRUBS
	XXXX± SF		BOUTELOUA DACTYLOIDES	
	XXXX± SF		BUFFALO GRASS MUHLENBERGIA CAPILLARIS MUHLY GRASS	

LANDSCAPE AREA REQUIREMENTS

TOTAL LOT AREA: 114,563 SF
TOTAL LANDSCAPED AREA: 15,935 SF
MINIMUM LOT LANDSCAPE COVERAGE: 20%
PROPOSED LANDSCAPE COVERAGE: 42%
TOTAL PARKING AREA: 30,883 SF
PARKING LOT LANDSCAPE COVERAGE REQ'D: 30,088 SF (10%)
TOTAL PARKING LOT LANDSCAPING PROVIDED: 3,992 SF (13%)
MINIMUM TREE PLANTING: 1 SHADE TREE/1,500 SF = 76
EXIST. SHADE TREES SAVED: 47
SHADE TREES TO BE PLANTED: 29

LANDSCAPE MAINTENANCE REQUIREMENTS

- ALL PLANTINGS SHALL BE CONTINUALLY MAINTAINED IN AN ATTRACTIVE AND HEALTHY CONDITION. MAINTENANCE SHALL INCLUDE, BUT NOT BE LIMITED TO, WATERING, TILLING, FERTILIZING AND SPRAYING, MOWING, WEEDING, REMOVAL OF LITTER AND DEAD PLANT MATERIAL, AND NECESSARY PRUNING AND TRIMMING.
- REQUIRED PLANTS THAT BECOME DISEASED OR DIE SHALL BE REPLACED NOT LATER THAN THREE (3) MONTHS FOLLOWING THE LOSS OF THE PLANT.
- THE PROPERTY OWNER IS RESPONSIBLE FOR THE MAINTENANCE OF ALL REQUIRED LANDSCAPING IN A HEALTHY, THRIVING CONDITION.
- TREES SHALL BE PRUNED ONLY AS NECESSARY TO PROMOTE HEALTHY GROWTH OR TO AVOID POWER LINES. TREES SHALL NOT BE SEVERELY PRUNED OR "HATRACKED" IN ORDER TO PERMANENTLY MAINTAIN GROWTH AT A REDUCED HEIGHT. PRUNING SHALL COMPLY WITH CURRENT NATIONAL ARBORISTS' SOCIETY STANDARDS.



LANDSCAPE PLAN
SCALE: 1:30

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BEACHES DERMATOLOGY**

AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

DATE: 5/16/22

SCALE: NONE

DRAWING IS REDUCED IF LESS THAN 22" x 34"

SITE DETAILS

North / Elev Key Sheet

DT-1

Page

NOTE:
SOIL ANALYSIS MAY INDICATE THE NEED FOR A THICKER BASE COURSE THAN SHOWN HEREIN.

6" THICK, 3,000 PSI CONCRETE SLAB
W/6x6xW1.4 W.W.M.

12" CONSTRUCTED SUB-BASE
MIN. LBR 40 & 98% COMPACTION

1'
TOP OF CURB
6"

DUMPSTER PAD DETAIL

RIBBON CURB DETAIL

NOTE:
SOIL ANALYSIS MAY INDICATE THE NEED FOR A THICKER BASE COURSE THAN SHOWN HEREIN. THE PAVEMENT THICKNESSES SHOWN HEREIN ARE NOT INTENDED TO BE ABSOLUTE, BUT ARE PRELIMINARY CRITERIA AND MAY BE MODIFIED TO ACCOMMODATE THE BEARING CAPACITIES OF VARIOUS SUBGRADES.

1 1/2" ASPHALTIC CONCRETE
TYPE SP-9.5

6" COMPACTED LIMEROCK BASE
MIN. LBR 100

12" CONSTRUCTED SUB-BASE
MIN. LBR 40 & 98% COMPACTION

NOTE:
SOIL ANALYSIS MAY INDICATE THE NEED FOR A THICKER BASE COURSE THAN SHOWN HEREIN. THE PAVEMENT THICKNESSES SHOWN HEREIN ARE NOT INTENDED TO BE ABSOLUTE, BUT ARE PRELIMINARY CRITERIA AND MAY BE MODIFIED TO ACCOMMODATE THE BEARING CAPACITIES OF VARIOUS SUBGRADES.

PERVIOUS PAVERS

12" CONSTRUCTED SUB-BASE
MIN. LBR 40 & 98% COMPACTION

TYPICAL PAVEMENT DETAIL

PERVIOUS PAVER SUB-BASE DETAIL

3/4" R (TYP)
3500 PSI CONCRETE
W/ ONE 6X6XW2.4 W.W.M.

18"
3'
7"

NOTE:
SOIL ANALYSIS MAY INDICATE THE NEED FOR A THICKER BASE COURSE THAN SHOWN HEREIN. THE PAVEMENT THICKNESSES SHOWN HEREIN ARE NOT INTENDED TO BE ABSOLUTE, BUT ARE PRELIMINARY CRITERIA AND MAY BE MODIFIED TO ACCOMMODATE THE BEARING CAPACITIES OF VARIOUS SUBGRADES.

6" THICK, 3500 PSI CONCRETE SLAB
W/ ONE 6X6XW2.4 W.W.M.

12" CONSTRUCTED SUB-BASE
MIN. LBR 40 & 98% COMPACTION

VALLEY GUTTER DETAIL

DRIVEWAY CONNECTION DETAIL

FINISH GRADE
6"
1/2" R
6"
24"
7"
18"
ASPHALT OR CONC.

MODIFIED TYPE "C" INLET
TOP EL. = 14.90
WEIR EL. = 13.5'
24" NORTH, 12" EAST
MATCH EXISTING GRADE
P-OFF-1, 18" Ø, 33-FT
INV. EL. 12.00'
EXIST. PARKWAY DITCH
6" CAP WITH 2.8" DRILLED ORIFICE
POND BOTTOM = EL. 5.0
L3:1 SLOPE (TYP)
B.O. WALL EL. 10.0'
RETAINING WALL (BY OTHERS)
TOP OF BANK EL. = 15.0'
FIN. GRADE
25 YR D.H.W.L. = 14.9'
WEIR EL. = 13.5'
N.W.L. = 12.5'POND CROSS SECTION AND DETAIL

6" HEADER CURB DETAIL

RETAINING WALL DETAIL (WHERE REQ'D)

GRATE
WEIR NOTCH (TYP N & E SIDES)
TOP OF BOX EL. 14.9
BOTTOM OF WEIR NOTCH EL. 13.5
MODIFIED TYPE "C" INLET
DRAW DOWN ORIFICE, CENTERLINE EL. 12.5'
18" N. SIDE; 12" E. SIDE
P-OUT-1, SOUTH SIDE (INV. EL. = 12.50)
54"

DUMMY JOINT
EXPANSION JOINT
PREFORMED EXPANSION JOINT MATERIAL

OUTFALL STRUCTURE DETAIL

SIDEWALK JOINT DETAILS

AT ASPHALT PAVEMENT

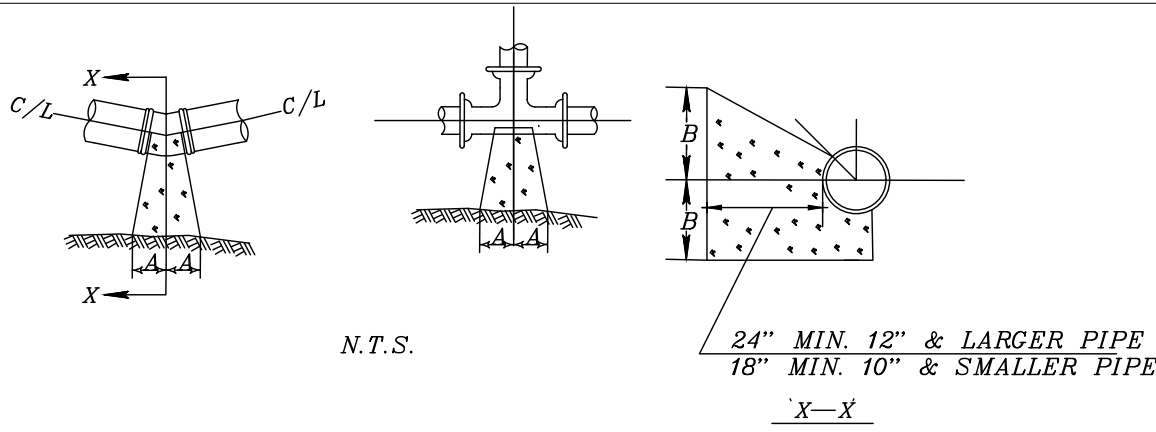
AT GRADE
TURN DOWN AS REQUIRED TO KEEP "A" ≥ 3"

SIDEWALK TURNDOWN DETAILS

30" AT LOWEST POINT
12" L 12"
12"
6"
3"
2"
6"
GRADE
#5 AT 16" O.C.
FORMED AND POURED CONCRETE
24"
6"
24"
#5 HOOK AT 16" O.C.
2-#4 CONT. (T&B)
#4 AT 24" O.C.

NOTE: SOME DETAILS IN THESE DRAWINGS ARE TO BE CONSIDERED TYPICAL AND MAY NOT BE CALLED OUT ON PLANS. CONTRACTOR SHALL BECOME FAMILIAR WITH AND FOLLOW ALL DETAILS DURING CONSTRUCTION IN ALL TYPICAL AREAS.

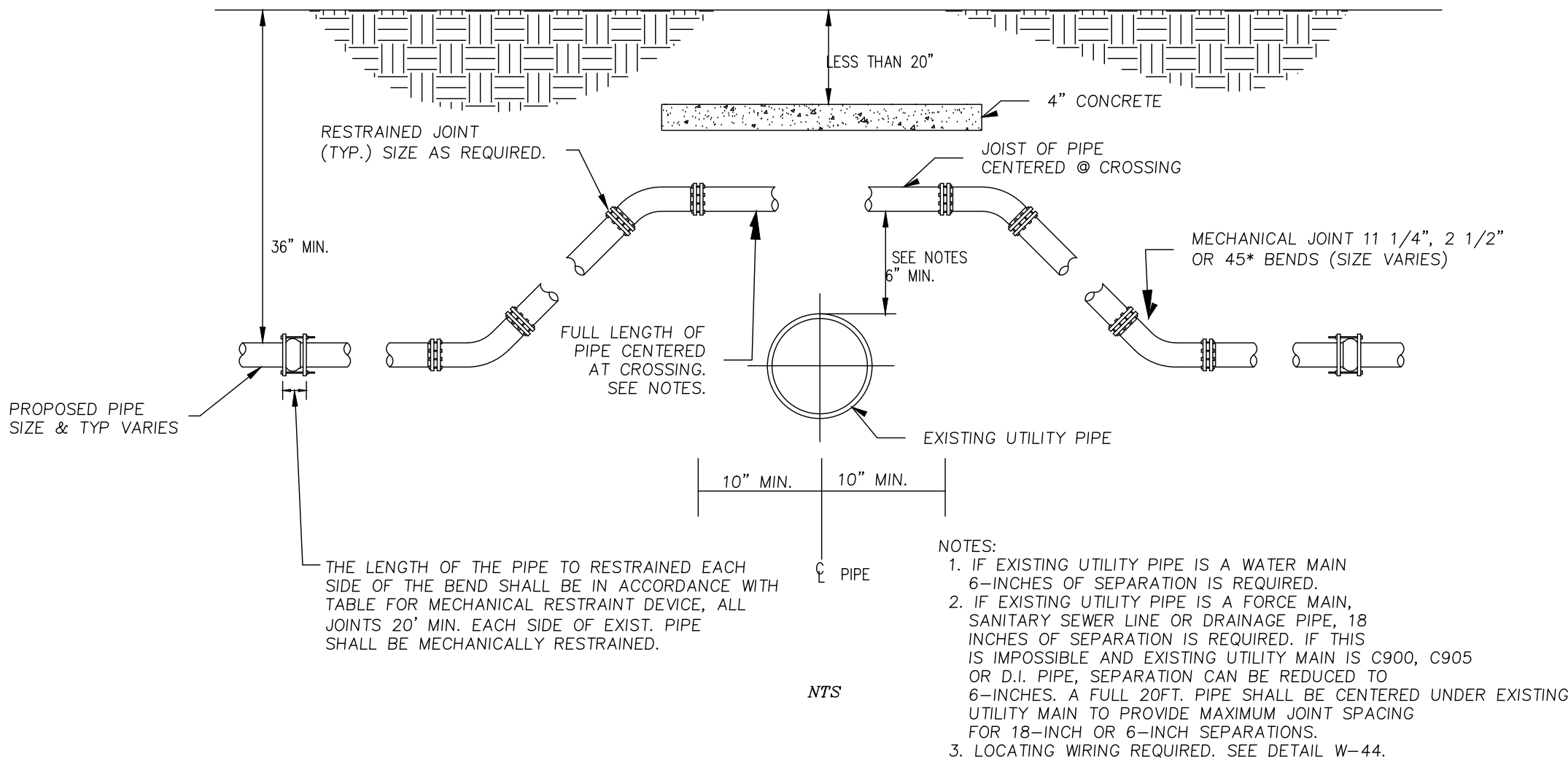
REGISTERED DESIGN PROFESSIONAL
ASA R. GILLETTE, P.E.
FLORIDA P.E. NO. 56177



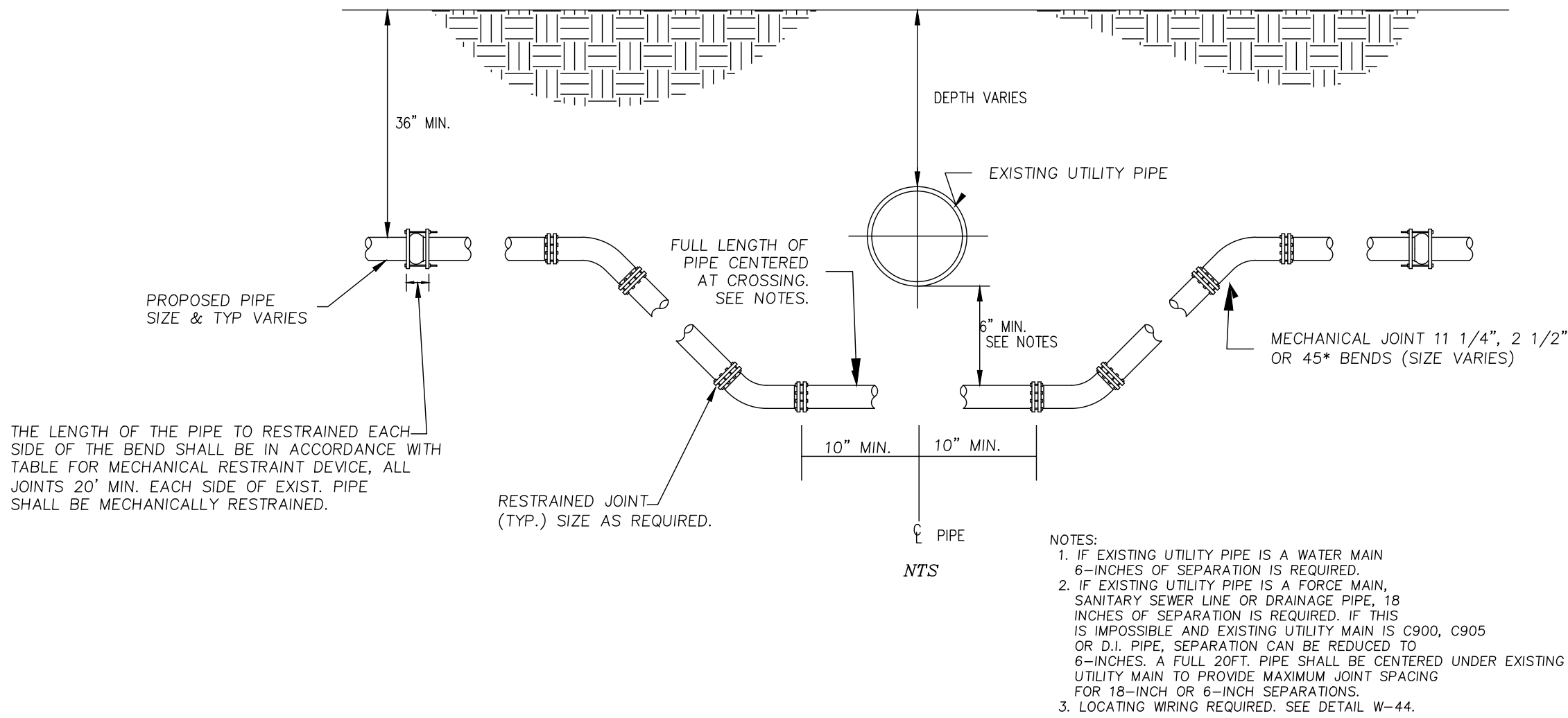
L/4 BENDS				FT/1/8 BENDS				FT/1/16 BENDS				TEE				CU FT	
SIZE	FT	B	CONC.	SIZE	FT	B	CONC.	SIZE	FT	B	CONC.	SIZE	FT	B	CONC.	CONC.	CONC.
6"	16"	10"	3.2	9"	10"	3.0	6"	8"	1.3	6"	10"	10"	12"			3.5	
8"	22"	13"	7.2	12"	13"	3.7	8"	10"	2.1	8"	13"	16"				5.4	
10"	26"	17"	11.5	14"	17"	5.9	10"	13"	3.6	10"	16"	20"				9.4	
12"	29"	21"	14.1	16"	21"	8.6	11"	16"	6.6	12"	18"	24"				16.6	
14"	35"	24"	29.8	19"	24"	14.9	12"	20"	8.9	14"	22"	27"				23.2	
16"	38"	27"	37.8	21"	27"	18.1	12"	24"	11.2	16"	24"	30"				28.6	
20"	47"																

NOTE:
1. ALL BEARING SURFACES TO BE CARRIED TO UNDISTURBED SOIL.
2. THESE TABLES SHOW MINIMUM SIZE THRUST BLOCKS FOR GOOD SOIL, (A-1 THROUGH A-3, CLEAN SAND AND GRAVELS)
3. POOR SOIL (A-4 THROUGH A-8, SILTY SOILS, CLAYS, MUCK AND PEAT) WILL REQUIRE LARGER THRUST BLOCKS.

THRUST BLOCKS FOR BENDS & TEE'S

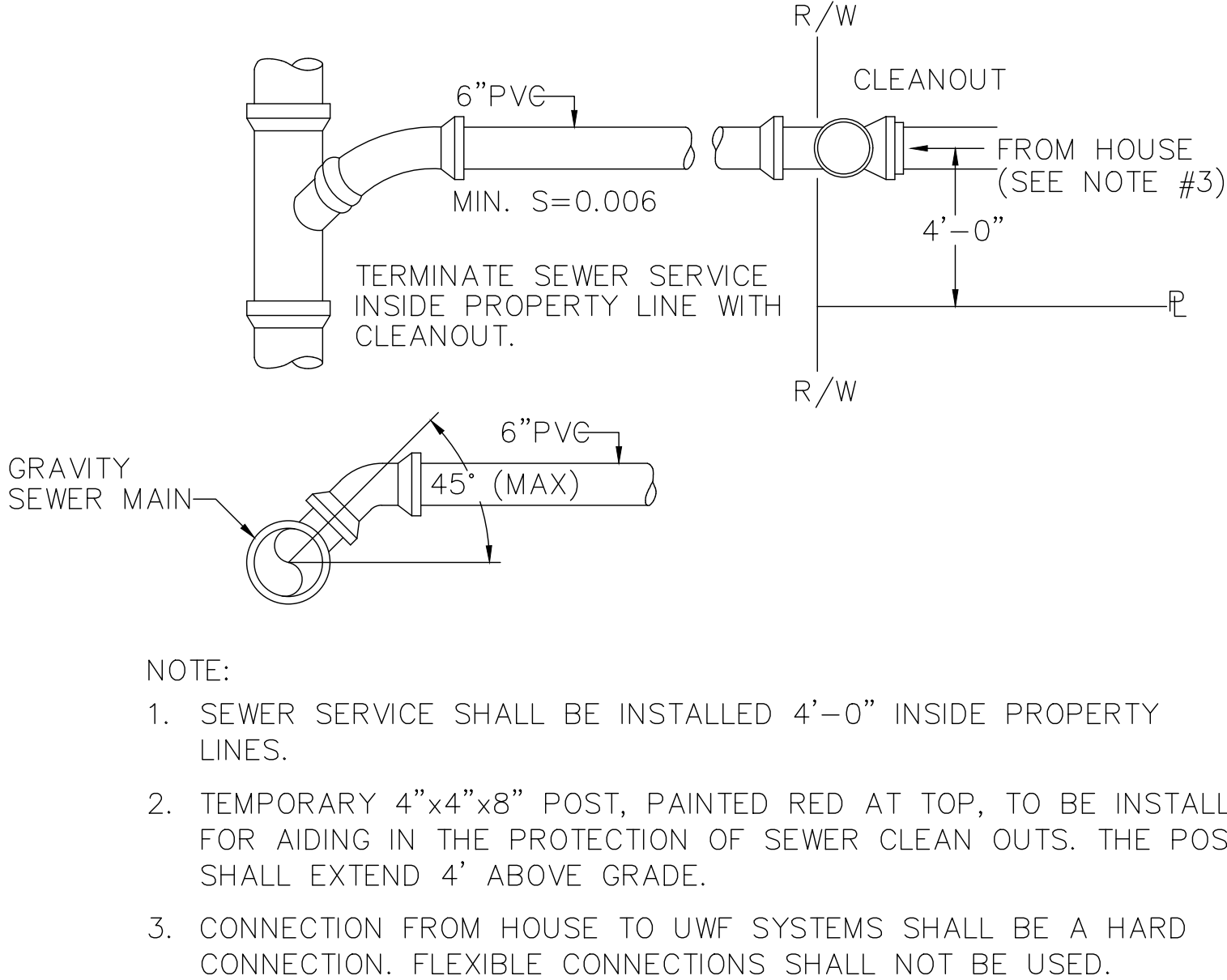
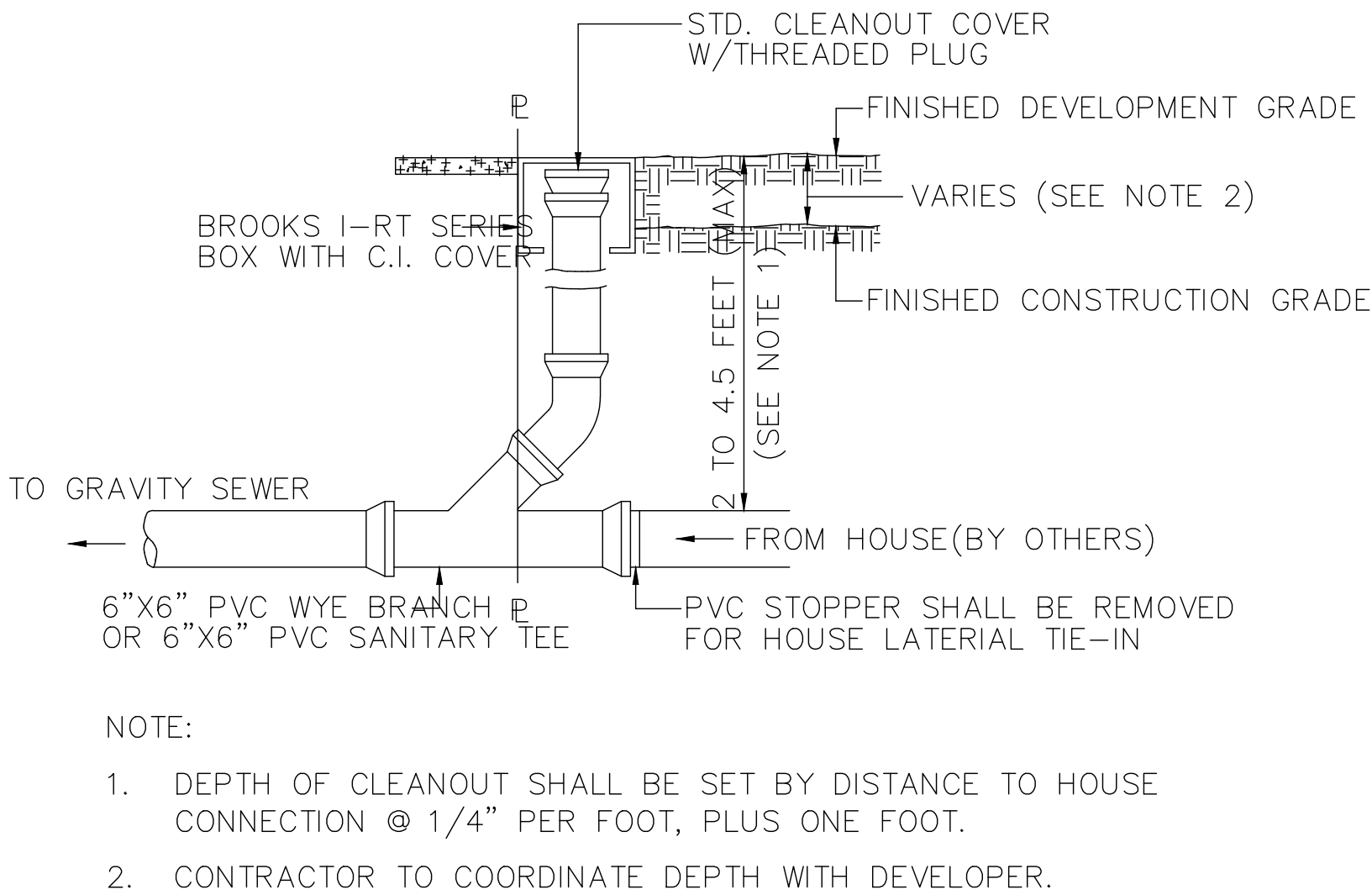


UTILITY CONFLICT DETAIL "A"



UTILITY CONFLICT DETAIL "B"

GRAVITY SEWER NOTES



TYPICAL SEWER SERVICE

CONTRACTORS ARE REQUIRED TO PAINT NEW HYDRANTS SAFETY YELLOW AND MARK HYDRANTS. THE TOPS AND NOZZLE CAPS SHALL BE PAINTED WITH THE FOLLOWING CAPACITY INDICATOR:

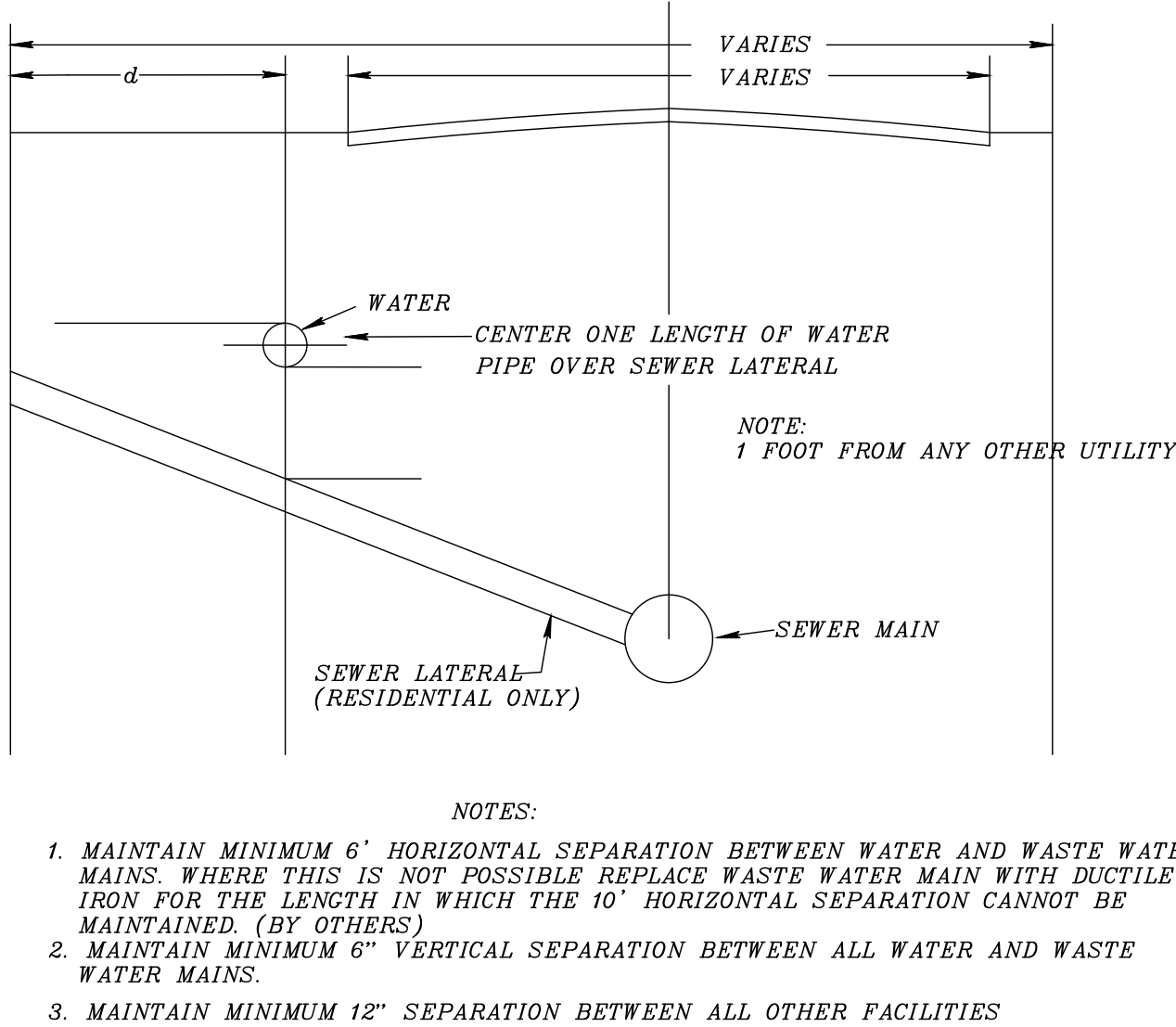
COLOR SCHEME:
BLUE (SW4079) 1500GPM AND HIGHER
GREEN (SW4085) 1000-1499 GPM
ORANGE (SW4083) 500-999 GPM
RED (SW4081) LESS THAN 500 GPM

IT IS REQUIRED THAT ALL NEW AND RELOCATED WATER MAINS BE PRESSURE TESTED AND LEAKAGE TESTED IN ACCORDANCE WITH AWWA STANDARD C600.

IT IS REQUIRED THAT ALL NEW AND RELOCATED WATER MAINS BE DISINFECTED IN ACCORDANCE WITH AWWA STANDARD C651 AND RULE 62-555.345, F.A.C..

ALL WATER MAINS WILL BE CONSTRUCTED WITH BLUE C900-DR25 PIPE
ALL WATER SERVICES WILL CONSTRUCTED WITH WHITE PVC SCHEDULE 80 PIPE. BLUE SPRAY PAINT WILL BE APPLIED TO THE TOP SURFACE.
JOINT RESTRAINERS MAY BE USED IN PLACE OF THRUST BLOCKS ON C900, DR25 PIPE. WITH APPROVAL OF THE CITY UTILITIES DEPARTMENT.

CLEANOUT



HORIZONTAL AND VERTICAL CLEARANCES

POTABLE WATER NOTES

GILLETTE & ASSOCIATES, INC.
CIVIL, ENVIRONMENTAL, MECHANICAL, & STRUCTURAL ENGINEERING
CERTIFICATE OF AUTHORIZATION NO. 9332
ASA R. GILLETTE, P.E. * FL PE NO. 56177
20 S. 4th STREET * FERNANDINA BEACH, FL 32034
PHONE: 904-261-8819 * www.gilletteassociates.com

AMELIA ISLAND MEDICAL OFFICE
BEACHES DERMATOLOGY
AMELIA ISLAND PARKWAY
FERNANDINA BEACH, FLORIDA

DATE: 5/16/22
SCALE: NONE
DRAWING IS REDUCED IF LESS THAN 22" x 34"
WATER & SEWER DETAILS
North / Elev Key Sheet
DT-3
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